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Agency Correspondence

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LITTLE BENNETT REGIONAL PARK DAY USE AREA

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4TH ELECTION DISTRICT CLARKSBURG, MARYLAND



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SEPTEMBER 14, 2011

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The Maryland-National Capital Park and Planning Commission

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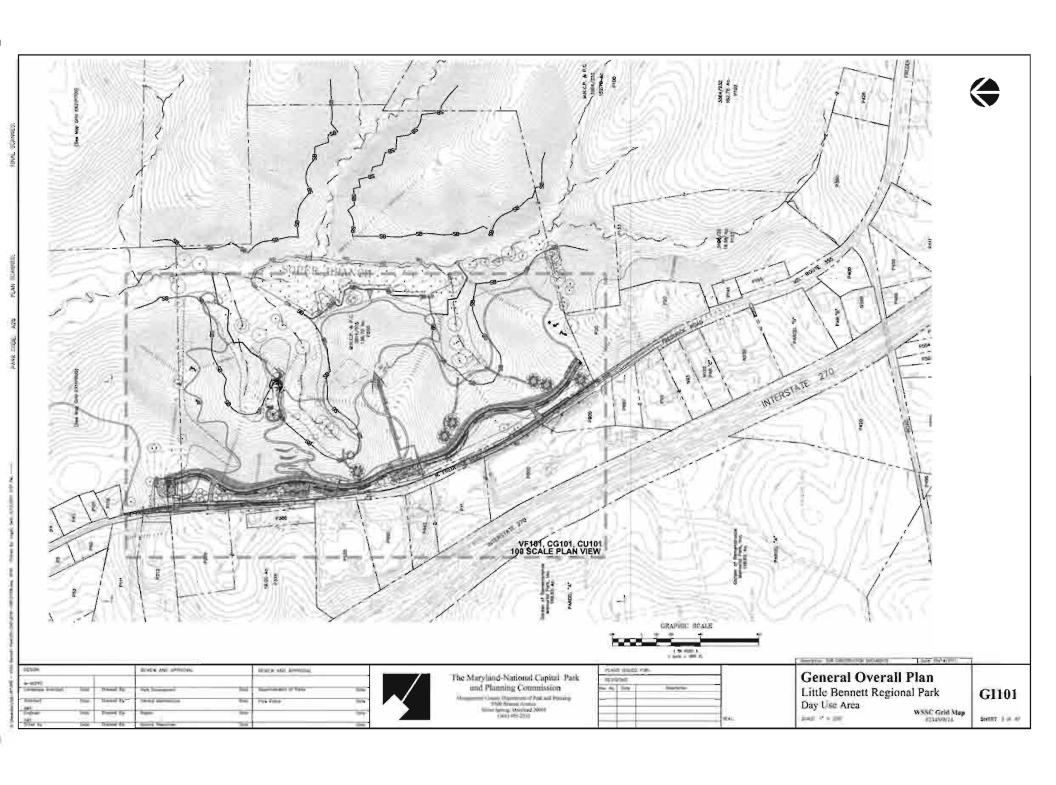
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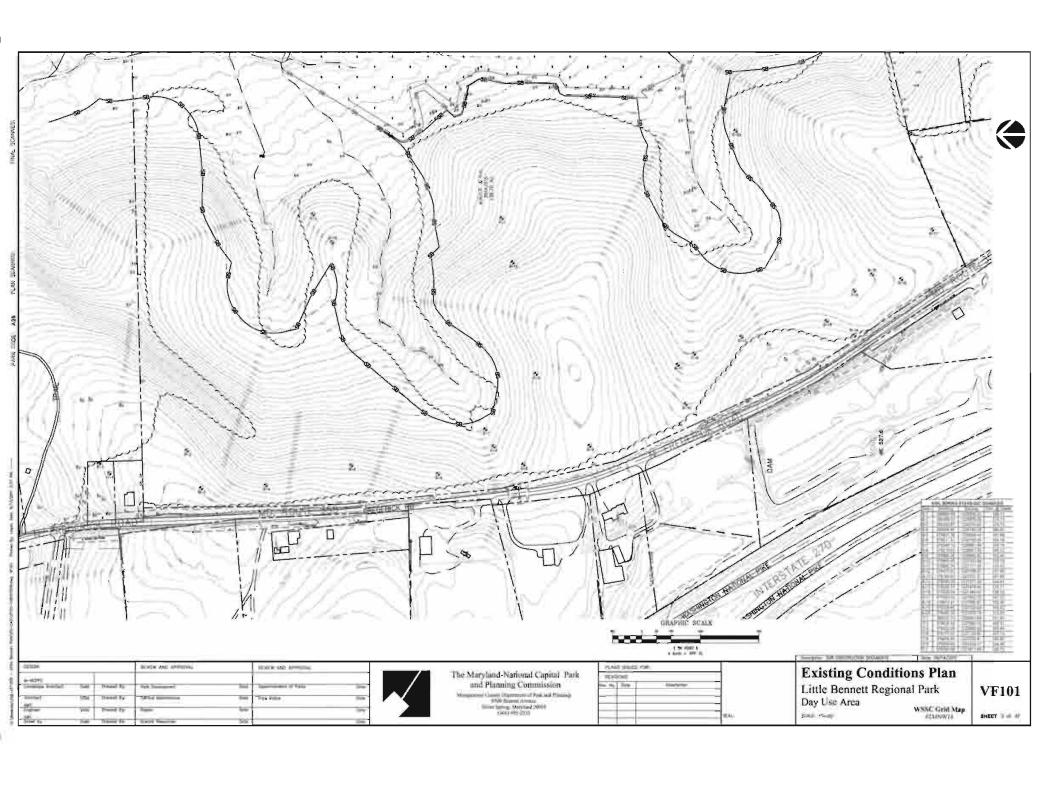
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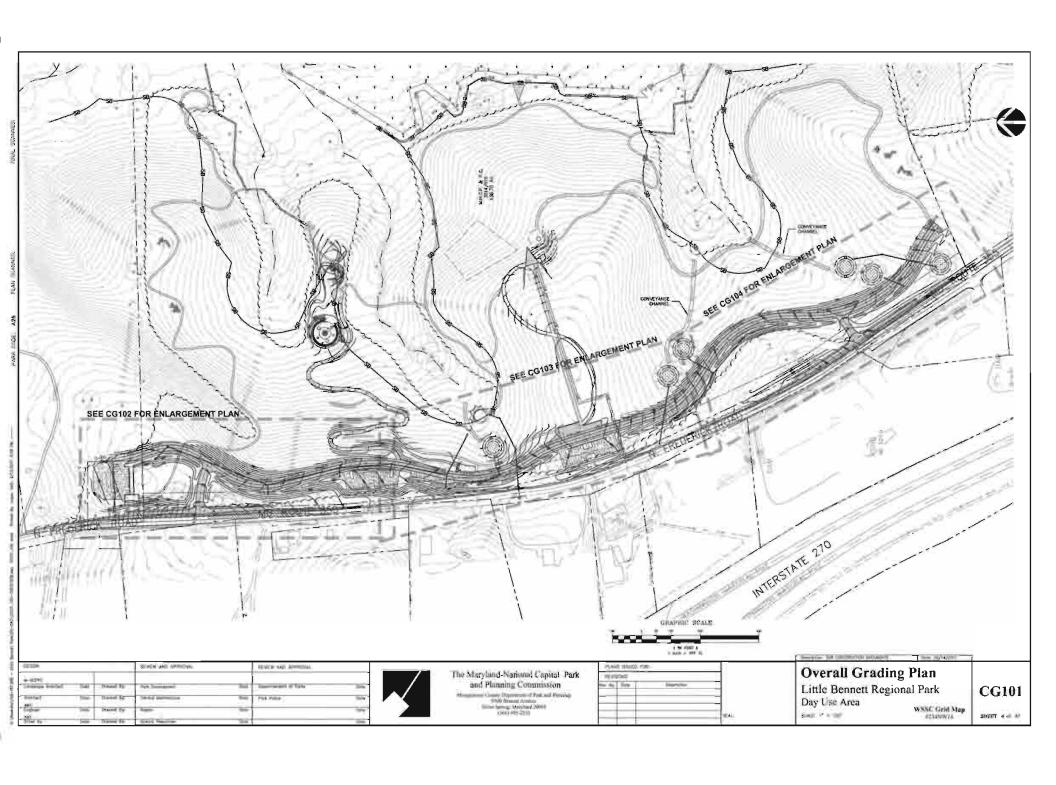
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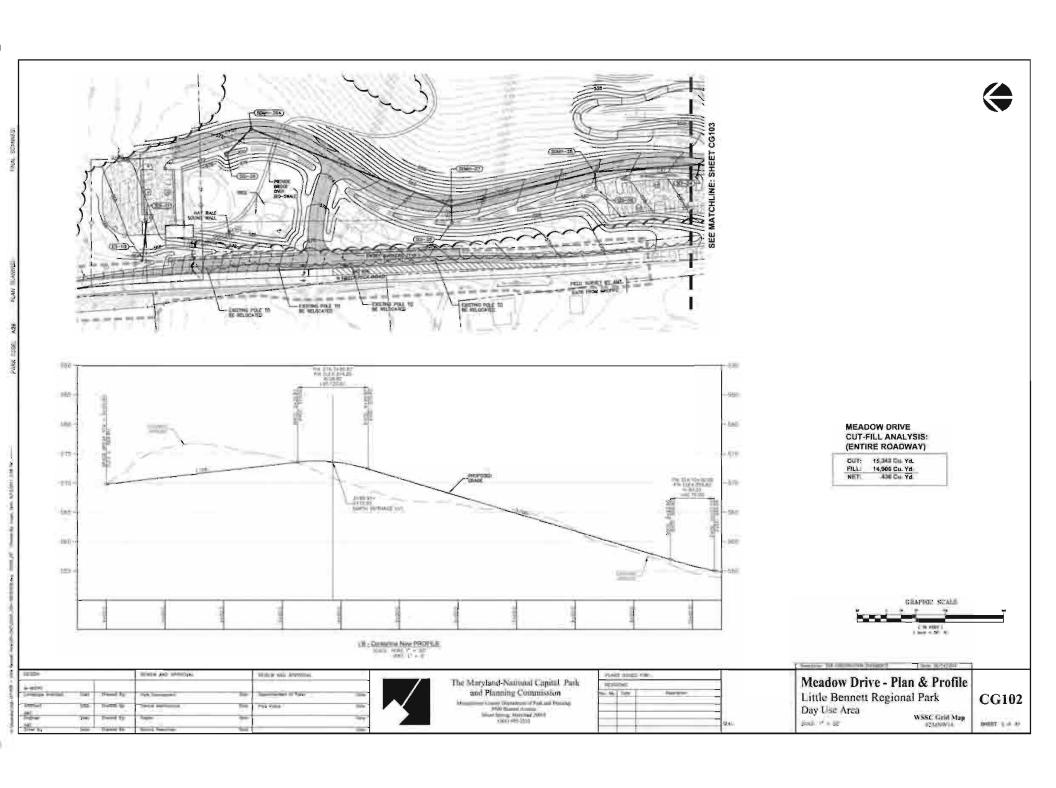
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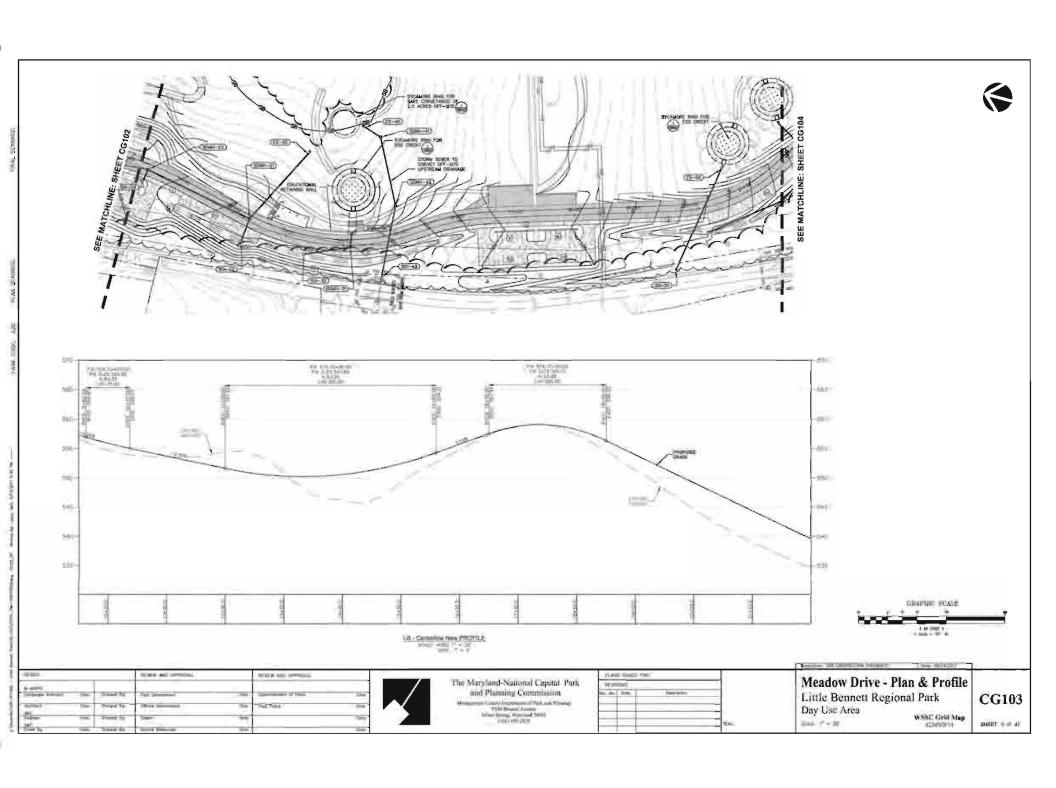
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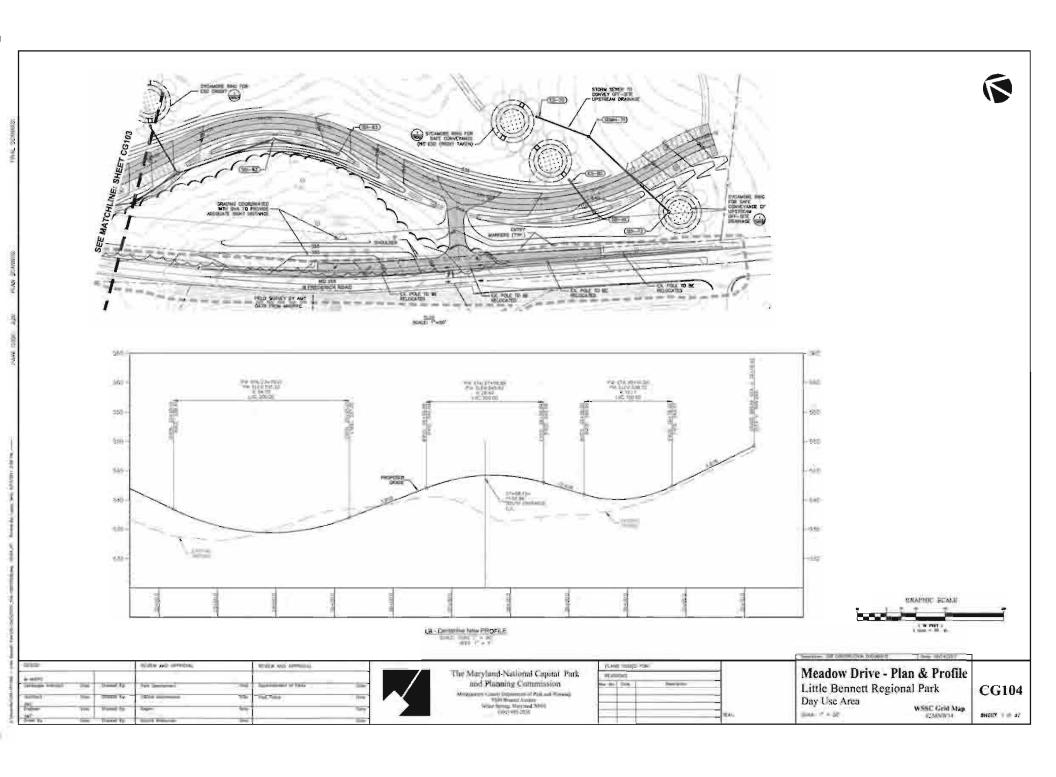












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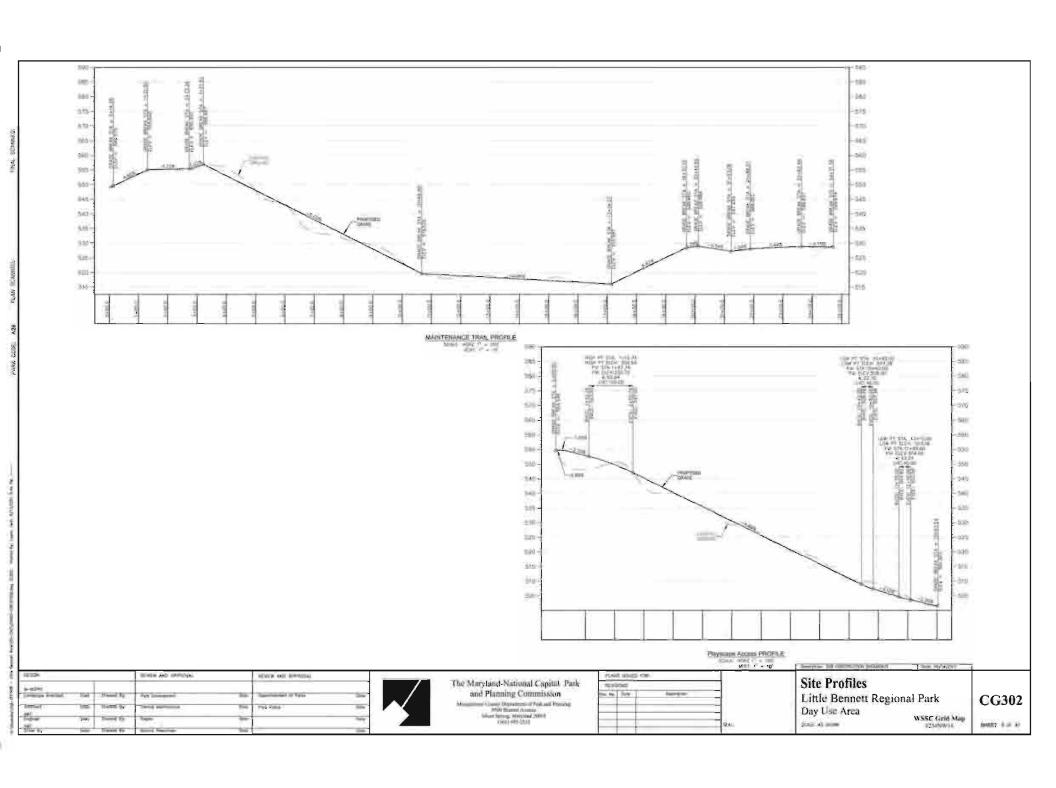
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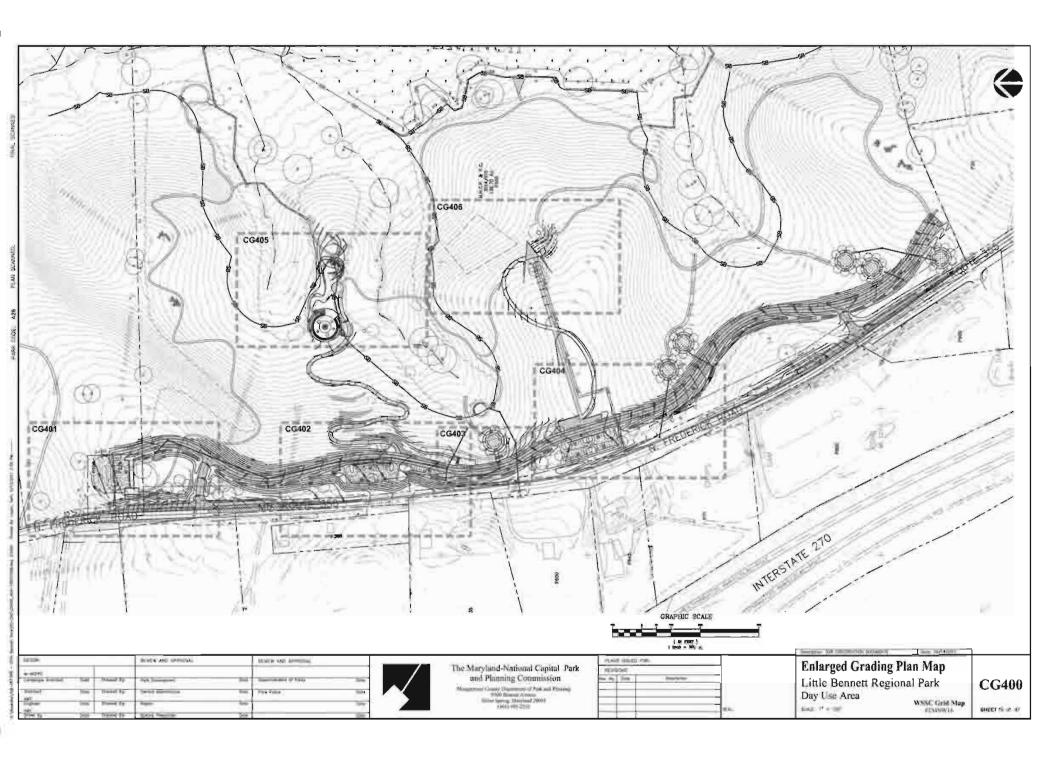
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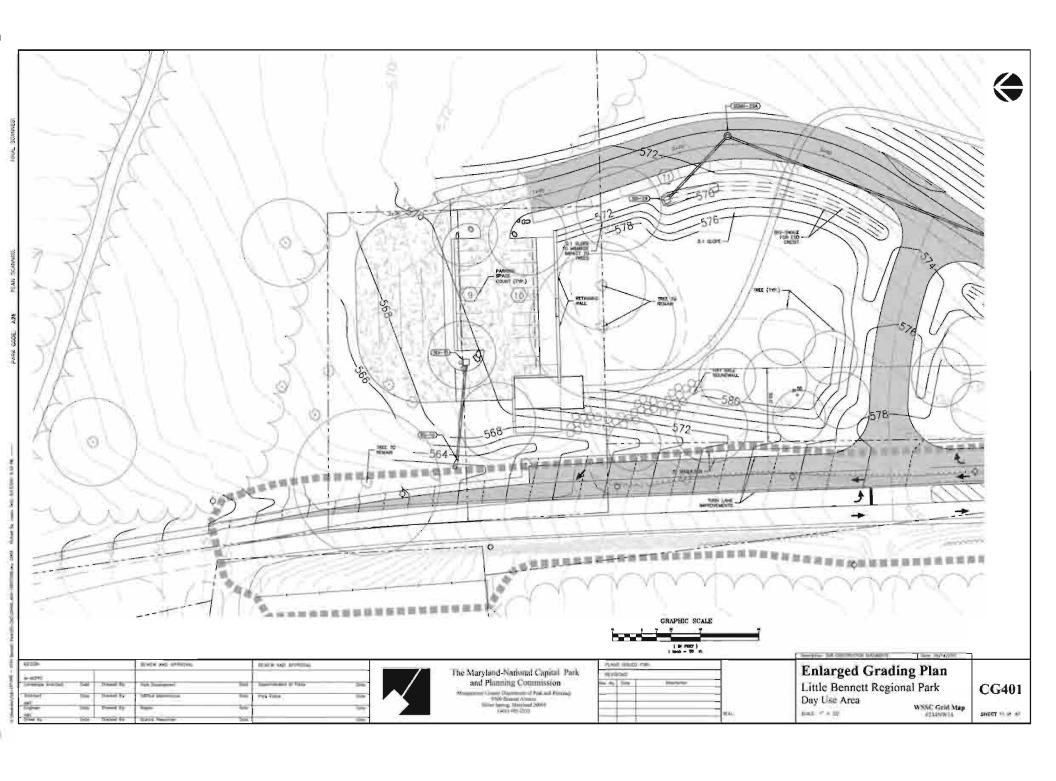
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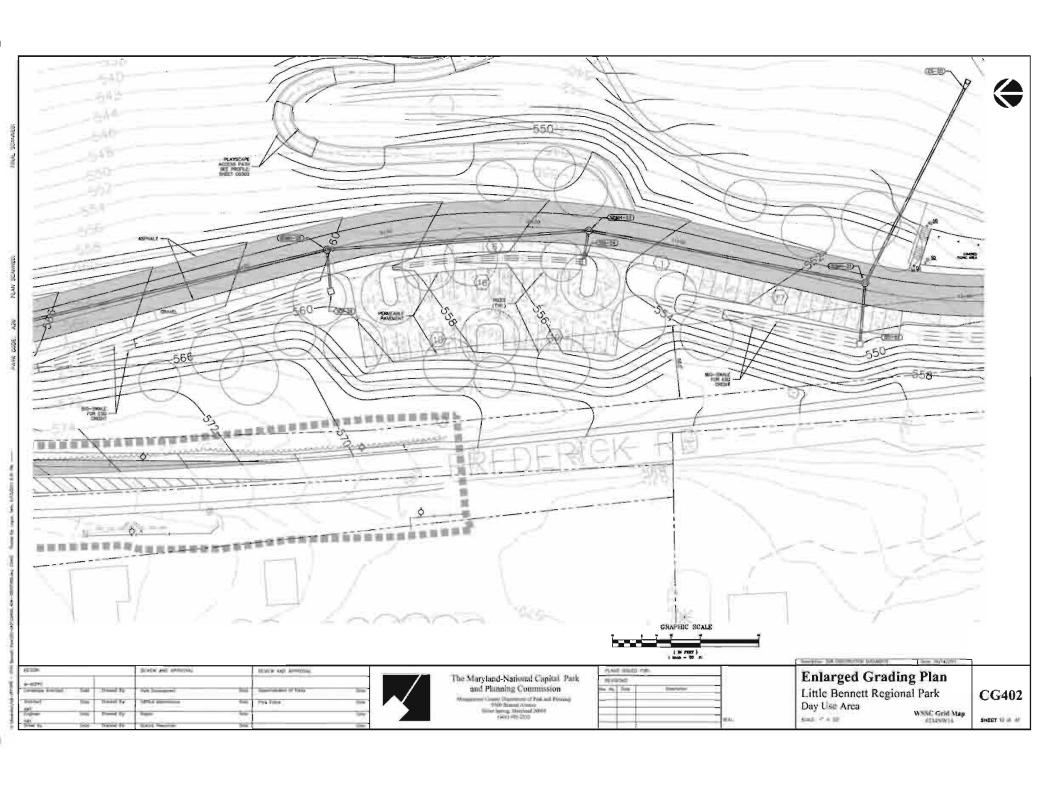
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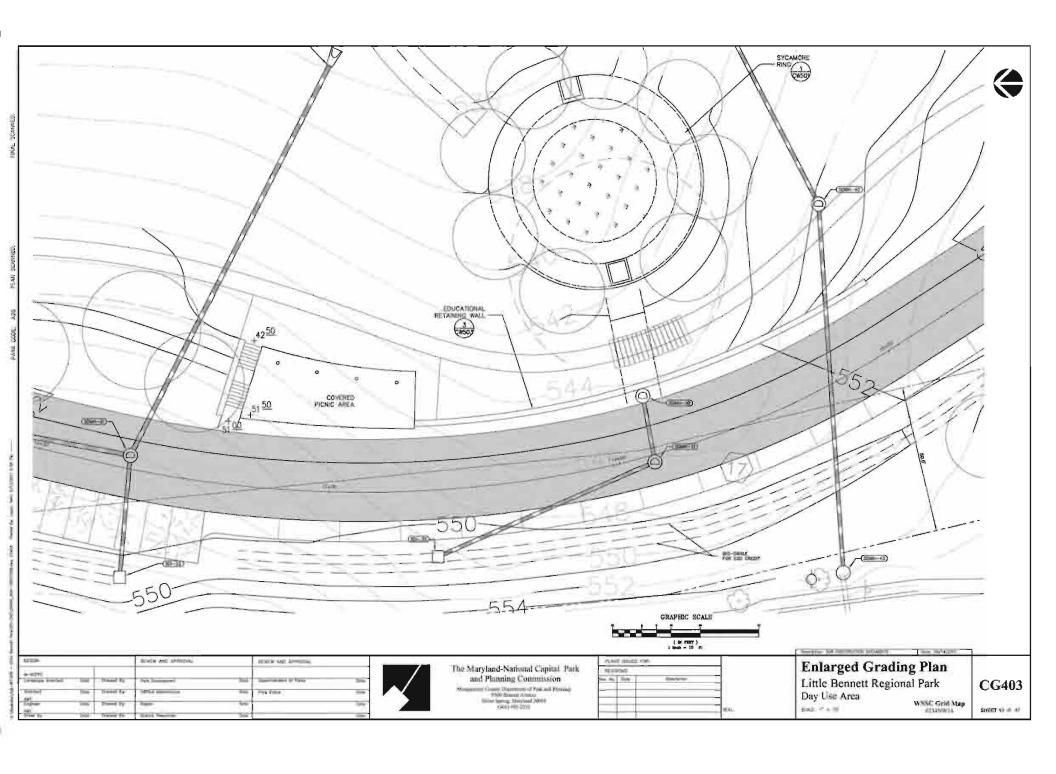
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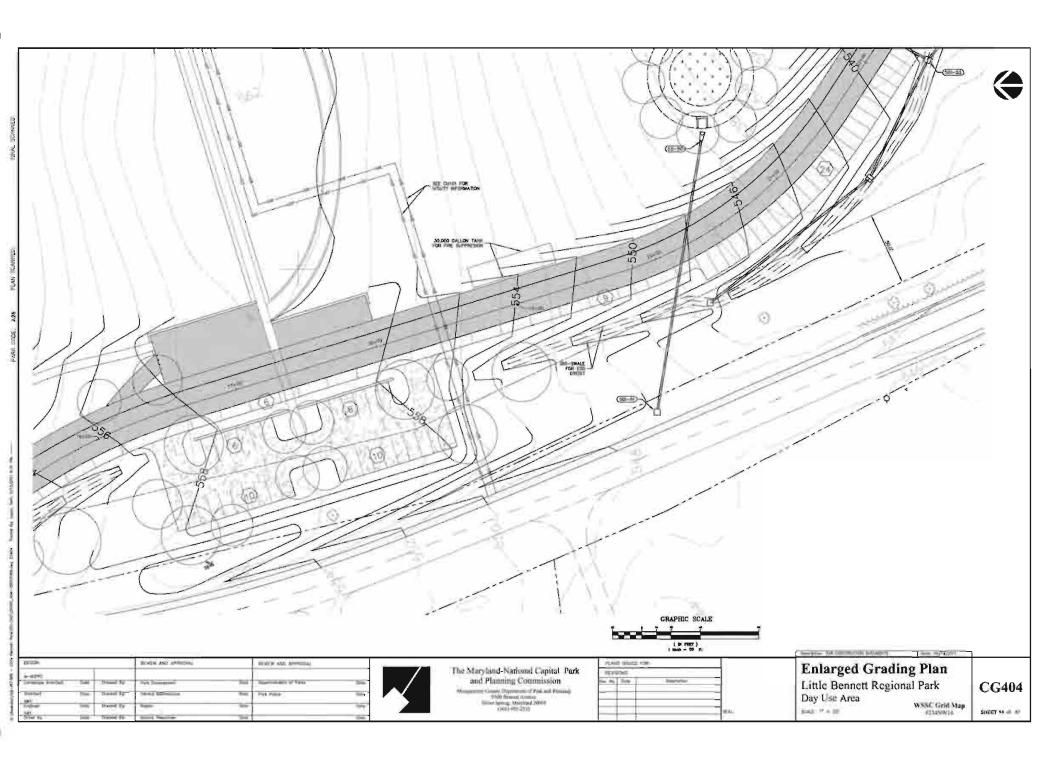


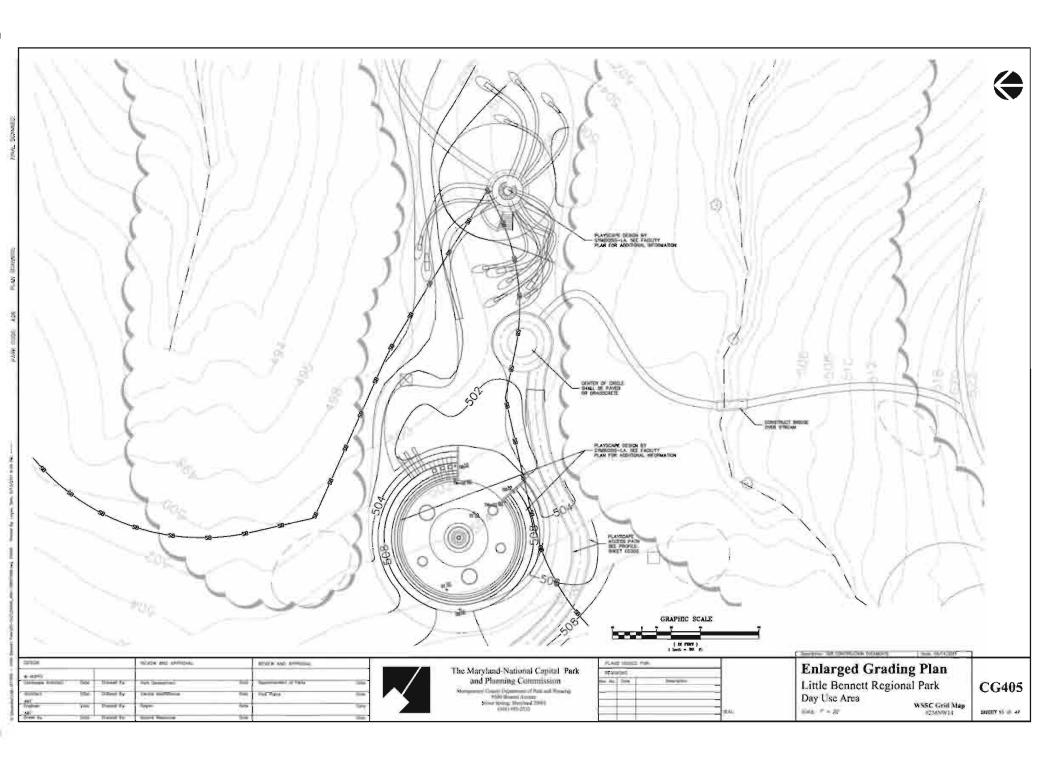


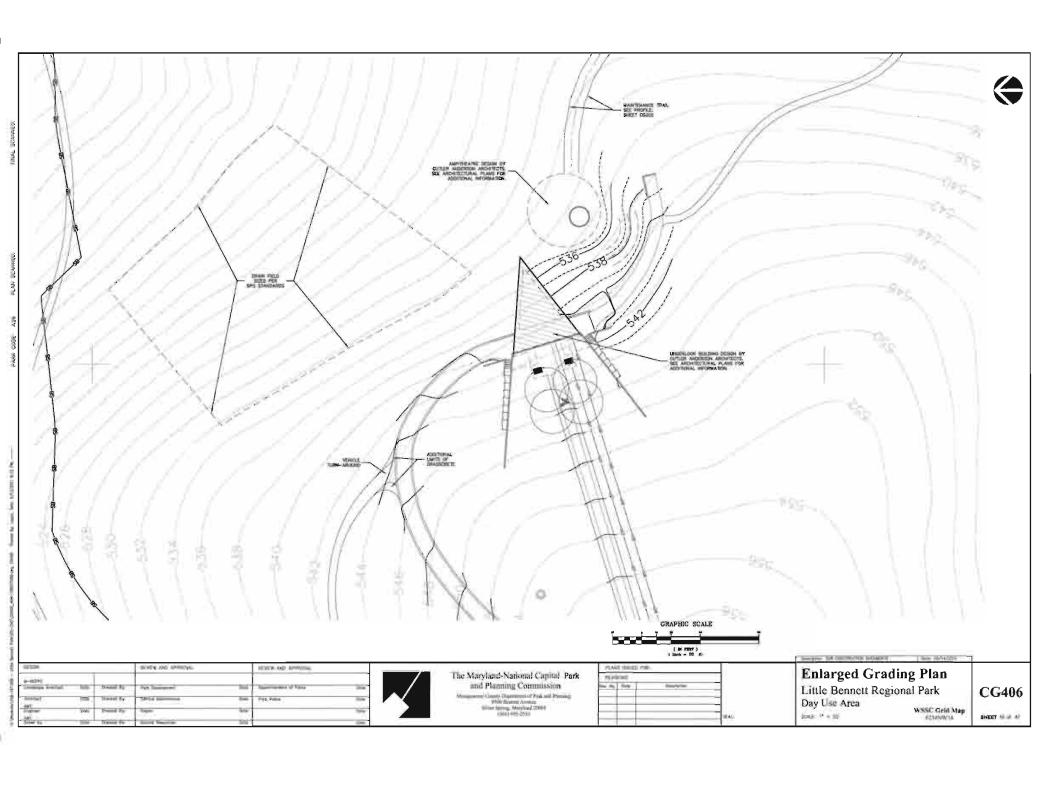


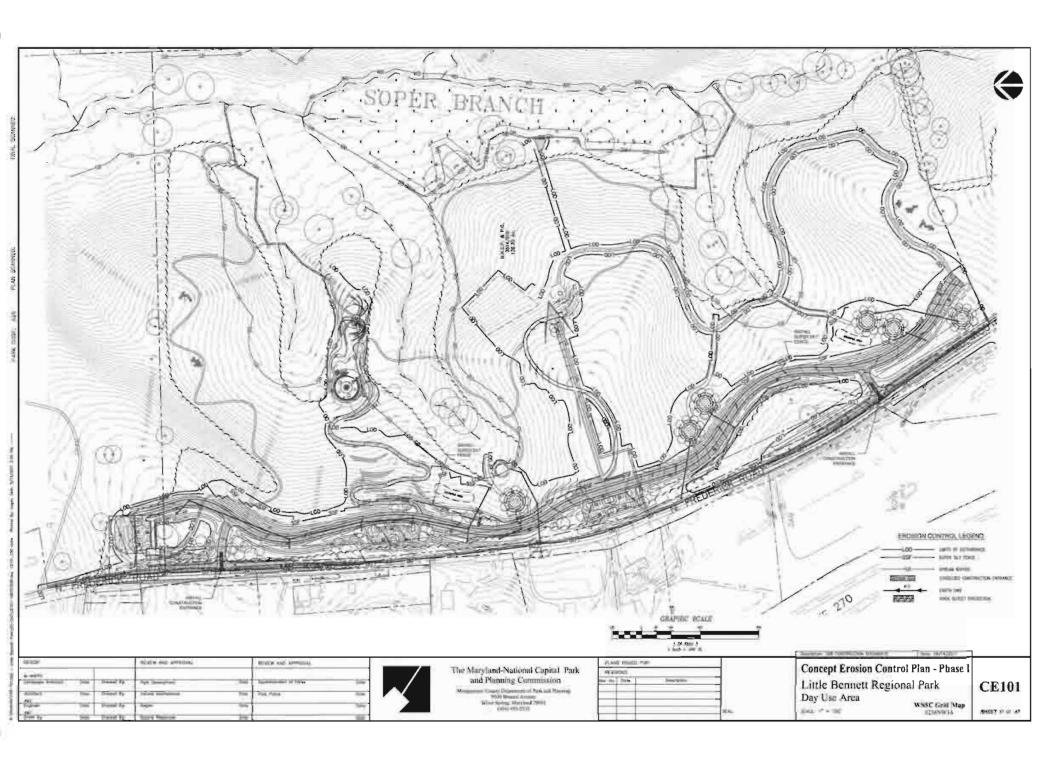


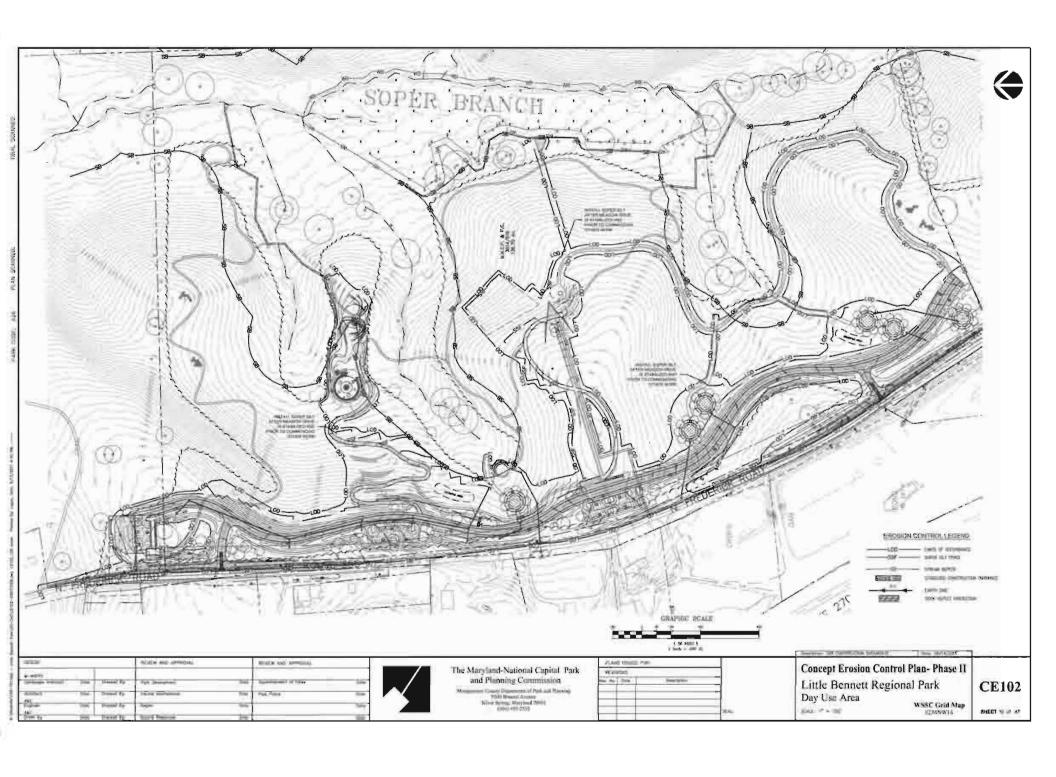


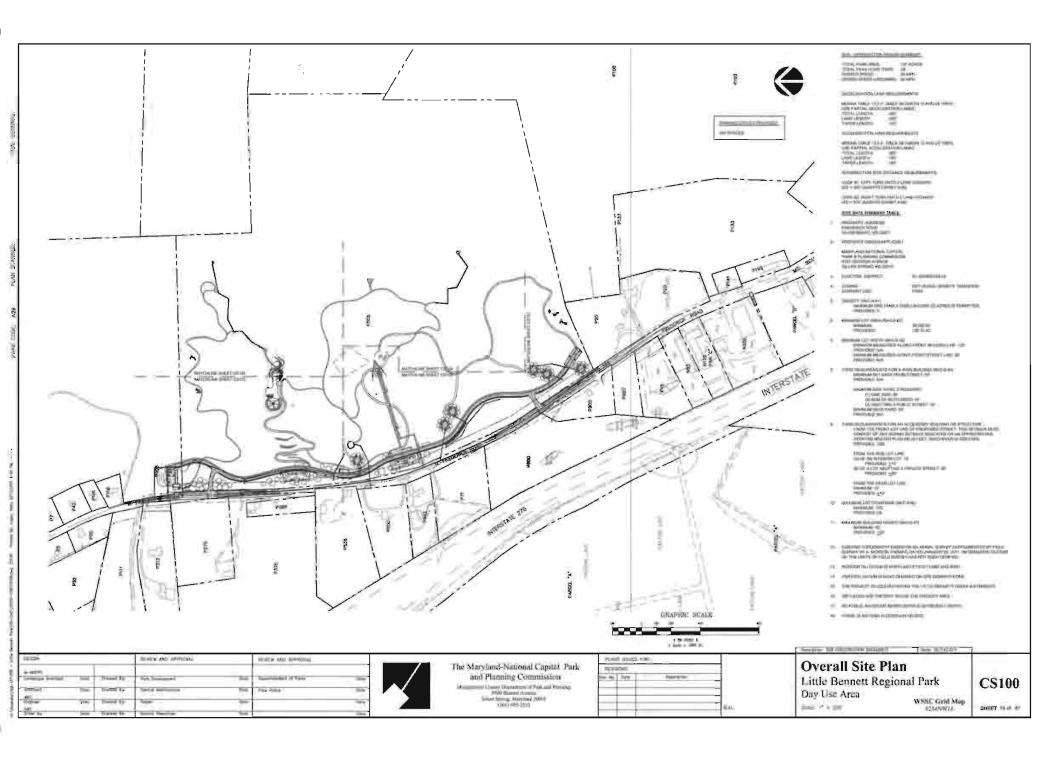


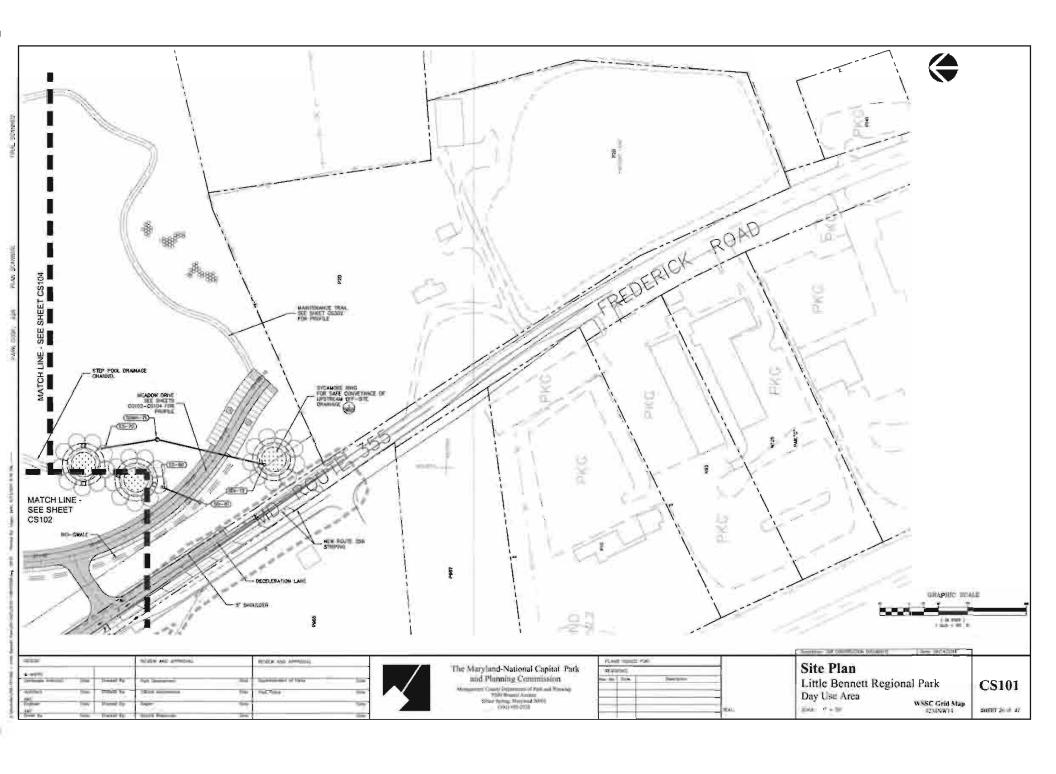


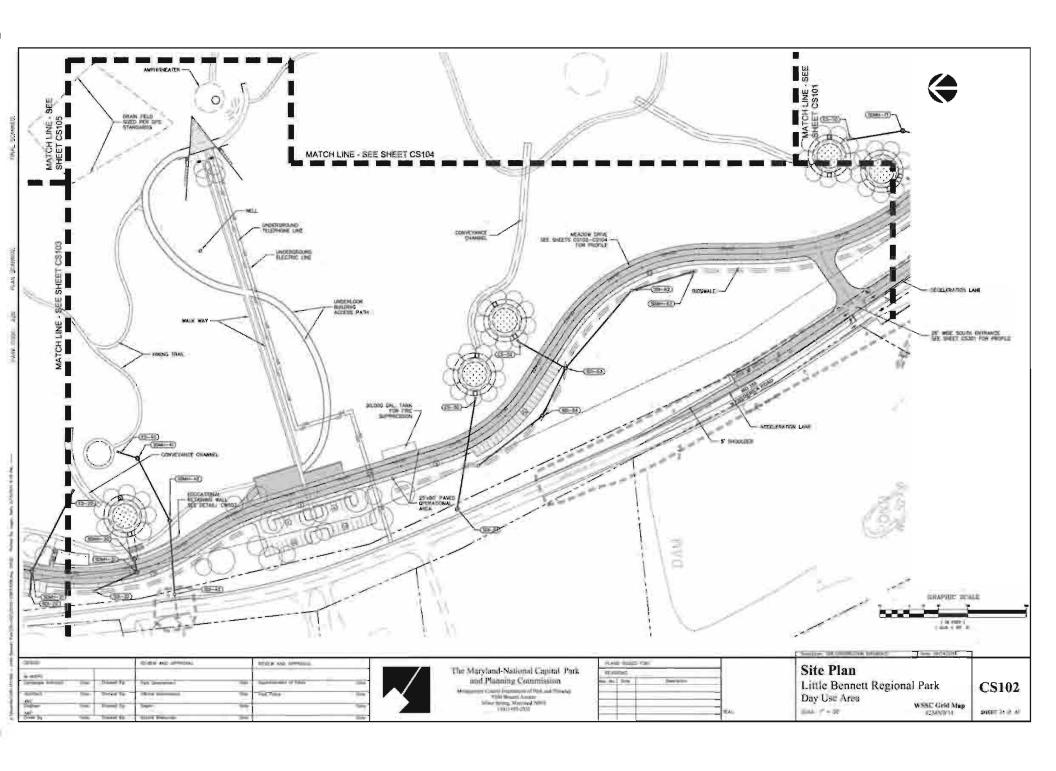


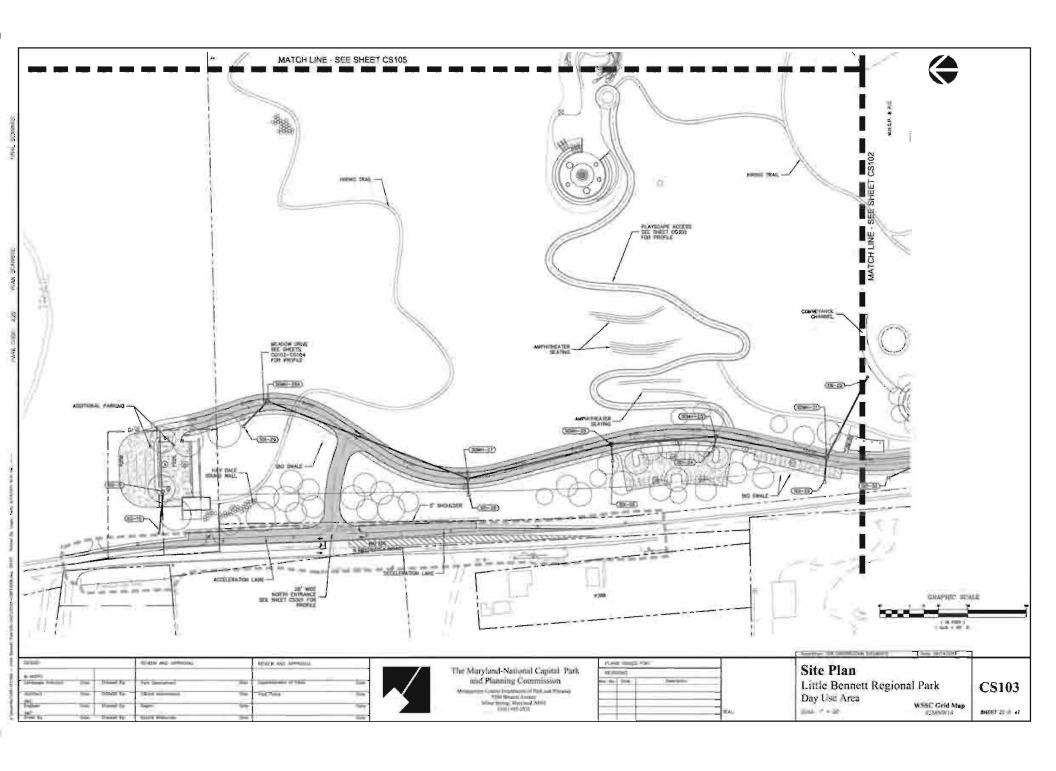


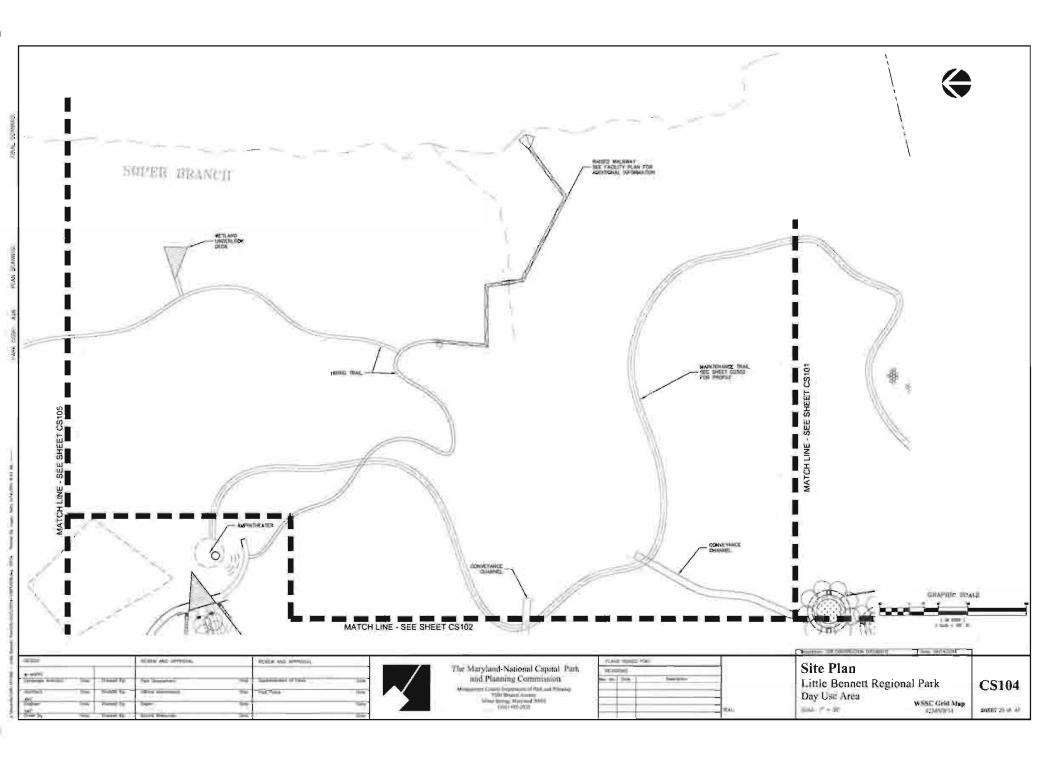


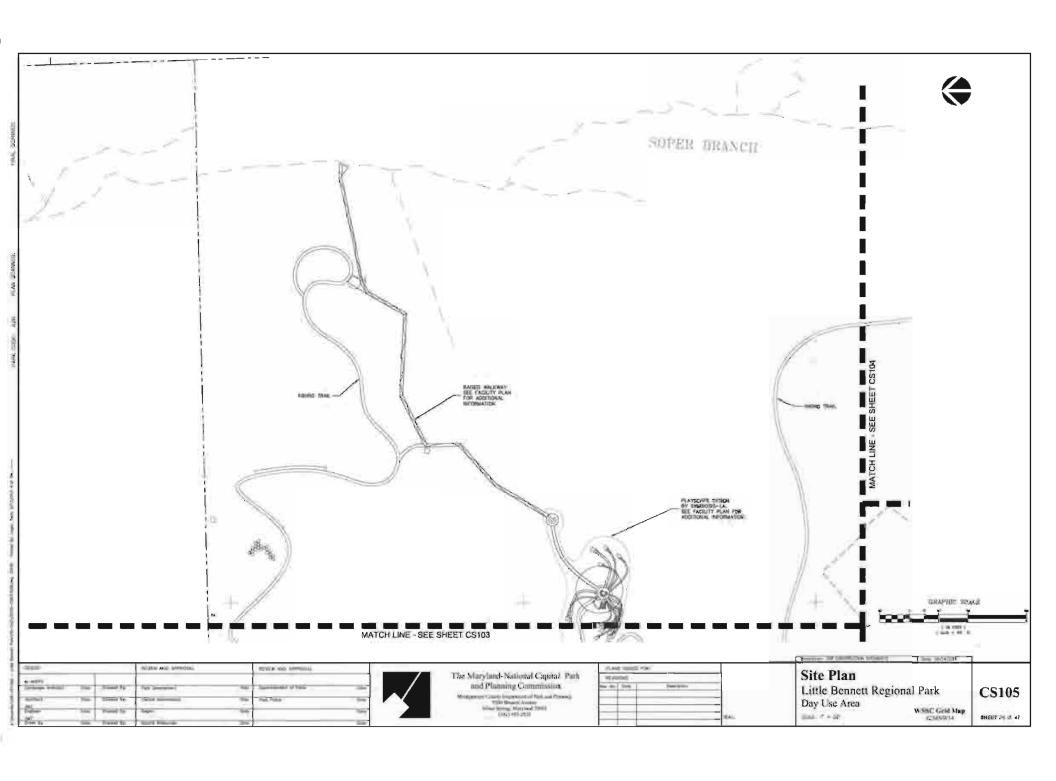


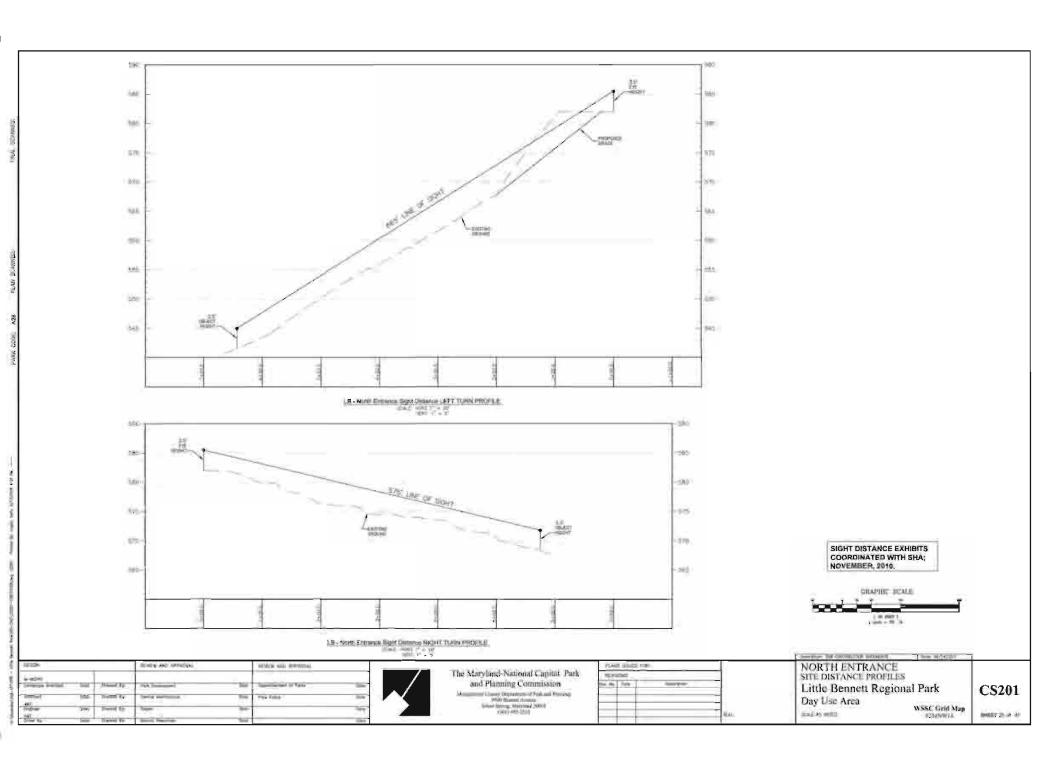


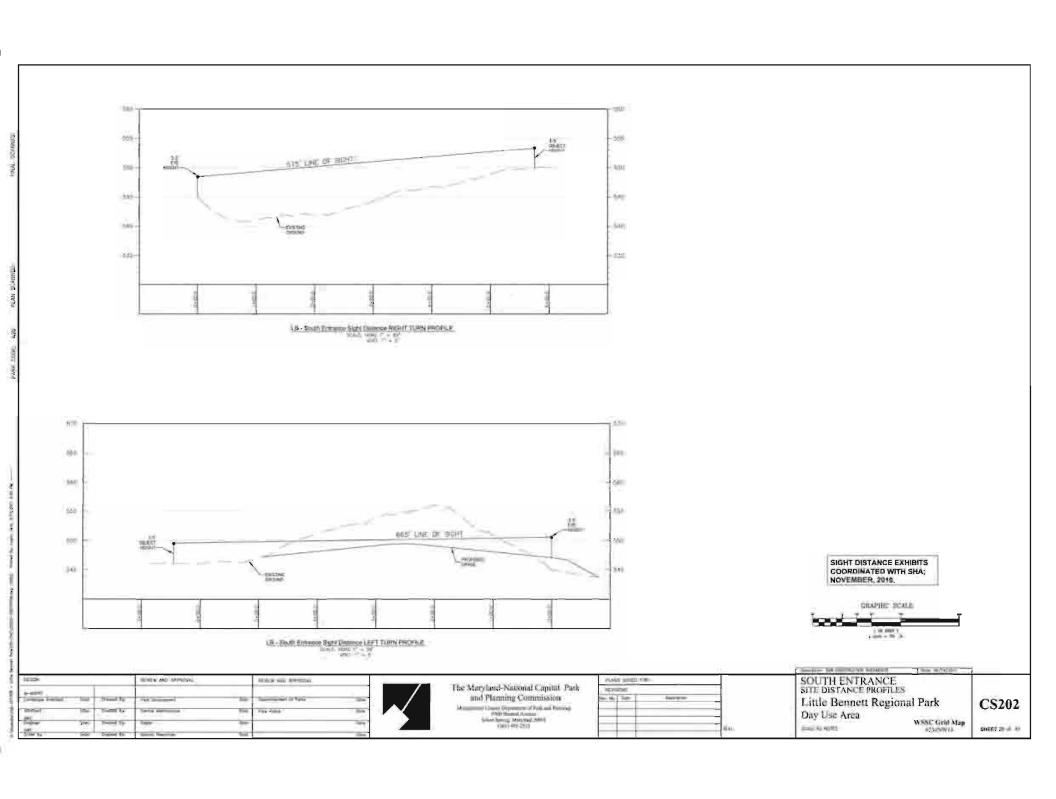


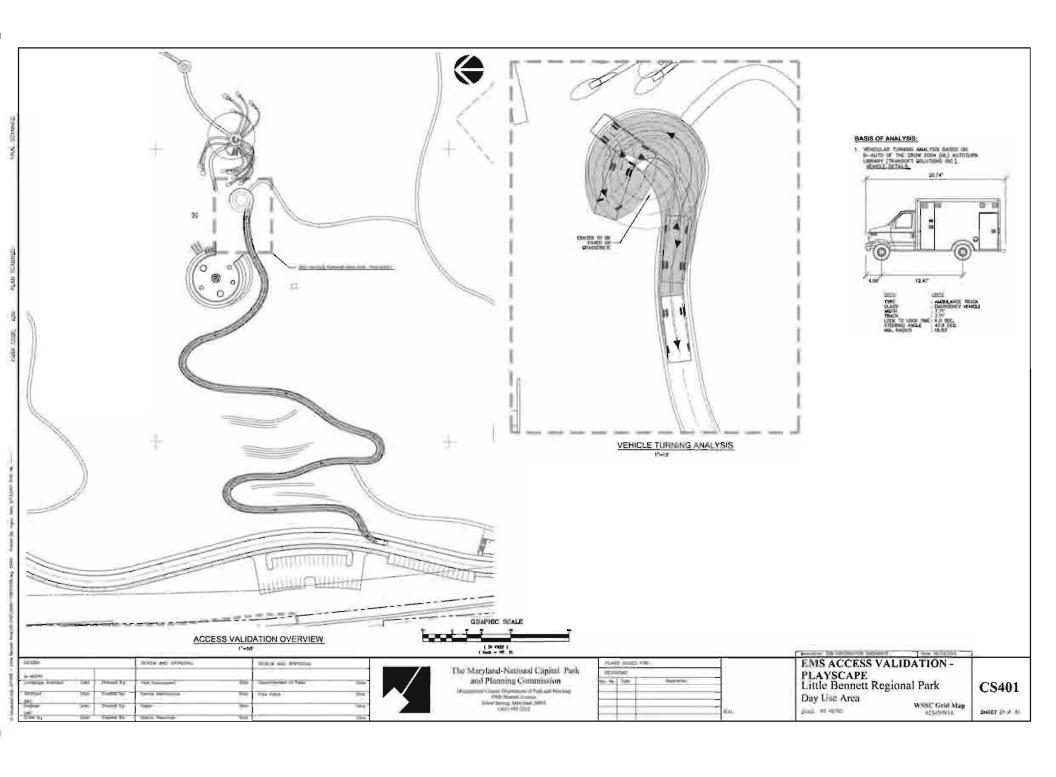


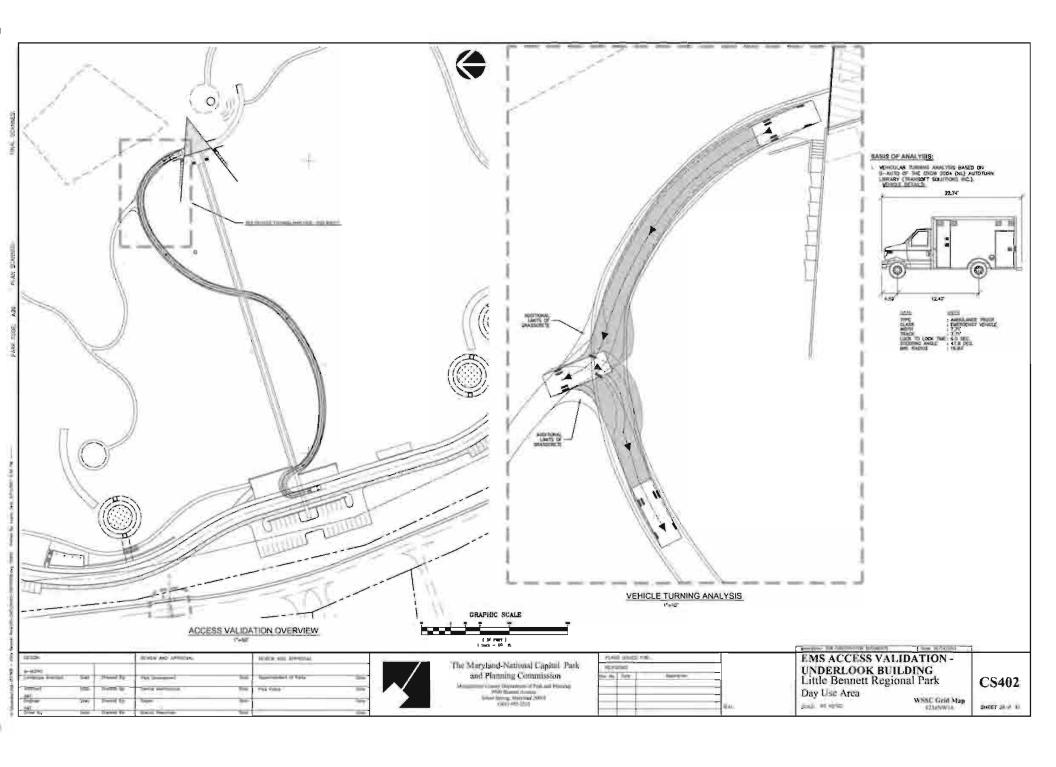


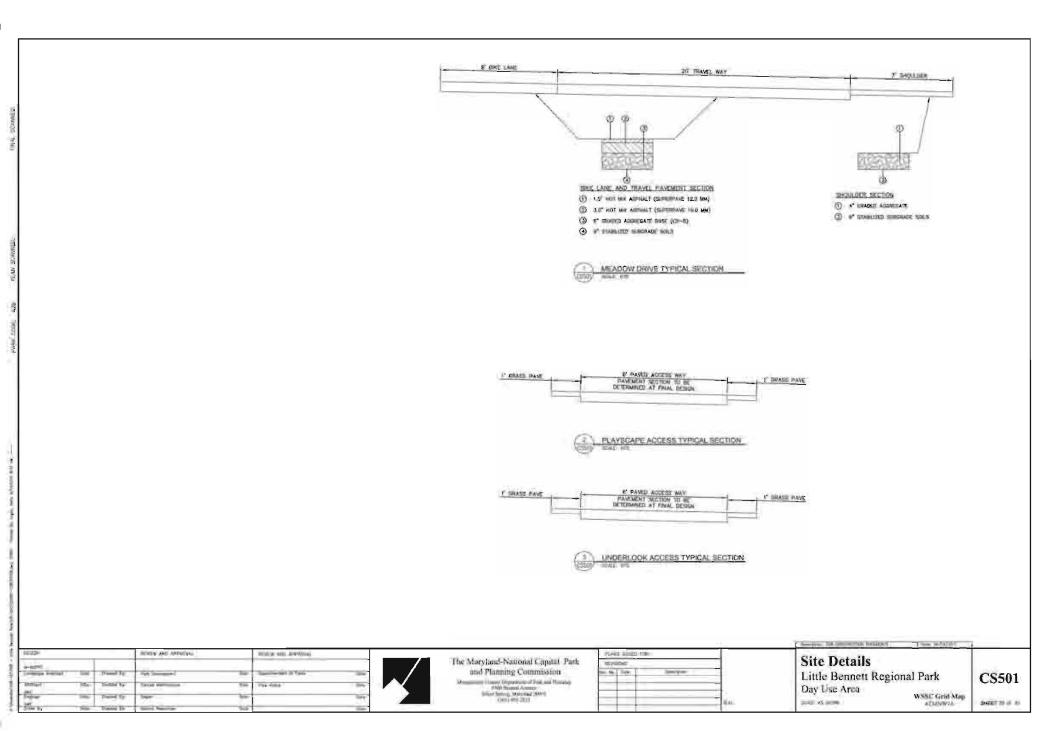


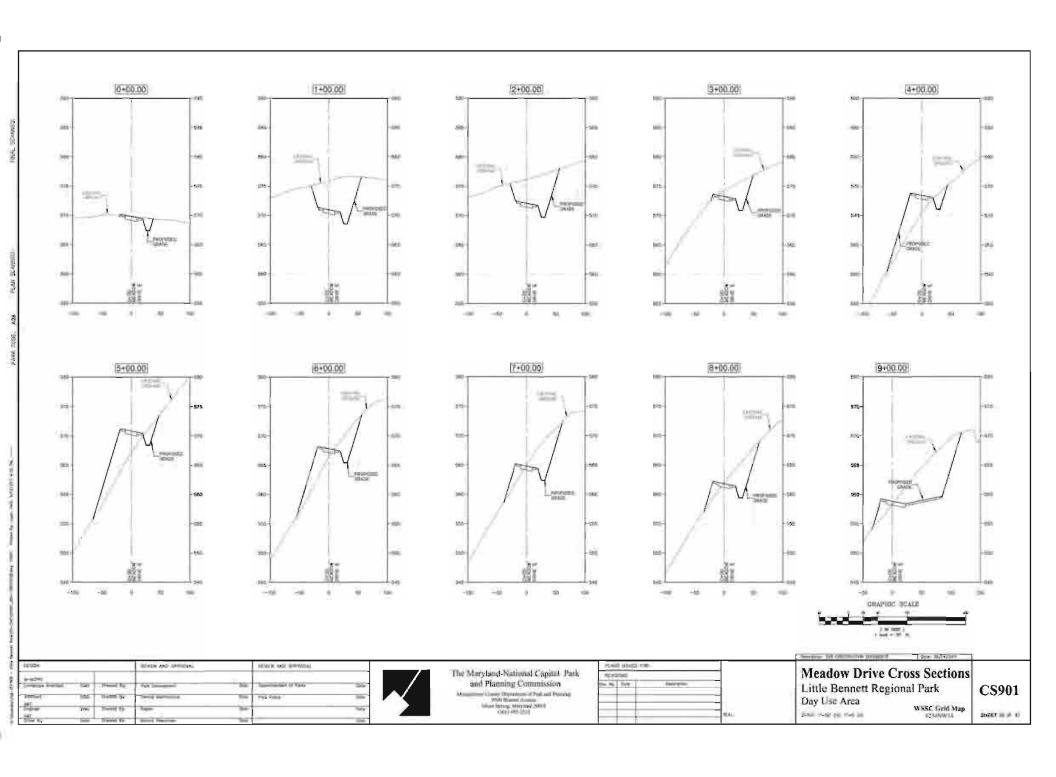


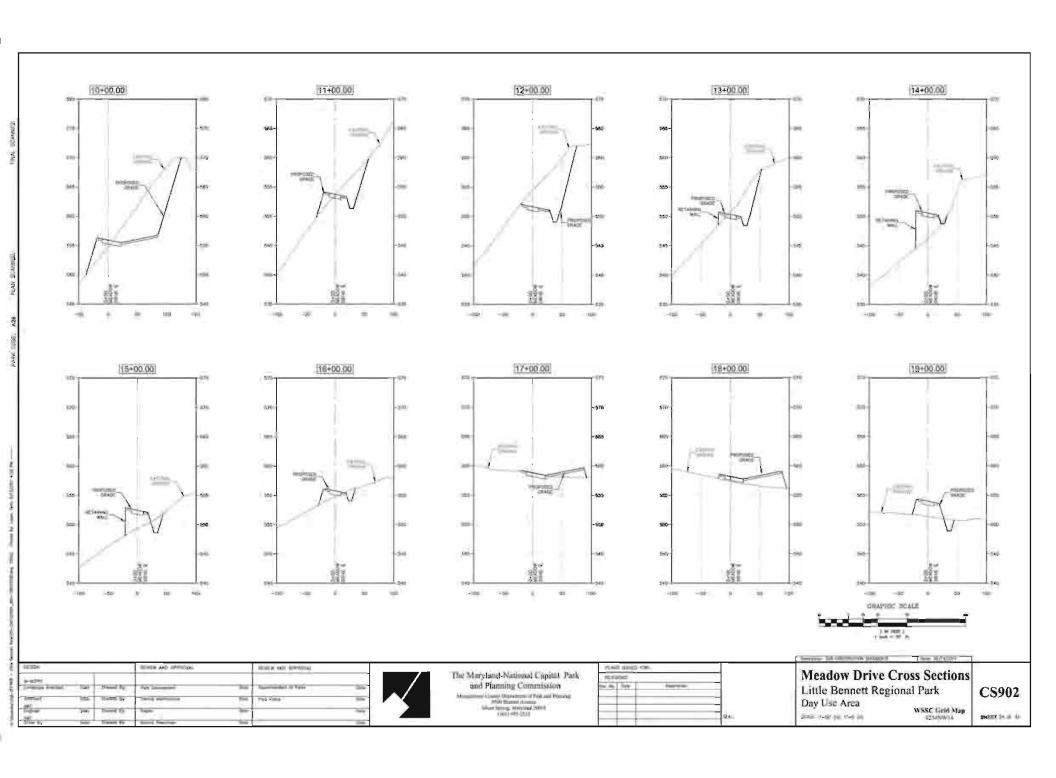


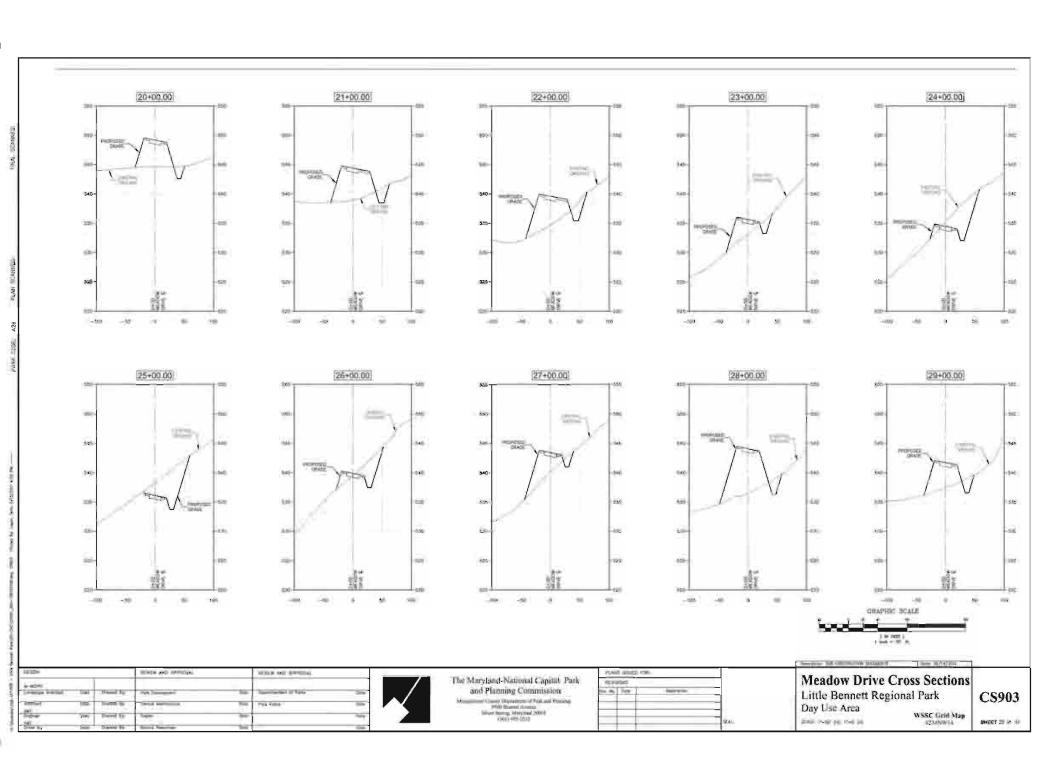


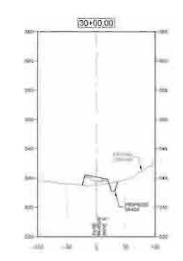


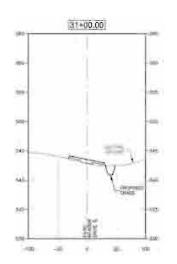


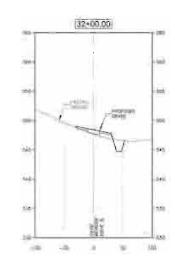














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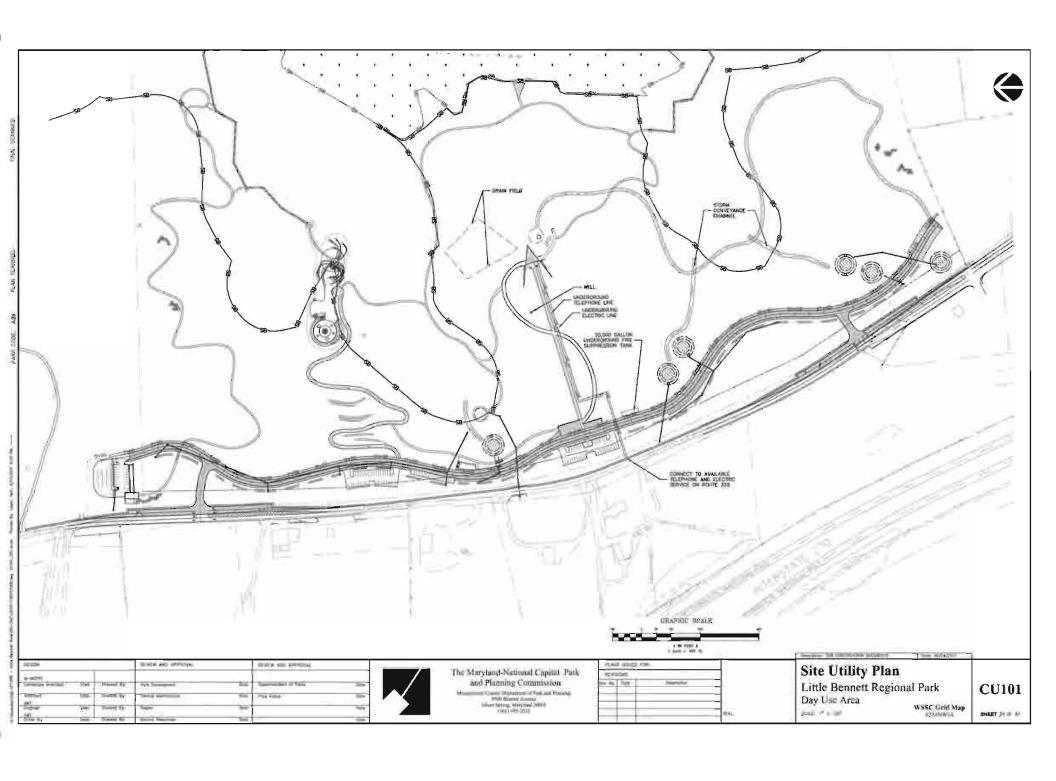
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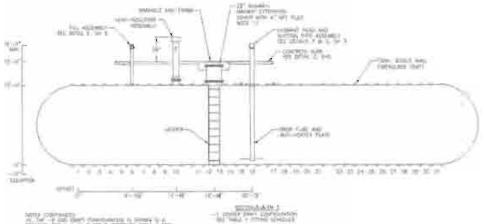
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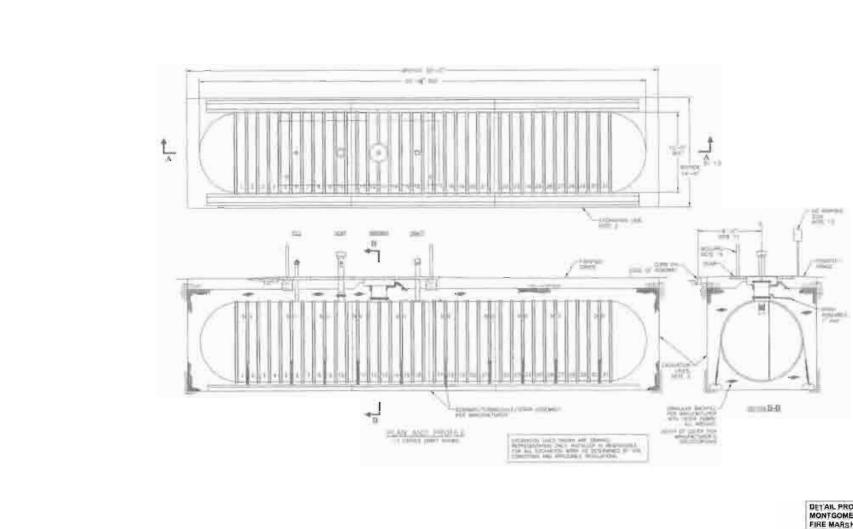


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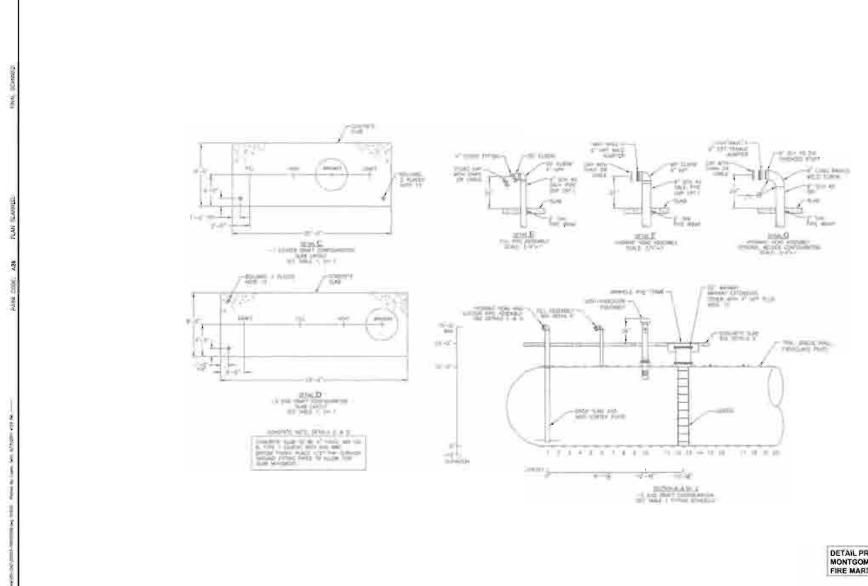
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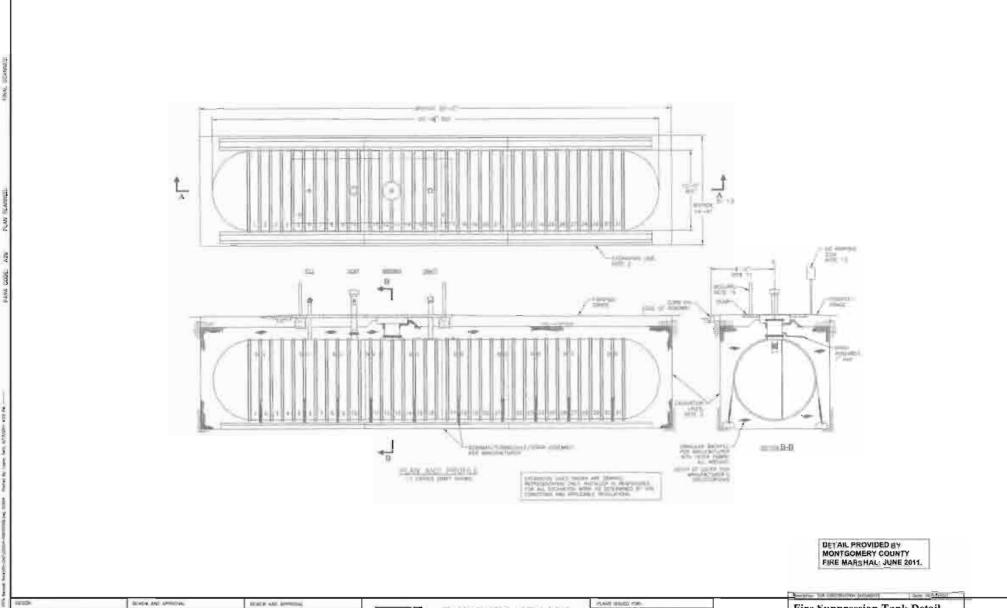
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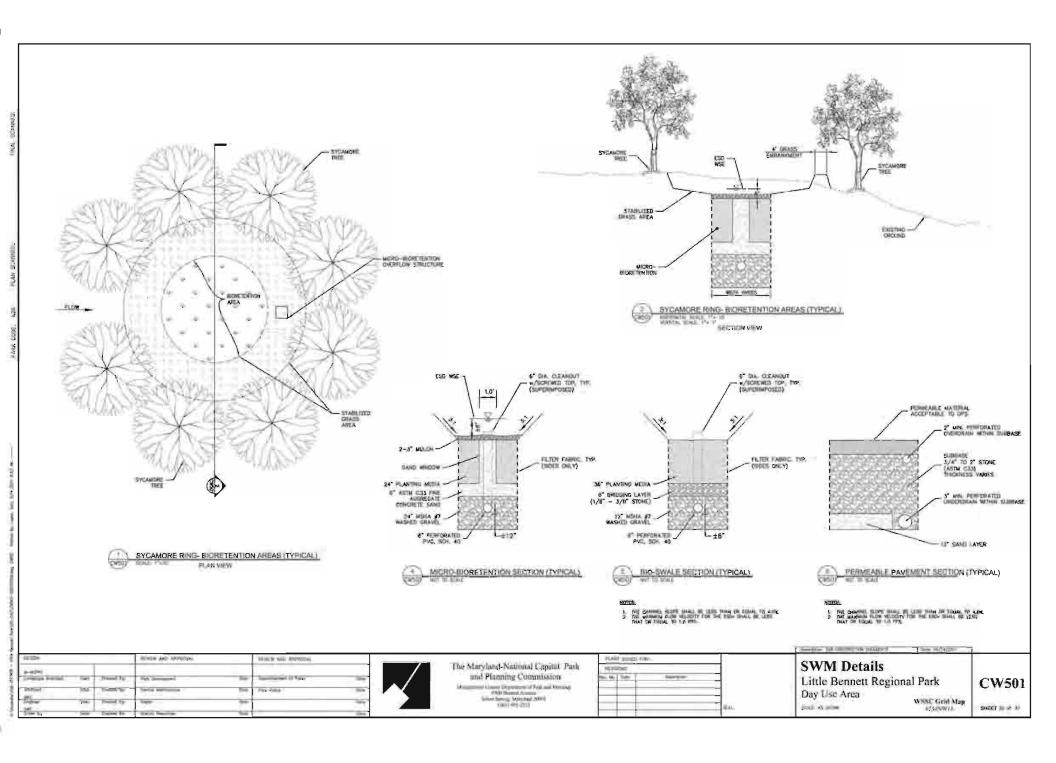
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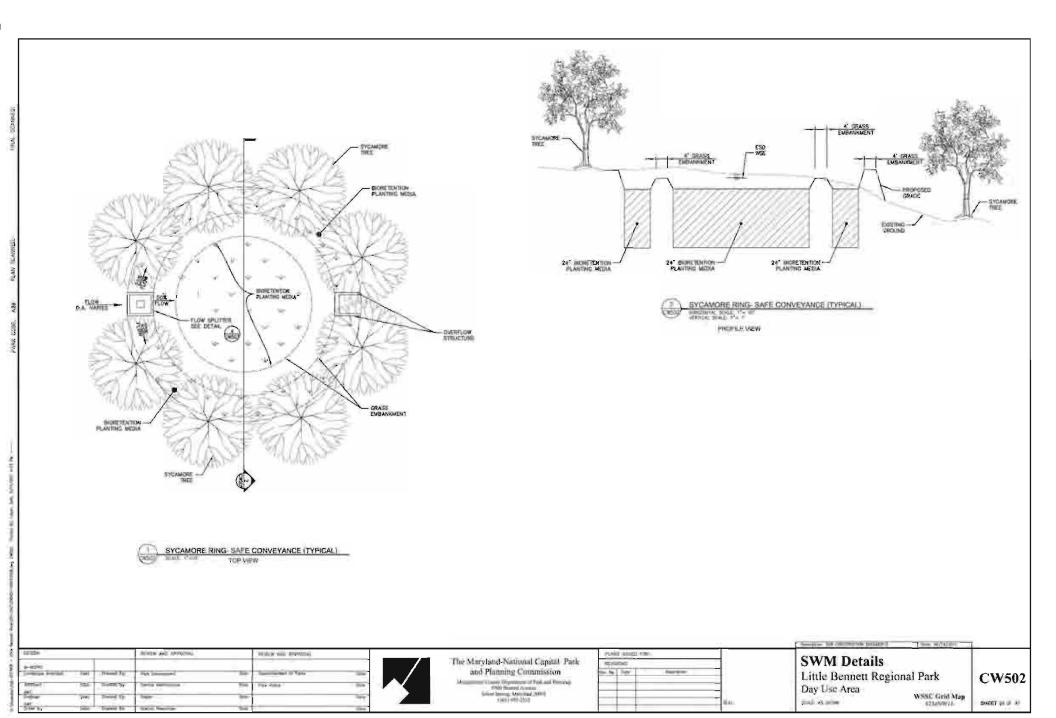
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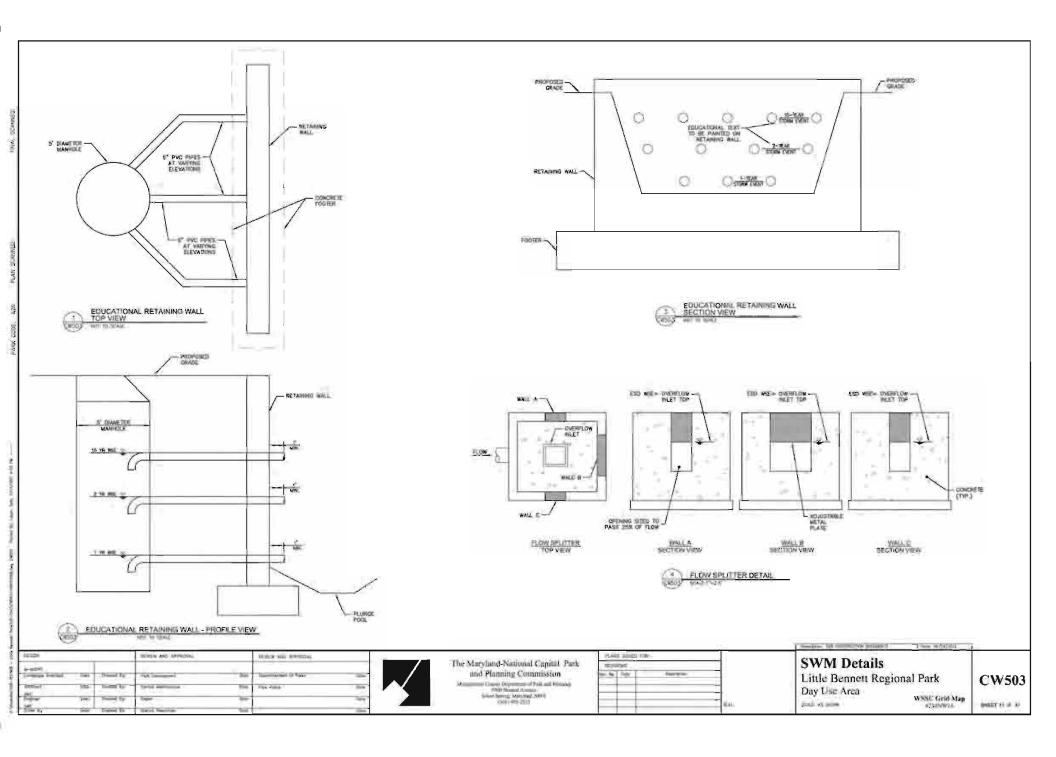
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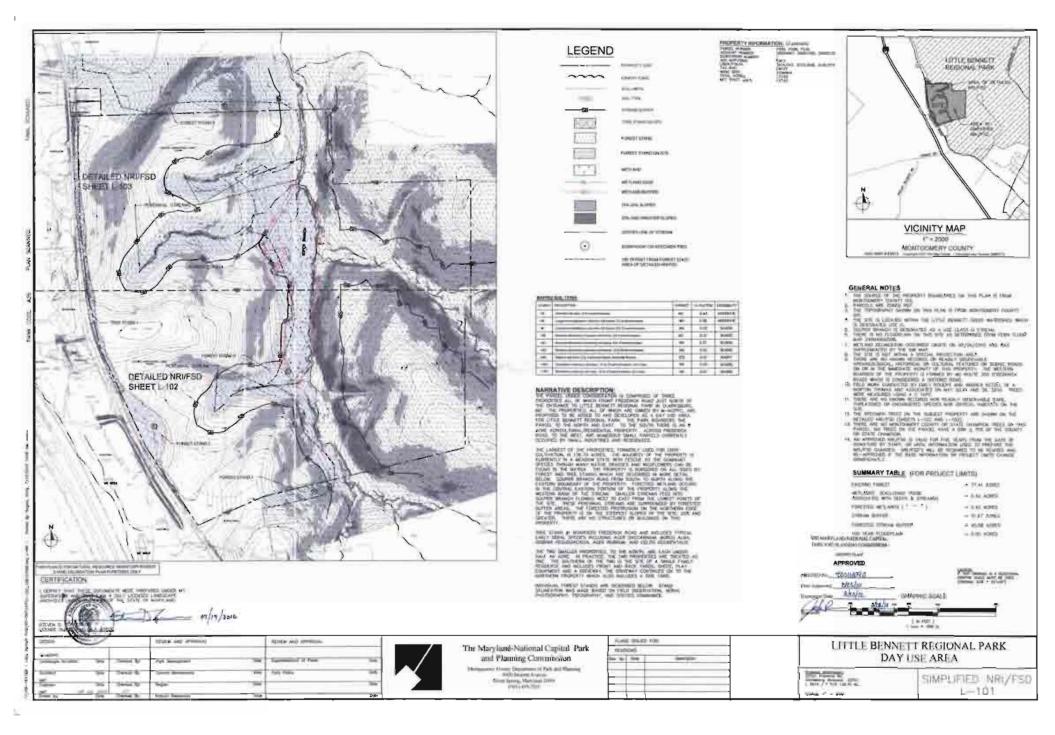
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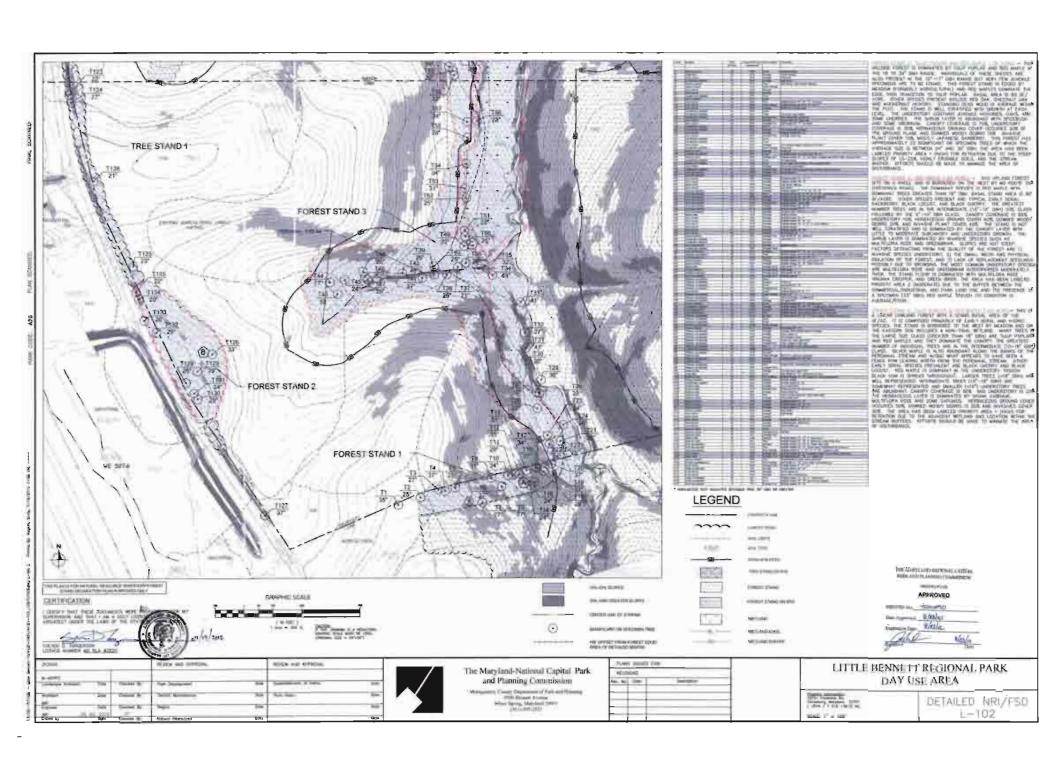
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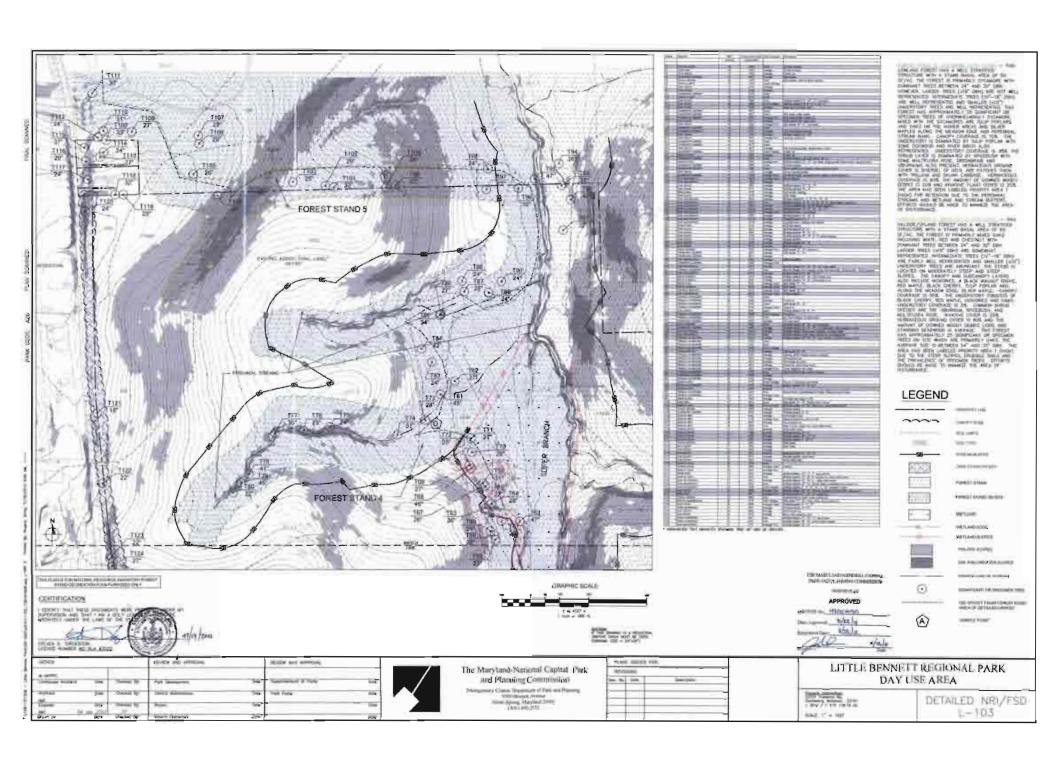


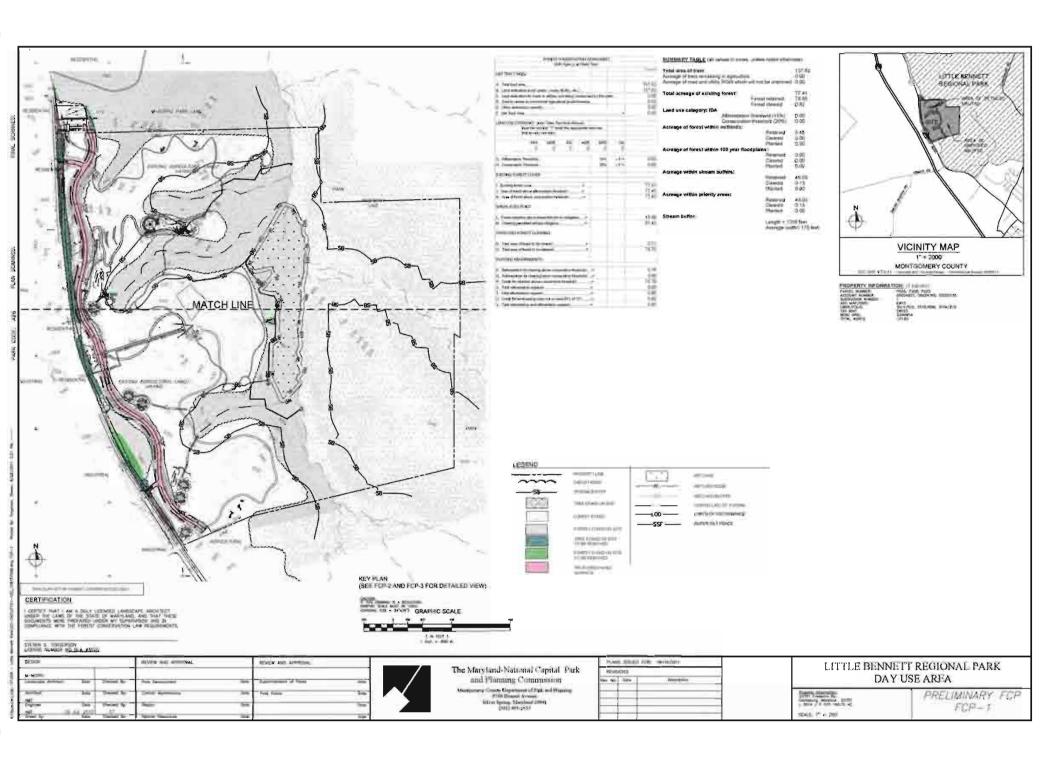


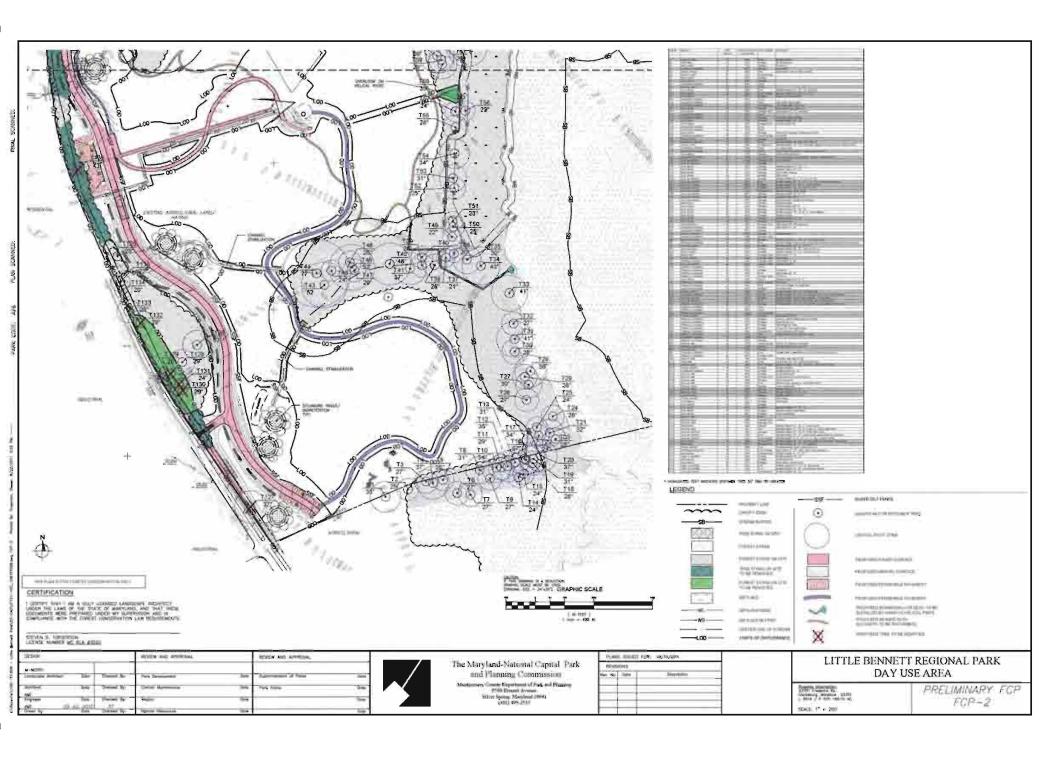


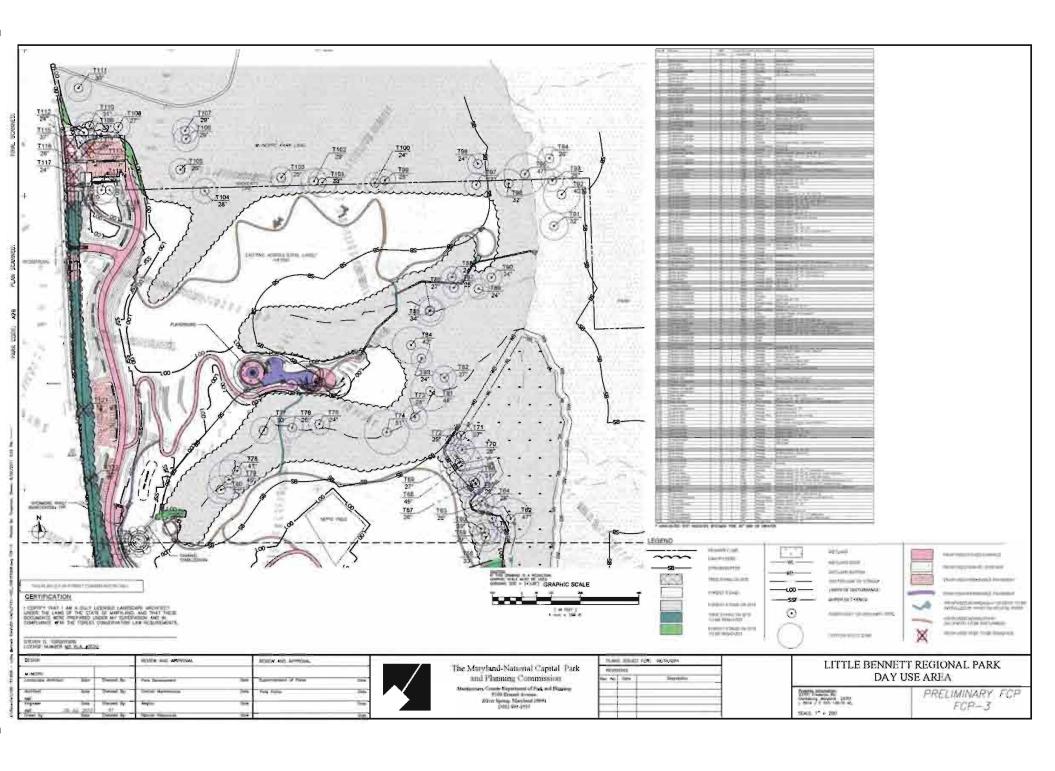




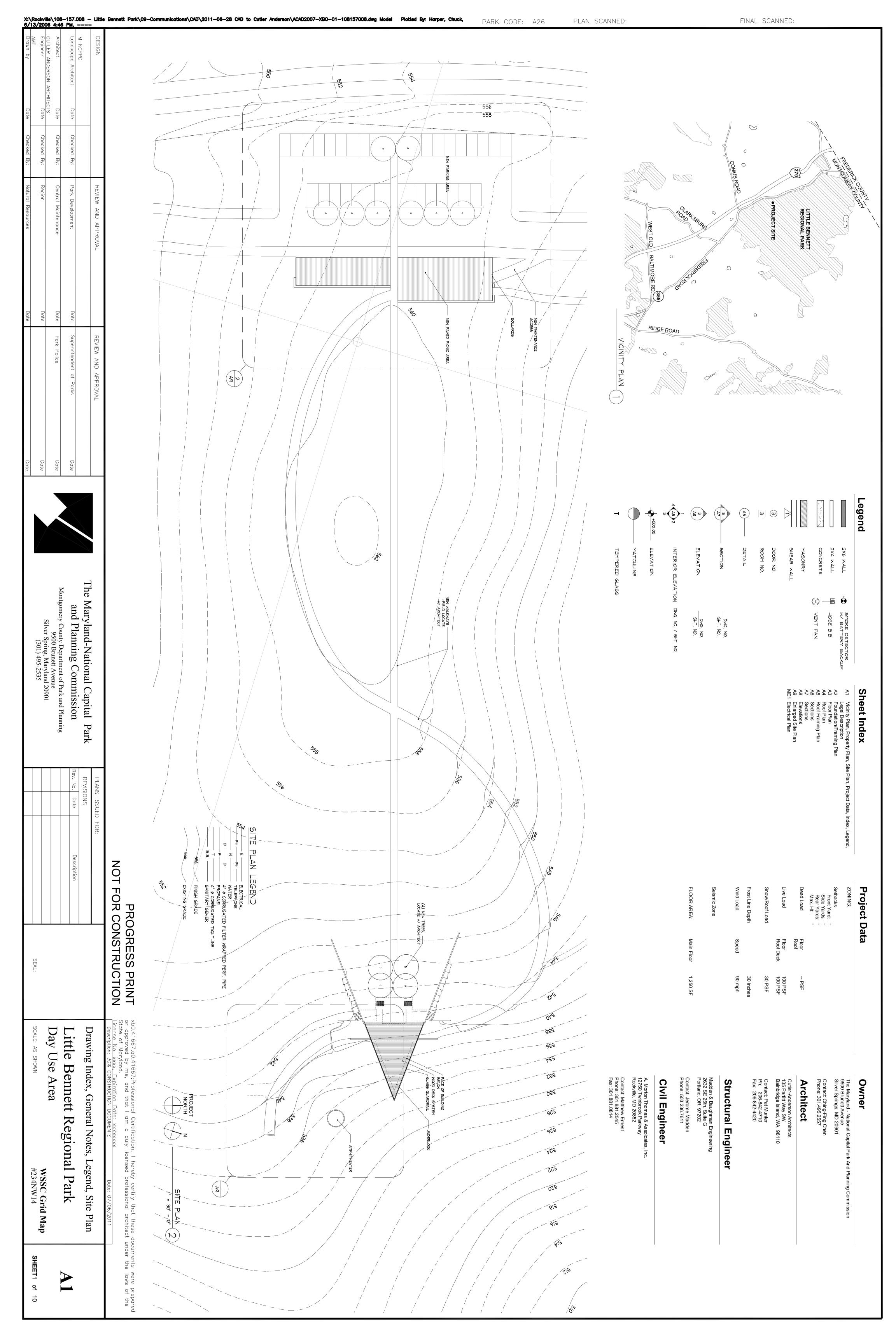


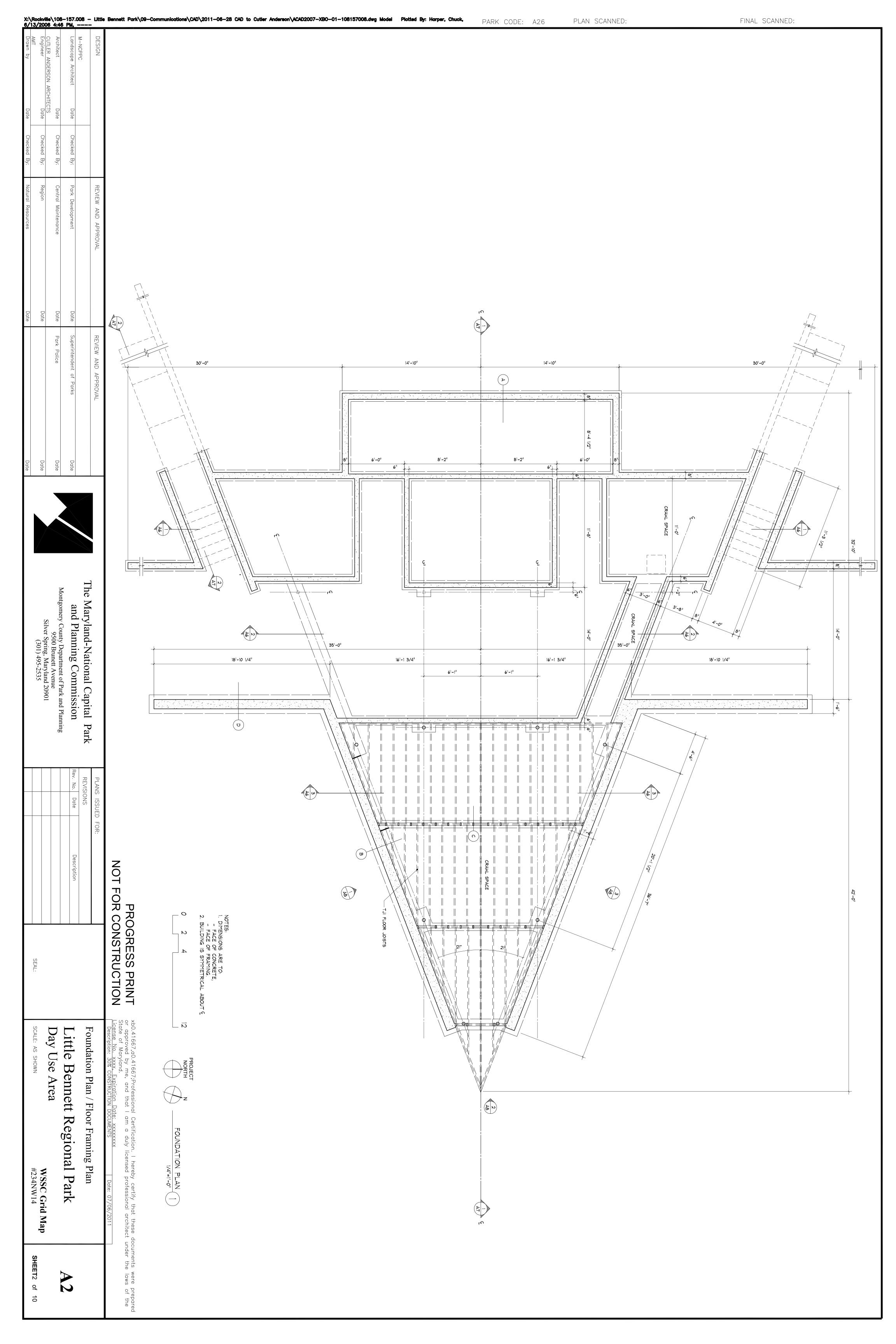


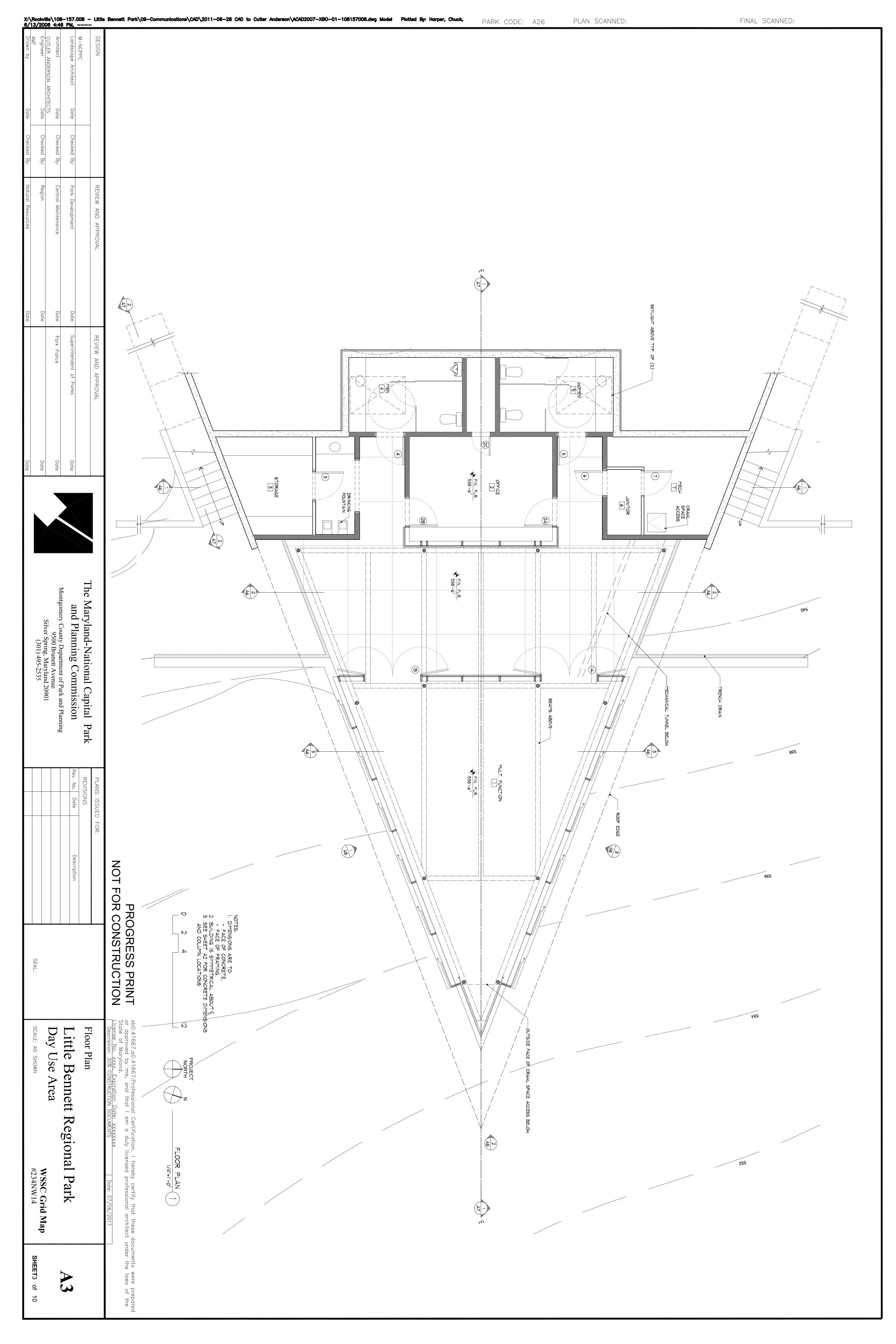


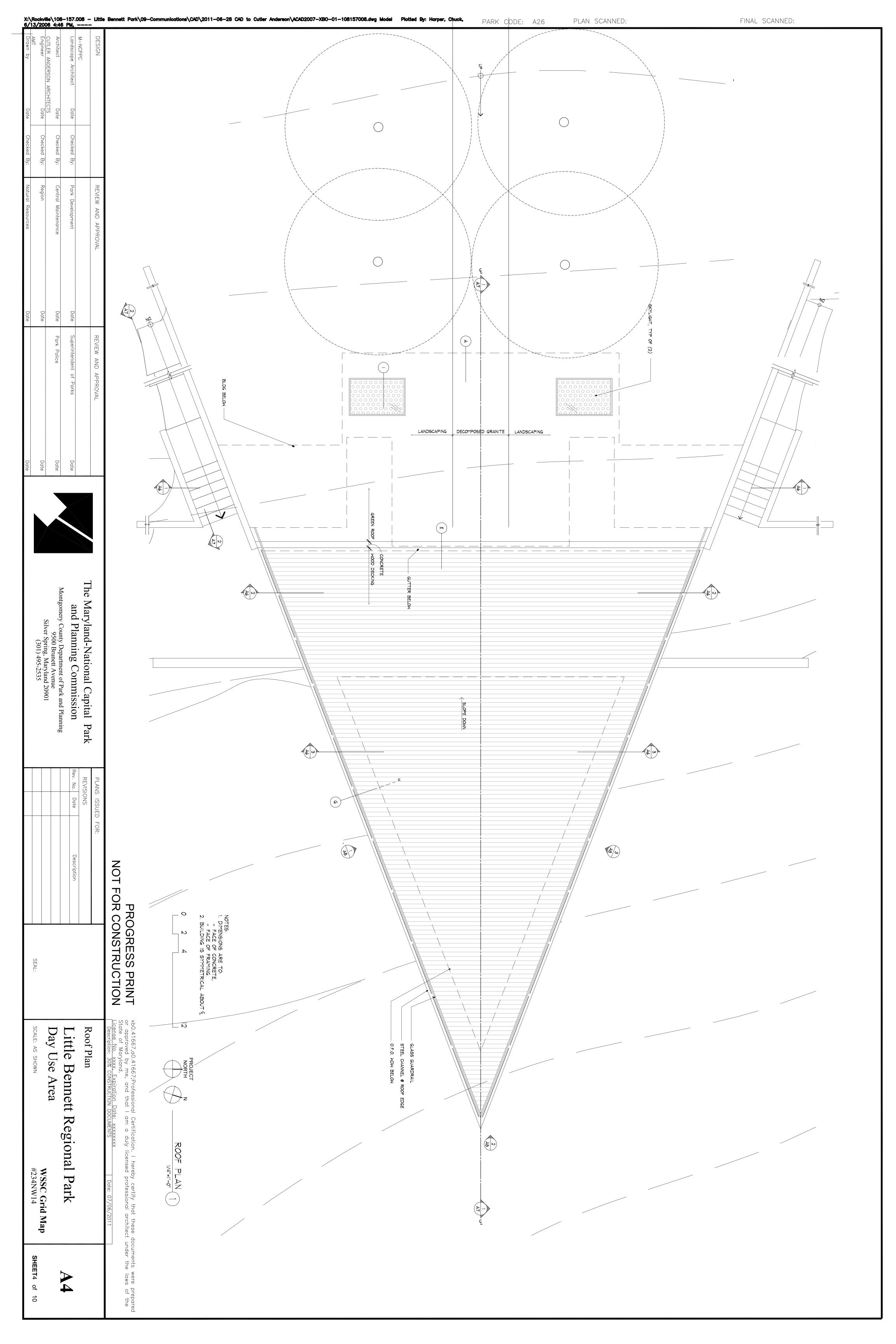


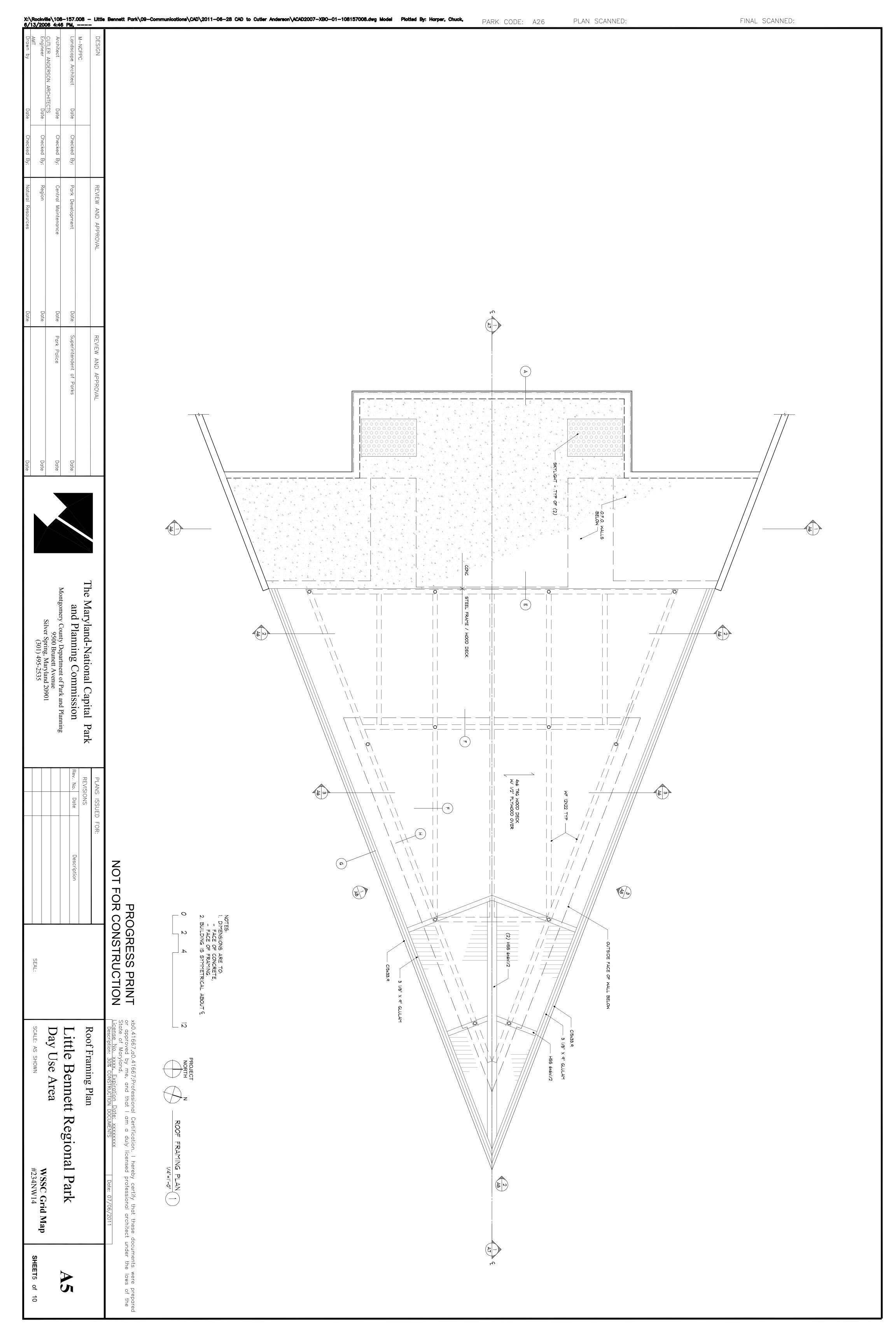
30% Construction Documents—Architecture

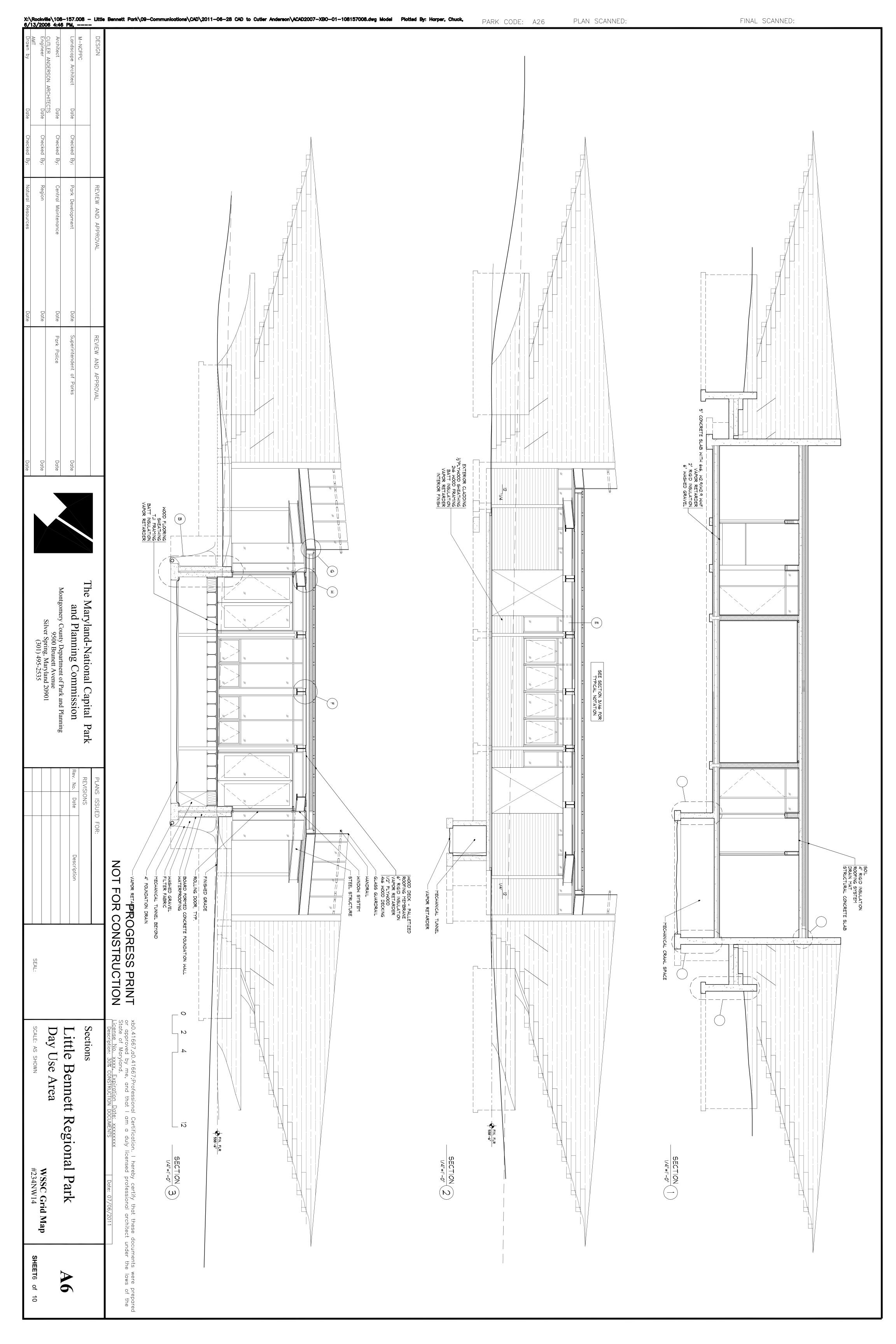


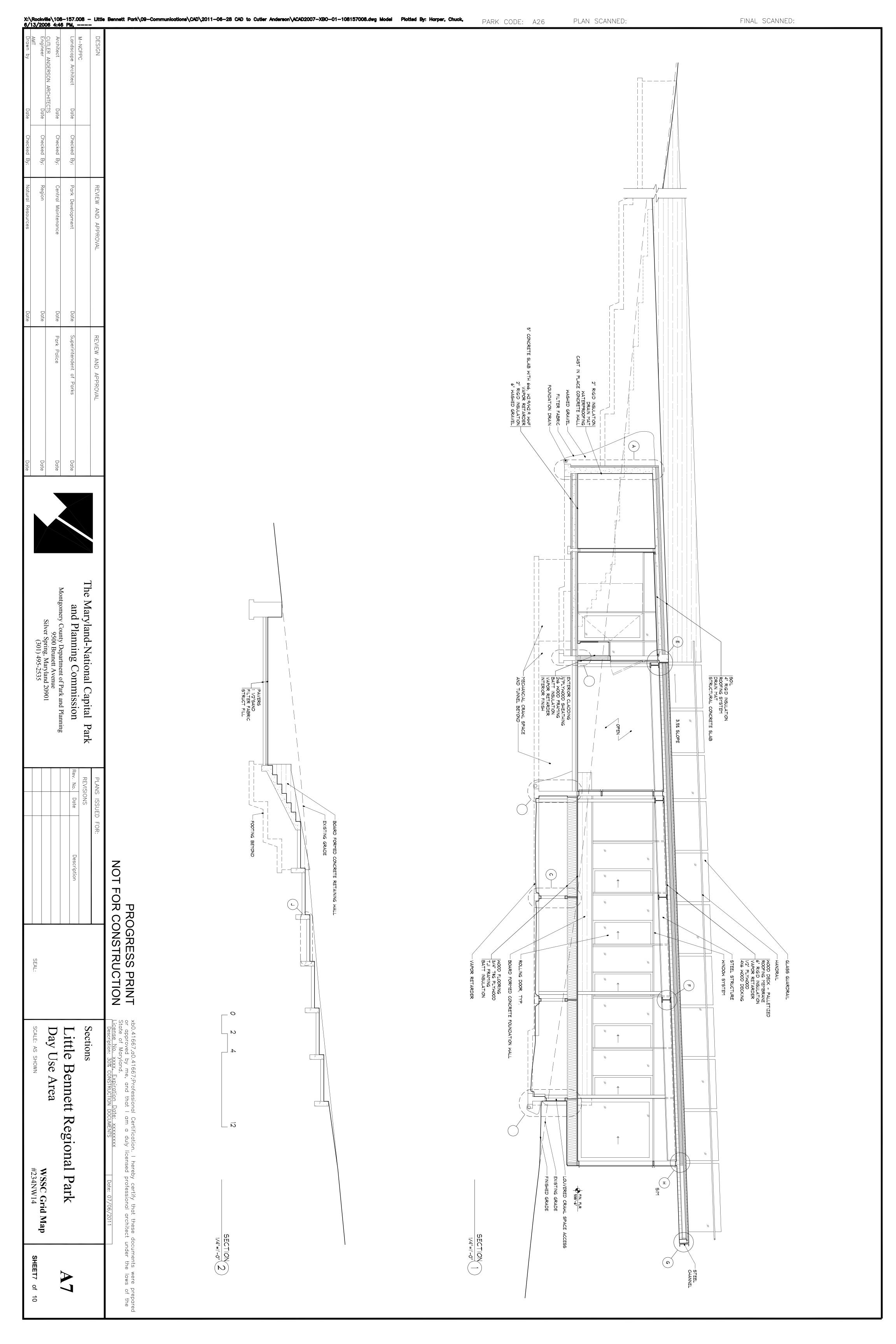


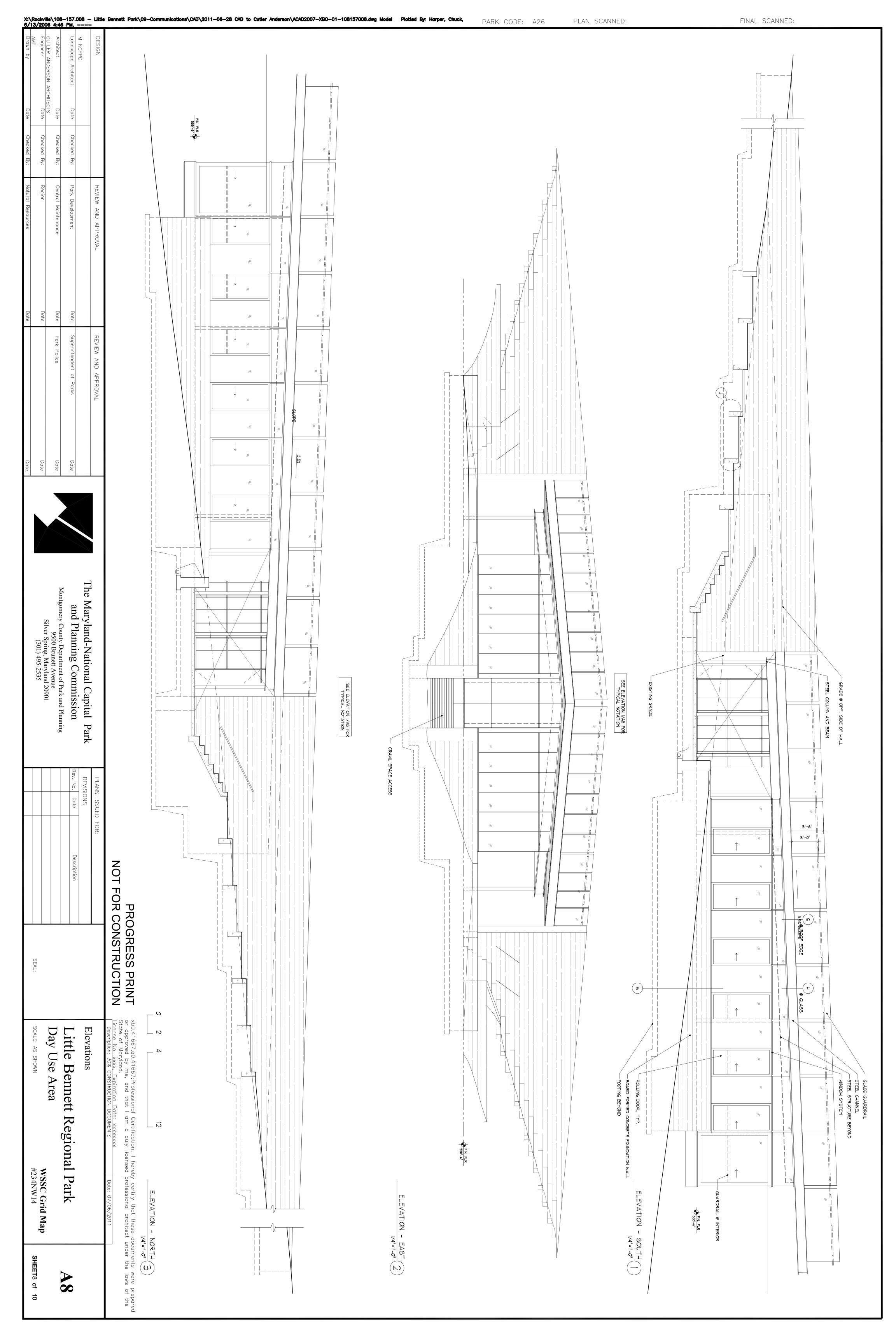


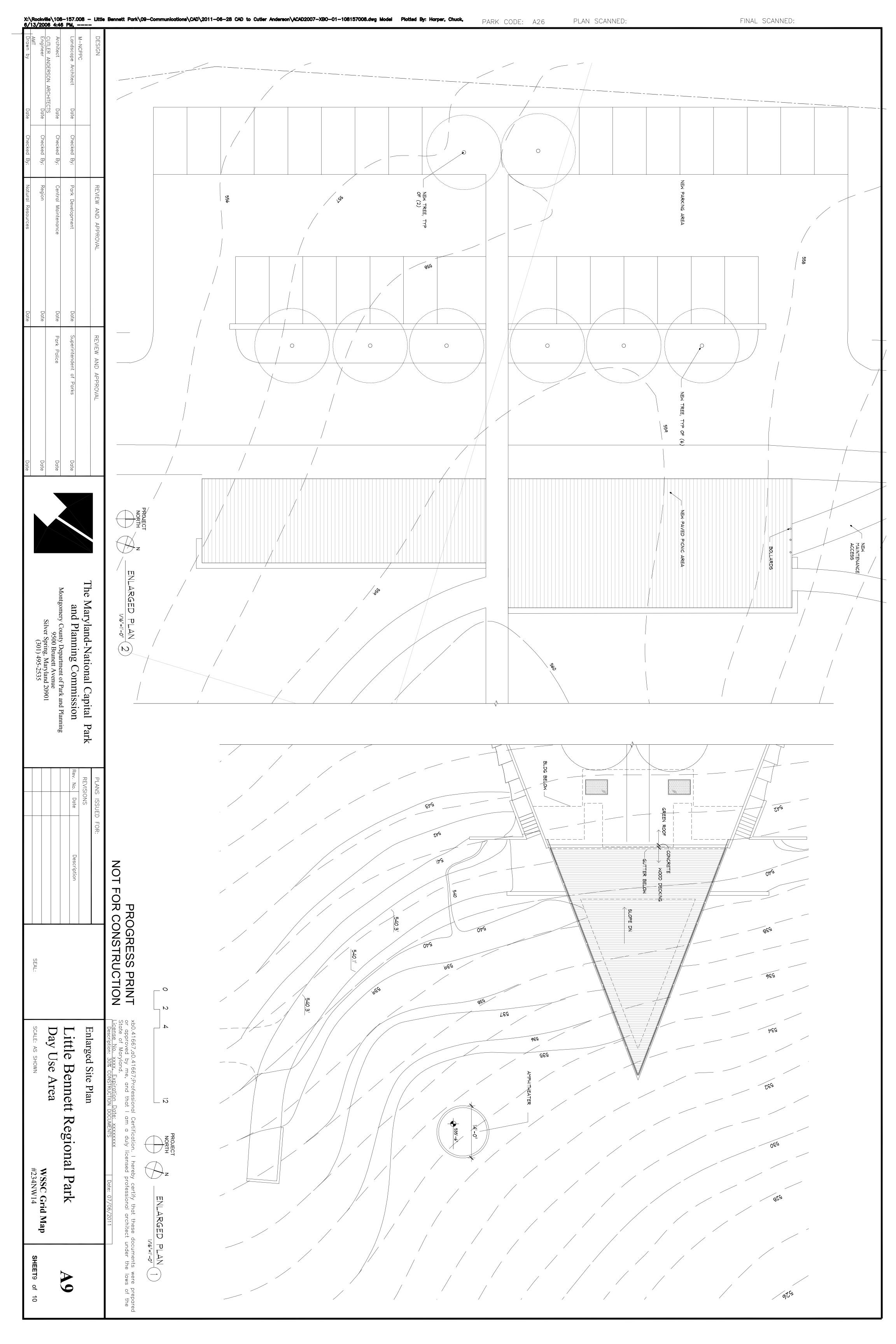


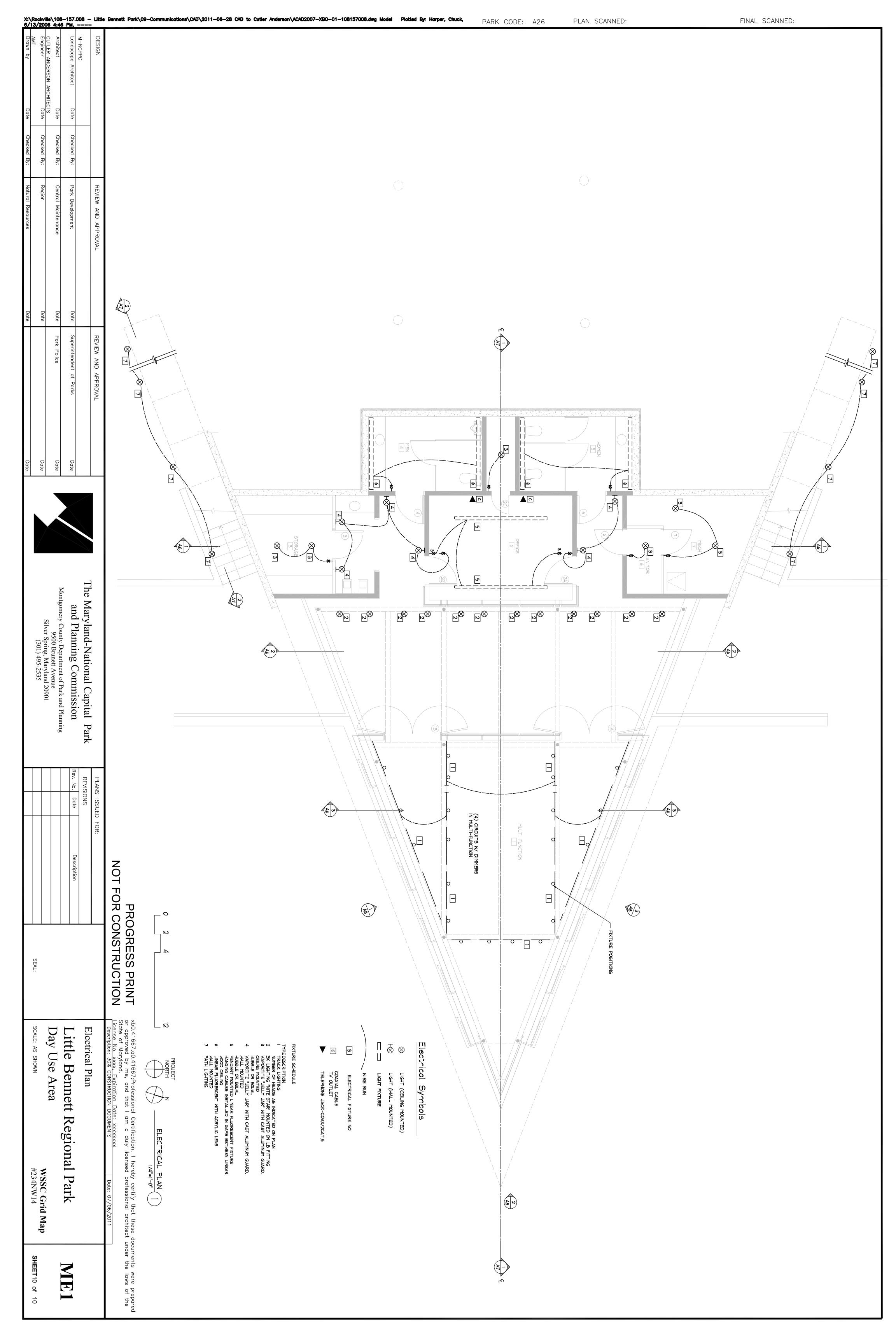




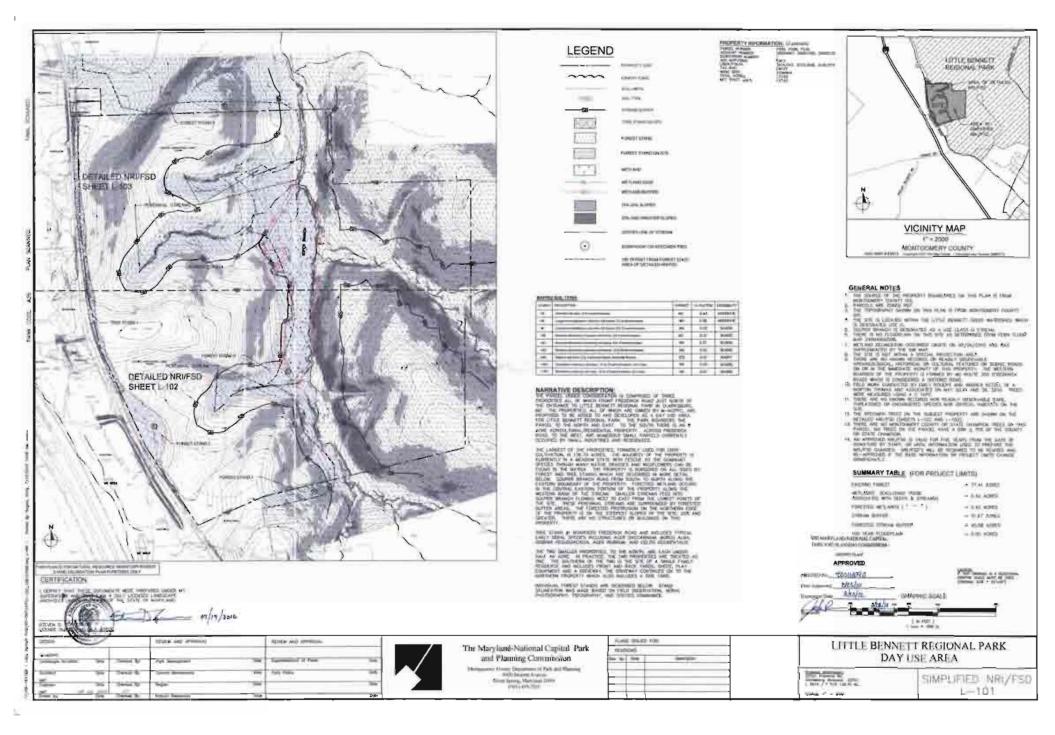


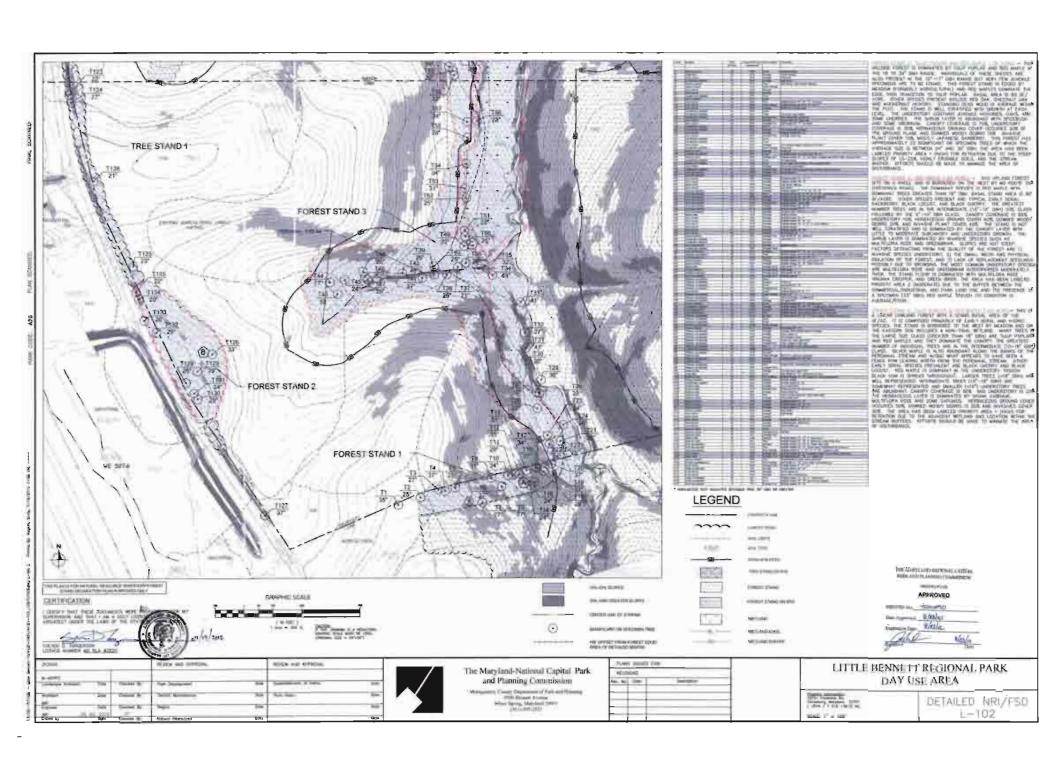


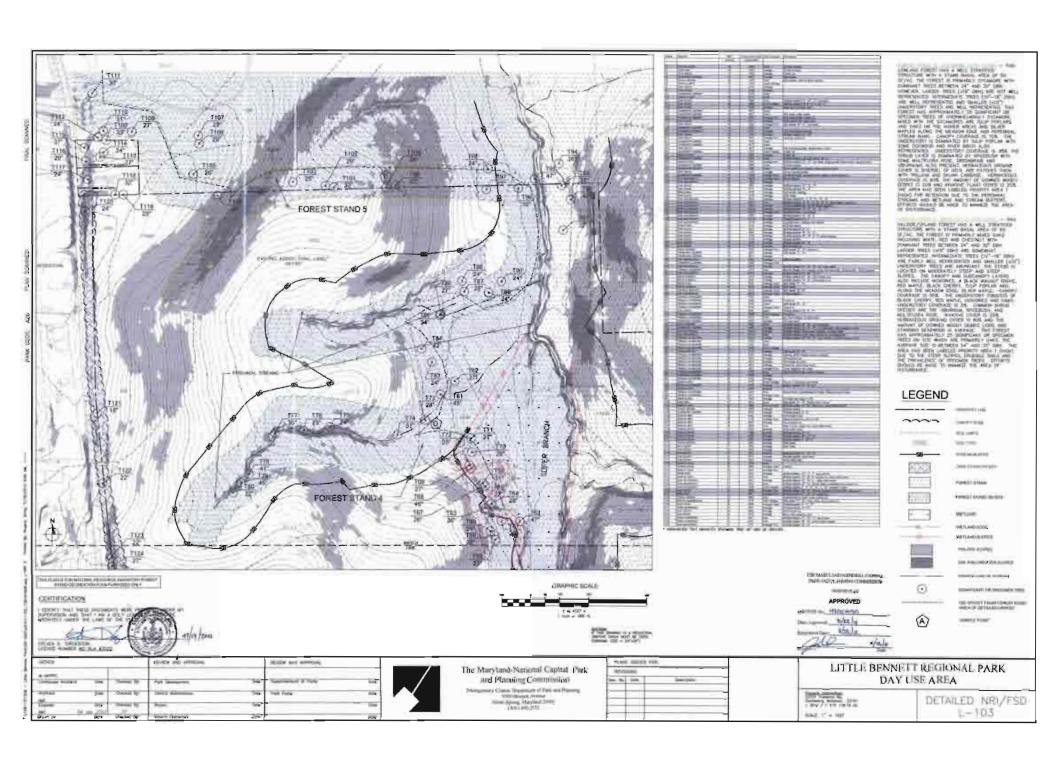




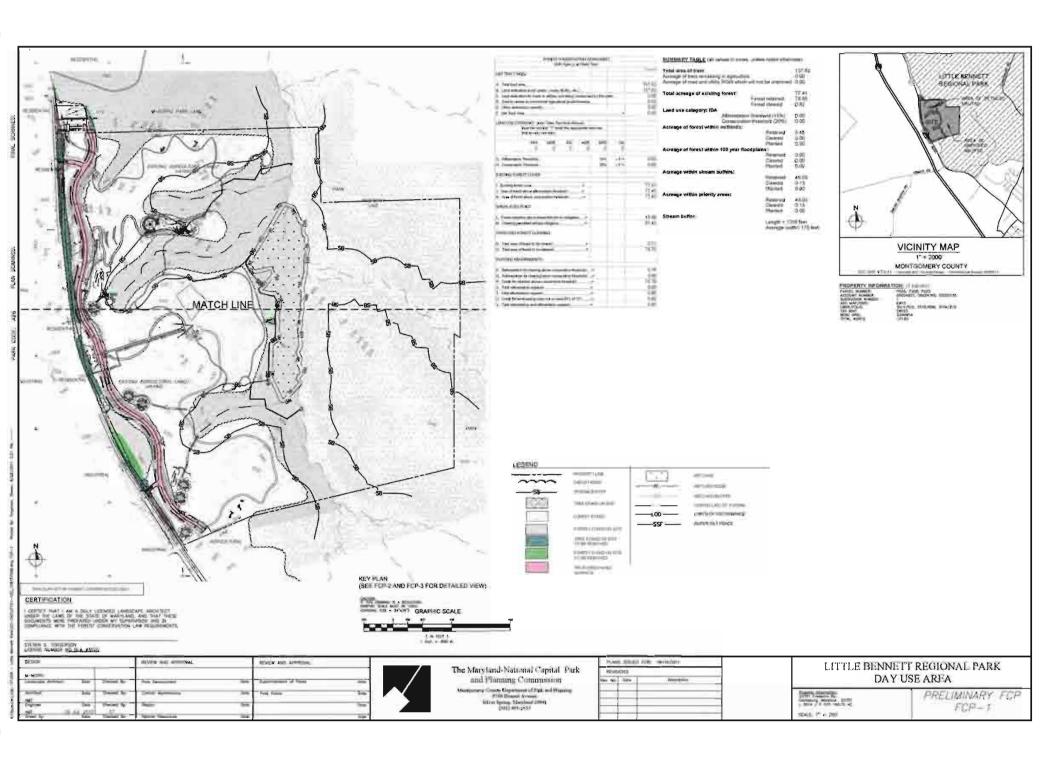
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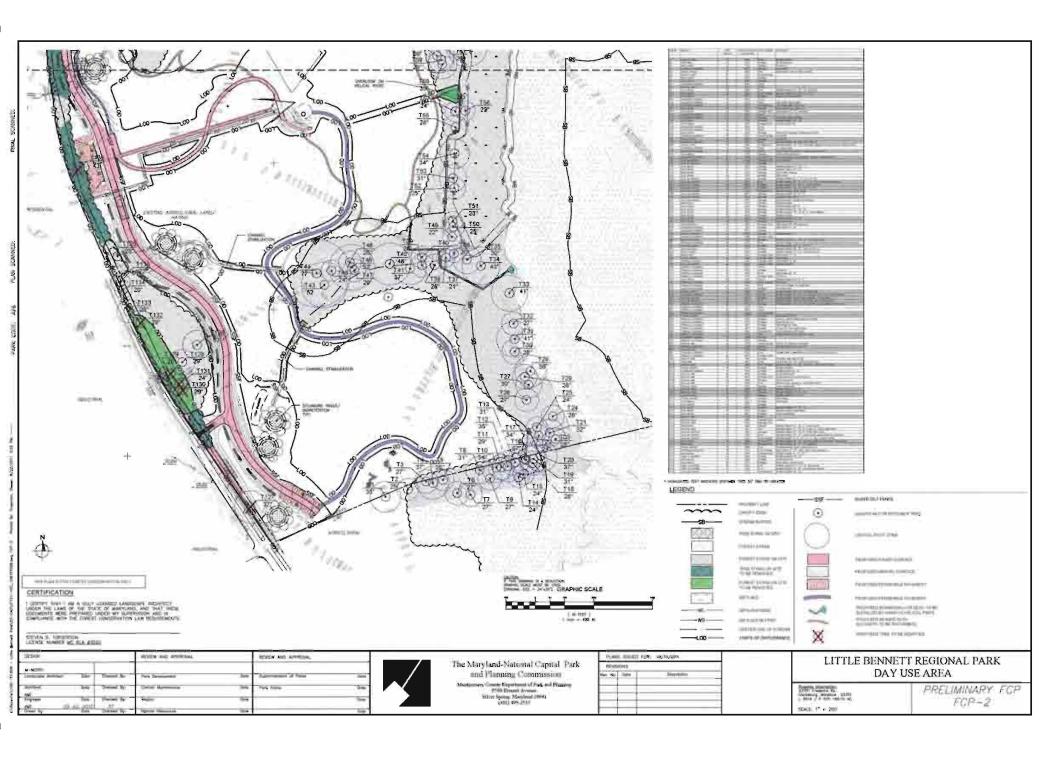


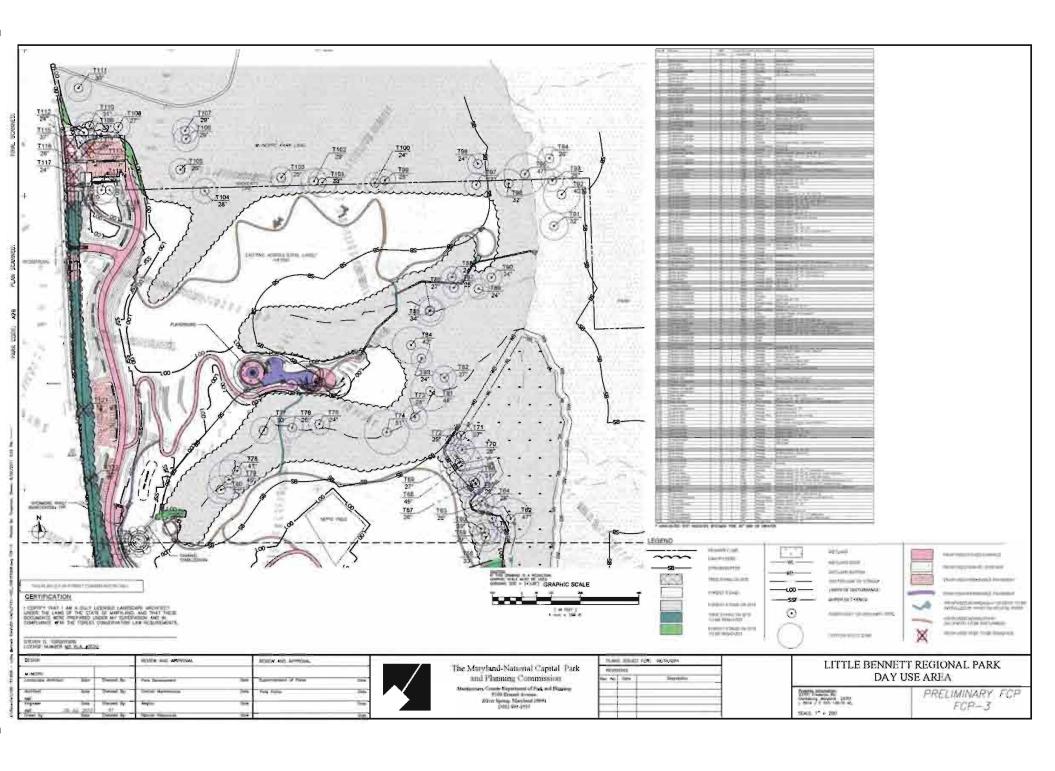




Preliminary Forest Conservation Plan







Geotechnical Report

GEOTECHNICAL REPORT

LITTLE BENNETT REGIONAL PARK MONTGOMERY COUNTY, MARYLAND

Prepared for:

A. MORTON THOMAS & ASSOCIATES, INC. 12750 Twinbrook Parkway Rockville, MD 20852

Attn: Mr. Jerry C. Kavadias, PE

Prepared by:



T.L.B. Associates, Inc. 7280 Baltimore-Annapolis Blvd. Glen Burnie, Maryland 21061

September 7, 2011



7280 BALTIMORE ANNAPOLIS BOULEVARD GLEN BURNIE, MARYLAND 21061 VOICE: 443-577-1600 / 301-621-0990

FAX: 443-577-1601 / 301-621-9737

www.tlbinc.net

Subsurface Explorations & Geotechnical Engineering Consultants THOMAS L. BROWN, CWD, P.E., President

September 7, 2011

A. Morton Thomas & Associates, Inc. 12750 Twinbrook Parkway Rockville, MD 20852

Attn: Mr. Jerry C. Kavadias, PE

Vice President/Principal

RE: Little Bennett Regional Park

Proposed Day Use Area

Clarksburg, Montgomery County, MD

TLB Reference No. 10-030-A

Dear Mr. Kavadias,

Pursuant to your request, we have performed a geotechnical study in support of your design efforts on the referenced project. The following report summarizes the results of our subsurface explorations and laboratory testing and presents geotechnical recommendations for the planned park renovations to include roadways, aesthetic structures and stormwater management facilities.

If you have any questions regarding this report or when we can be of further assistance on this or

other projects, please do not hesitate to call us.

Yours very truly,

T. L. B. ASSOCIATES, INC.

Khadija Ngozi-Bullock

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APPENDIX B -- Laboratory Test Results

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1.0 PROJECT DESCRIPTION

The Maryland-National Capital Park and Planning Commission (M-NCPPC), and the Park Development Division of the Montgomery County Department of Parks is planning to renovate the Little Bennett Regional Park in Clarksburg, Maryland. A. Morton Thomas and Associates, Inc. (AMT) of Rockville, MD has been requested to perform engineering services for the project. AMT engaged T.L.B. Associates, Inc. (TLB) of Glen Burnie, Maryland to provide geotechnical input in support of the design and construction.

1.1 Existing Conditions

Little Bennett Regional Park is a 3,700-acre park which is bisected by Little Bennett Creek in Clarksburg, Maryland, Montgomery County, Maryland. Little Bennett Park is the largest park in the county with 11 historic sites, a 91-site campground, an 18-hole golf course and driving range, and over 23 miles of natural surface trails. The Site Location is indicated in the site location map denoted as Drawing No. 1. The park is bounded by North Frederick Road (MD Rte 355) to the west, Lewisdale Road to the north, and Burnt Hill Road to the east. Beyond the adjacent roadways, the park is surrounded by commercial properties to the west, and farmland and residential properties to the north, west and south.

The portion of the park that will be renovated as part of this project scope is located adjacent to North Frederick Road and consists of fields previously used as farmland, now overgrown with grass, intermittently bisected by wooded areas.

1.2 Existing Grades and Proposed Grading Plan

Existing topography is comprised of rolling grassy hills. Based on the topographic information provided, the site varies in elevation from a low of 502 feet near the northeast portion of the site (in tree-lined areas) to a high of about Elevation 580 feet in areas adjacent to North Frederick Road, on the northeast portion of the site.

The proposed grades will be similar to that existing with planned localized cut and fill depths on the order of -4 to +4 feet for planned roadway/parking construction.

1.3 Proposed Development

Information regarding the proposed development is based on plans provided by and discussions held with AMT. The project will consist of developing an undeveloped area of the park adjacent to North Frederick Road as a Day Use Area. The planned development will include the following:

- Entrance/Exit roadways via North Frederick Road (MD Rt. 355).
- Roadway with parking adjacent to North Frederick Road that will traverse the proposed entrance/exits. Proposed parking lots and portions of the proposed roadways leading to parking lot areas will be constructed of permeable pavement. The remainder of the

roadways, particularly entrance/exits will consist of asphalt pavement and/or gravel. Drawing No. 2 presents the proposed roadway to include the permeable/impermeable areas.

- A playground will be located on the northeast section of the site. The surface of the playground is planned to consist of recycled rubber.
- A triangular shaped building is proposed on the eastern portion of the site and is proposed to be bunkered into the hill side with the roof at grade (from east to west). Details regarding building construction and design loads were not known during the preparation of this report.
- Immediately southeast of the triangular shaped building is an area proposed to be used as an amphitheatre.
- Leading to and away from the proposed amphitheatre, playground, and throughout the Day Use Area will be walking trails consisting of compacted at-grade areas.
- A wooden bridge is proposed where planned walking trails cross existing streams.
- Group picnic areas will be dispersed throughout the park along the planned roadway and parking areas.
- The remaining site will include green space and/or native groundcover plantings.
- A septic field is preliminarily planned in the eastern, middle portion of the property.

Stormwater management facilities incorporating infiltration techniques are planned throughout the site consisting of permeable pavements.

Pavements are planned extending from and adjacent to North Frederick Road. Based on information provided by AMT from M-NPCPPC in coordination with SHA, typical volumes anticipated at each entrance are 29 trips per day. Two entrances are proposed along North Frederick Road, meaning a total of 58 trips per day for the site. The trips will predominantly consist of passenger vehicles with heavy truck traffic limited to less than once per day.

1.4 Scope of Services

TLB's scope of services comprised of the following:

- Meetings and Coordination with the Design Team.
- Formulation of Field Investigation Program. Borings to be field located via instrumented survey by AMT.
- Mobilizing an All Terrain Vehicle (ATV) mounted drill rig to perform field exploratory studies at discrete locations.
- Drilling Borings B-1 through B-20 and IT-1, IT-2, IT-4, IT-6, IT-8, IT-9, and IT-10 located at the project site and as shown in Drawing No. 2, Site Exploration Plan.
- Performing infiltration tests in boreholes for 'IT' series borings (with the exception of IT-10) at approximate anticipated SWM facility depths.

- Perform informal percolation test at location of P-1 for planning purposes prior to the performance of the actual test by Montgomery County.
- Performing visual classification of the retrieved soil samples during the field explorations.
- Performing laboratory tests on representative samples.
- Analyzing soil and groundwater conditions encountered as they pertain to the design of proposed facilities and structures.
- Preparing this report describing the conditions encountered, providing recommendations for storm water management analysis and geotechnical-related aspects of the proposed infrastructure improvement.

The following paragraphs summarize the activities, conclusions, and recommendations resulting from TLB's efforts.

2.0 FIELD INVESTIGATIONS

2.1 Soil Borings

TLB's field explorations included:

- Notifying Miss Utility of the intent to excavate at the proposed boring locations prior to TLB's mobilization to the field to commence any intrusive explorations. TLB coordinated utility clearances with the relevant utility locators to confirm where the underground utilities were marked in the field.
- Mobilizing an All Terrain Vehicle (ATV) mounted drill rig to perform the planned field explorations. Borings were marked in the field via instrumented survey performed by AMT.
- Drilling twenty-six (26) soil test borings denoted in Drawing No. 3 Site Exploration Plan as B-1 through B-20 and IT-1, IT-2, IT-4, IT-6, IT-8, IT-9, and IT-10 to collect geotechnical information for design and construction of the proposed facilities. Boring IT-1 was combined with Boring B-1 due to the close proximity of the borings. The borings were drilled to depths of 10 to 15 feet.
- Performing standard penetration testing (SPT) in accordance with ASTM D 1586. SPT sampling was performed continuously in Boring B-1 and 'IT' series borings and at 2.5 feet intervals in the remaining 'B' series borings.
- Determining depth to groundwater table during and upon completion of drilling.
- Following the completion of geotechnical explorations, each of the borings was backfilled with auger cuttings and the site restored.

The approximate boring locations are shown in the attached Drawing No. 3.

2.2 Collecting Topsoil Samples

TLB obtained twenty (20) samples of topsoil throughout the site for textural and chemical analysis for lawn and planting areas. The topsoil samples were taken at or adjacent to Borings B-1, B-3 through B-7, B-9, B-11, B-14 through B-20, IT-2, IT-3, IT-5, IT-6, and IT-10.

2.3 Infiltration Testing

Field infiltration testing was performed per guidelines published by Montgomery County Maryland, Department of Permitting Services, Water Resources Section. Those guidelines are also in line with the Appendix D1 of the Maryland Department of Environment (MDE) 2009 Stormwater Design Manual.

TLB's exploration included:

- Drilling seven (7) borings with continuous SPT sampling denoted as IT-1, IT-2, IT-4, IT-6, IT-8, IT-9, and IT-10 to characterize the soil and groundwater conditions. The 'IT' series borings were drilled to depths of 10 feet. Boring IT-1 was combined with B-1 due to their close proximity. Infiltration testing was omitted from IT-10 at the client's request.
- Drilling boreholes without SPT sampling at locations adjacent to 'IT' series borings to infiltration test depth, and inserting a 4 inch PVC casing.
- The PVC casing was filled with water to a depth of 2 feet above the bottom of the bole and allowed to pre-soak for 24 hours.
- Performing infiltration tests twenty four hours later, by refilling the PVC pipe to a depth of 2 feet and monitoring water level for 4 hours on the hour.
- Infiltration tests performed in all designated 'IT' series boring locations (with the exception of Boring IT-10).

2.4 Informal Percolation Testing

By the time the field testing was being conducted, the County Percolation testing season had passed. The client desired to get an idea of the percolation capability of the site for planning purposes, in anticipation of performing the official testing during the County stipulated season, hence use of the term 'informal testing'.

Informal percolation testing was conducted at P-1 near planned septic field location. The informal percolation testing involved excavating a 15-foot test pit for assessment of soil strata and groundwater conditions. After leaving the test pit open overnight to observe dry conditions, five (5) additional test pits were excavated adjacent to the deep pit. Logs of the test pits are presented in Appendix A. At the bottom of each test pit, a test hole was dug using a post-hole digger. The test holes were approximately 8 to 10 inches in diameter and 24 to 30 inches in depth. Five gallons of water was put in each test hole as a presoak. The following day

percolation test measurements were taken, which consisted of maintaining a known level of water in each test hole and measuring the water drop every 30 minutes for a total of 4 hours.

- Test pit, P1-G was excavated to 15 feet to assess groundwater conditions.
- Six test pits denoted as P-1A through P-1E and P-1G were performed in the area of P-1 for the purpose of informal field percolation testing.
- Informal percolation tests were performed in test pits P-1A through P-1E at test depths of 2.25 feet to 16 feet.

3.0 EXISTING INFORMATION

3.1 Geologic Setting

According to the Geologic Map of Maryland, Maryland Geologic Survey, dated 1968, the site is underlain by the Western Piedmont Metasedimentary Rocks, more specifically the Ijamsville Formation and the Marburg Schist. As mapped in this area, the formation consists of:

Ijamsville Formation – blue, green, or purple phyllite and phyllitic slate, with interbedded metasiltstone and metagraywacke.

Marburg Schist – bluish-gray to silvery-green, fine-grained quartz schist; intensely cleaved and closely folded, contains interbedded quartzites.

Above the parent rock is highly weathered rock or **Saprolite**, a soil byproduct of the weathering or decomposition of the parent rock. The Saprolite is usually reddish brown or grayish white and contains some structure that was present in the parent rock.

4.0 SUBSURFACE CONDITIONS

Logs describing the subsurface conditions encountered in each boring are presented as "Records of Soil/Rock Exploration" in Appendix A. The descriptive terminology used to classify the soils encountered during this study is summarized on the first page of Appendix A. A Generalized Subsurface Profile illustrating the soil and groundwater conditions that were encountered are presented as Drawings No. 4.

4.1 Soil Conditions

As indicated on the Records of Soil Exploration and the generalized subsurface profile, the following soil conditions were observed at the site:

The borings encountered 4 to 7 inches of topsoil. Beneath the topsoil, the borings encountered variable natural soils comprised of sandy clay and clayey sand soils with varying amount of

gravel and quartzite at near surface grades overlying silty sand, silt and/or clay soils with varying amounts of quartz, mica and Saprolite.

The near surface (0.3 to 3 feet below grade) sandy clay and clayey sand soils were generally soft to stiff or loose to medium dense based on SPT N-values ranging from 3 to 11 blows per foot (bpf). Below a depth of 3 to 4 feet, the SPT N-values increased with depth to reflect medium stiff to hard or medium dense to dense soils based on N-values ranging from 7 bpf to over 51 blows for 4 inches of penetration.

Saprolite, soft, thoroughly decomposed rock, was found at depths extending below 4 to 6 feet, and extended to the termini of the borings. This material often contained quartzites and fractures of the less weathered parent rock. Saprolite was primarily encountered in the northern portion of the site below Elevations 569.7 to 485.7 feet.

4.2 Groundwater

Groundwater was not encountered during drilling and test pit operations in any of the locations. After drilling, the borings remained dry above cave-in depths on the order of 6 to 12.3 feet. Seasonal and/or long-term fluctuations of the site's groundwater levels should be anticipated.

4.3 Infiltration Tests

As described in Section 2.3. infiltration tests were conducted at six locations near planned SWM facilities (permeable pavements incorporating infiltration techniques). The infiltration test results are summarized as follows.

Table No. 1 Summary of Infiltration Test Results

Boring No.	Infiltration Test Depth (ft)	Soil at Test Depth	Average Infiltration Rate (in/hr)	
B-1 / IT-1	4.0	Silty SAND	26.01	
IT-2	5.0	SAPROLITE	0.93	
IT-4	6.0	Silty SAND	< 0.12	
IT-6	5.0	Silty SAND	3.78	
IT-8	4.0	Sandy SILT	1.35	
IT-9	4.0	Sandy SILT	3.51	

The low infiltration rate at IT-4 is likely attributed to large rock fragments near test depth.

4.4 Informal Percolation Test

As described in Section 2.4, informal percolation testing was conducted at P-1 near planned septic field location. The results are summarized as follows:

Table No. 2
Informal Percolation Test Measurements

	Test Depth	Measured Drop in Inches (Measured in 30-Minute Intervals)									
Test Location	from Surface (ft)	1st	2nd Reading	3rd Reading	4th Reading	5th Reading	6th Reading	7th Reading	8th Reading		
P-1A	2.25	7.0	4.0	4.0	4.0	2.5	2.5	1.0	1.0		
P-1B	2.50	3.0	4.5	4.5	6.0	3.0	3.0	1.5	1.5		
P-1C	5.65	10.2	5.7	11.7	5.7	4.2	5.7	5.7	5.7		
P-1D	16.00	8.4	8.4	8.4	8.4	8.4	8.4	8.4	8.4		
P-1E	6.15	9.3	6.9	7.5	4.8	5.1	4.8	4.8	4.8		

During the informal percolation test, caving conditions were observed in test holes P-1C and P-1E after the 5th and 3rd readings, respectively. Fractured rock was observed in the bottom 15 inches of test hole P-1D, no caving conditions were observed. Test holes P-1A and P-1B (near surface tests) were observed to experience caving conditions after the 6th reading. Based on the field measurements, the following results were tabulated.

Table No. 3
Summary of Informal Percolation Test Results

	Test									
Test Locatio n	Depth from	1st Reading	2nd Reading	3rd Reading	4th Reading	5th Reading	6th Reading	7th Reading	8th Reading	Stabilized Percolation Rate (Min/In)
P-1A	2.25	7.0	4.0	4.0	4.0	2.5	2.5	1.0	1.0	30.00
P-1B	2.50	3.0	4.5	4.5	6.0	3.0	3.0	1.5	1.5	20.00
P-1C	5.65	10.2	5.7	11.7	5.7	4.2	5.7	5.7	5.7	5.26
P-1D	16.00	8.4	8.4	8.4	8.4	8.4	8.4	8.4	8.4	3.57
P-1E	6.15	9.3	6.9	7.5	4.8	5.1	4.8	4.8	4.8	6.25

Decreasing percolation rates are likely attributable to caving conditions.

5.0 LABORATORY TESTS

5.1 Geotechnical Testing

Laboratory testing was conducted to develop geotechnical parameters to provide recommendations on the geotechnical related issues of the site. Representative soil samples retrieved from each of the SPT split spoons were preserved in glass jars. Bulk bag samples were obtained from select borings. All of the soil samples were visually classified by an engineer/geologist, and then representative SPT samples were subjected to a laboratory testing program which included the general index tests:

- Natural moisture content (ASTM D 2216)
- Atterberg limits (ASTM D 4318)
- Gradation analysis with hydrometer (ASTM D 422)
- Moisture-Density Relationship (ASTM D 1557)
- California Bearing Ratio (ASTM D 1883)

Laboratory test results are summarized as follows:

Table No. 4
Summary of Laboratory Test Results

Test	Silty	Sand	Sandy Clay/Silt		
Test	Range	Average	Range	Average	
Moisture Content (%)	7.8 to 17.0	11.7	4.4 to 33.9	18	
Liquid limit (%)	34 to 40	36	31 to 42	36	
Plasticity Index (%)	4 to 7	6	6 to 12	9	
Passing #4 Seive (%)	23 to 83	61	76 to 100	92	
Passing #200 Seive (%)	8 to 36	24	50 to 72	60	
Passing 0.02mm Sieve (%)	13 to 26	20	40 to 56	46	

The gradation analysis with hydrometer and the Atterberg limits test results were used to determine the Unified Soil Classification System (USCS) and the United States Department of Agriculture (USDA) textural soil classification for the soils. The classifications are summarized in the following table.

	Summar	y of Son Classification at Inflittration	Test Deptils	
Boring	Depth of	Soil Classification (USCS;USDA)	Average Infiltration	
No.	Test (ft)	Son Classification (USCS, USDA)	Rate (in/hr)	
B-1 / IT-1	4.0	silty Sand (SM; SANDY LOAM)	26.01	
IT-2	6.0	*silty Gravel (GM; SANDY LOAM)	0.93	
IT-4	4.0	silty Sand (SM; LOAM)	< 0.12	
IT-6	4.0	silty Sand (SM; SANDY LOAM)	3.78	
IT-8	4.0	sandy Silt (ML; LOAM)	1.35	
IT-9	2.0	sandy Silt (ML; SILT LOAM)	3.51	

Table No. 5
Summary of Soil Classification at Infiltration Test Depths

Two bulk bag samples obtained from Borings B-2 and B-17 were also tested for moisture density relationship (modified proctor) in accordance with ASTM D 1557 and California Bearing Ratio (CBR) in accordance with ASTM D 1883. The modified proctor results indicate a maximum dry density of 129.2 pcf corresponding to an optimum moisture content of 11 percent (%) for the clay soils tested from B-2. The bulk bag collected from Boring B-17 had a maximum dry density of 118.2 and corresponding optimum moisture content of 13.5 for the silt soils. Soaked CBR values of 0.4% and 0.8 % were obtained after a maximum swell of 16% and 11.1% for the soils from Borings B-2 and B-17, respectively.

The first part of Appendix B presents a compilation of the geotechnical laboratory tests results that were completed during this study. Included in that appendix is Summary of Laboratory Test Results and Atterberg Limits' Results, followed by plots of the gradation analysis, modified proctor, and CBR.

The remaining soil samples are being temporarily stored in our Glen Burnie, Maryland laboratory and are available for review. Forty-five (45) days following the submittal of this report, however, those samples may be discarded unless other arrangements are made.

5.2 Chemical Testing of Topsoil

Topsoil samples were obtained throughout the site and submitted for chemical analyses and grain size distribution with hydrometer to determine proper chemical components and textural classification of the soil to promote healthy lawn and shrubs. The topsoil test results are summarized below.

^{*}Note – Classification of Gravel due to high rock content.

Table No. 6 Summary of Topsoil Chemical Analyses

Topsoil Sample	Soil	Phosphorus (P)	Potassium (K)		Magnesium (Mg)		Soluble Salts	Organio	e Matter
No.	pН	(ppm)	(ppm)	(ppm)	(ppm)	(ppm)	(mmhos/cm)	(%)	(ENR)*
B-1	5.1	140	167	717	97	0.1	0.19	2	79
B-3	5.5	40	43	712	81	0.1	0.08	2.3	88
B-4	5.9	40	39	828	113	0.2	0.16	2.8	97
B-5	5.7	29	42	634	98	0.1	0.09	2	83
B-6	6.2	17	30	805	86	0.1	0.05	1.9	80
B-7	7.6	16	51	3751	118	1	0.28	2.3	72
B-9	6.2	19	43	2066	97	0.4	0.36	2.1	74
B-11	5.9	26	65	1387	110	0.4	0.2	2.4	84
B-14	6.3	27	115	1335	154	0.5	0.18	2.6	89
B-15	5.9	24	64	889	115	0.3	0.15	2.3	86
B-16	7.4	31	74	3652	132	1	0.33	2.9	84
B-17	6.2	41	68	2136	131	0.4	0.29	2.9	89
B-18	5.6	27	40	785	106	0.2	0.13	2	81
B-19	5.1	20	29	548	50	0.1	0.11	1.7	77
B-20	5.3	75	47	402	63	0.1	0.1	1.9	82
IT-2	5.4	32	36	654	82	0.1	0.12	1.6	74
IT-3	6.2	22	48	1013	100	0.3	0.1	2.1	82
IT-5	5.9	31	45	751	80	0.2	0.08	1.9	80
IT-6	5.6	27	40	636	73	0.1	0.08	2.1	85
IT-10	7.8	28	55	3650	111	1	0.6	3	86

*Note: ENR denotes Estimated Nitrogen Removal

The majority of the topsoil samples classified as Loam with the exception of B-15 and B-16 which classified as Silt Loam and IT-10 which classified at Clay based on the USDA textural classification system. Topsoil test results are presented in Appendix B.

6.0 ENGINEERING ANALYSIS

The recommendations of the 1993 AASHTO Guide for Design of Pavement Structures were adopted to develop the pavement subgrade support conditions for the proposed paved roadway in the proposed Day Use Area. Subgrade soils will consist of low to medium plasticity sandy clay, sandy silt, clayey sand, or silty sand soils.

CBR tests were conducted on representative bag samples from Borings B-2 and B-17. Those results are presented in Appendix B, and indicate that the soaked CBR values for the natural subgrade soils are less than 1 percent. The amount of swelling in the CBR samples was very high and ranged from 11 to 16 percent.

6.1 Existing Drainage Conditions

Groundwater was not identified at any boring locations. However, the majority of the subgrade materials are anticipated to be comprised primarily of natural sandy clays, silty sand, sandy silt and clayey sands. Therefore, localized poor support conditions and frost susceptibility of the subgrade is a concern. The tendency of these types of materials to hold moisture can create support problems for the pavement during wet or freezing weather. Poor support conditions / frost susceptibility and high swell potential of the subgrade material would require undercutting and replacing the subgrade with good quality select fill material having good drainage characteristics. Due to the extent of poor support conditions along the full extent of the proposed roadway, thought should be given to thickening the gravel layer beneath pavements and the use of a geotextile fabric between the gravel layer and the natural, fine-grained soils. Please refer to the subsequent sections for additional information.

6.2 Frost Susceptibility

The subgrade materials encountered as described in Section 4.0 –Subsurface Conditions, were classified using Soil Frost Groups as recommended by the AASHTO Guide for Design of Pavement Structures. Based on the hydrometer test results from representative samples, and the unified soil classification, the percent by weight of tested material finer than 0.02 mm for the subgrade soils ranged between approximately 40 and 56 percent. Atterberg limit test results indicate that the Plasticity Index (PI) for the clay soils is between 6 and 12. Accordingly, these subgrade material fall within frost susceptibility Group F4 for the lean clays, corresponding to medium to high frost heave rate. The subgrade soils are thus prone to frost susceptibility. Those values indicate poor support conditions for the onsite soils.

For the project site, the depth to frost penetration is about 30 inches. Non-frost susceptible material should thus be provided to at least this depth. Based on the results of this investigation, and laboratory tests performed on representative soil samples, frost susceptible materials were encountered within the proposed roadway subgrades and throughout the site. As a result, non-frost susceptible material will be required for pavement construction. However, the subgrade soils in those sections are somewhat uniform. Therefore, if granular material is provided below the pavement section, the layer acts as a capillary break and lessens and/or eliminates the frost/heave effect on the pavement.

6.3 Asphalt Pavement Analysis

The project calls for construction of new pavements for parking and a bus loop. A concrete pavement section is desired in bus parking lanes, and all other areas are anticipated to consist of full depth asphalt pavement. TLB has provided a pavement analysis to determine the minimum pavement section that will be adequate to support the anticipated traffic loads.

The CBR values obtained from the laboratory test program were assigned to similar materials encountered in the various borings within the project limits. The average CBR value computed using this method was estimated to be 0.4 percent. Using the recommended correlation in the

1993 AASHTO Pavement Design Guide, a design CBR of 0.4 translates to a design subgrade resilient modulus of 540 psi.

Traffic data is needed for performing the engineering analysis to develop the pavement design recommendations. The recommendations of the 1993 AASHTO Pavement Design Guide are based on cumulative expected 18-kip equivalent single-axle load (ESAL). In order to complete this analysis, traffic information provided by M-NCPPC was used to predict the future traffic and cumulative ESALs. The provided traffic information indicates a volume of 58 trips per day with heavy truck traffic limited to once per day as summarized in Section 1.3

Axle classifications of the traffic data were not available. TLB reviewed traffic information from the Federal Highway Administration's (FHWA) Long Term Pavement Performance Program to develop the likely truck loadings. Based on the review, a truck factor of 1.1 was considered for design of the pavements. Using the available data, the calculated minimum design ESALs was were less than 10. Therefore, minimum 50,000 18-kip ESALs were used for design analysis, as summarized in Table below.

Table No. 7
Pavement Design Parameters

Design Pameter	Asphalt Pavement Section
Average Daily Traffic	58
Percent Heavy Trucks	1
Reliability Level	75%
Truck Factor (ESALs/Truck)	1.1
Flexible Pavement Design	
18-kip ESALs - 20-years life	*50,000
Initial Serviceability	4.2
Terminal Serviceability	2.4
Overall Standard Deviation	0.45
Subgrade Resilient Modulus	540

^{*}Minimum design ESALs (calculated ESALs of less than 10)

7.0 CONCLUSIONS AND RECOMMENDATIONS

Based on the subsurface conditions encountered, the site is considered moderately suitable for the planned development. The planned stormwater management facilities incorporating infiltration techniques are considered feasible, and subgrades are suitable for foundations for lightly loaded structures without major subgrade preparation.

The near surface soils are not considered suitable for support of pavements. Subgrade preparation will be required due to the moisture sensitive, and frost susceptible on-site soils with

very low CBR values. Subgrade preparation will consist of the use of a thickened granular aggregate base layer in conjunction with a geotextile fabric for separation.

7.1 Pavement Design

There are two areas of different pavement sections desired, namely:

- *Flexible pavement:* The flexible pavement design and analysis was performed with the assistance of DARWin v.3.1 software program in conjunction with the 1993 AASHTO Pavement Design Guide.
- *Permeable Pavement:* As of writing of this report, the client did not know exactly the type of permeable pavement that was to be featured in the areas slated for permeable pavement. Accordingly, TLB assumed permeable pavers for the sake of providing some idea what thickness of pavement section to be anticipated in the permeable pavement areas. The Permeable Pavement design was accomplished by use of algorithms developed by Interlocking Concrete Pavement Institute (ICPI).

Due to the existing field conditions and the susceptibility of the material to moisture and frost effects as discussed in Section 6.3, a **maximum design CBR value of 0.4** is recommended. The recommended CBR value and subgrade modulus are based on the top 12 inches of subgrade material being compacted to 95 percent of their maximum dry unit weight as determined by AASHTO T-180 (ASTM D-1557). Upon stripping the exposed subgrade soil in areas to receive flexible pavement should be densified in place and then proof rolled using a loaded dump truck. Any yielding sections that cannot be densified in place should be undercut and constructed in strict adherence of the earthworks section of this report. Compaction is not recommended in areas to receive permeable pavers.

The analysis of the traffic data provided by the client indicated an average daily traffic volume of 58 vehicles per day and less than one heavy truck per day. The anticipated traffic corresponds to less than 10 18-kip ESALs. A minimum design value of 50,000 18-kip ESALs is recommended in the AASHTO Guide for Design of Pavement Structures. The results of the flexible pavement and of permeable pavement design are summarized in Appendix C. Based on those results, recommendations for the pavement sections are as follows.

7.1.1 Flexible Pavement Section

Based on the assumed minimum design ESALs and given the very low CBR values representing the in-situ soils, a very thick pavement section would be required necessitating expensive excavation and removal of materials. Consequently, we considered stabilizing the in-situ soil as a cost cutting measure alternative. We have presented two pavement sections for the flexible pavement, and we recommend that consideration be given to the stabilized subgrade section in lieu of the more than 2 feet of excavation.

The pavement sections are as follows for the Flexible Pavement:

Unstabilized Pavement Section

- > 2.0 inches Hot Mix Asphalt Superpave 12.5mm, PG 70-22 for Surface
- > 3.0 inches Hot Mix Asphalt Superpave 19.0 mm, PG 70-22 for Base course
- > 26.0 inches Dense Graded Aggregate Base conforming to Table 901 A of Section 901.01 of the 2008 MD SHA Standard Specifications for Construction and Materials.
- > Geotextile Class "ST" conforming to Section 921.09 of the MD SHA Standard Specifications should be placed on top of the existing subgrade for separation.

The recommended pavement section is total 31 inches.

Stabilized Subgrade Soils mixed with Aggregate Base

- > 1.5 inches Hot Mix Asphalt Superpave 12.5mm, PG 70-22 for Surface
- > 3.0 inches Hot Mix Asphalt Superpave 19.0 mm, PG 70-22 for Base course
- > 15.0-inch Stabilized Subgrade soils/Aggregate Mixture (6 inches gravel and 9 inches of subgrade soils). The gravels could be asphalt millings or recycled concrete of CR-6 gradation.

The recommended pavement section is total 19.5 inches.

7.1.2 Permeable Pavement Section

Subgrade preparation for permeable pavement sections should be performed in accordance with the Earthworks section of this report. Permeable pavements should be constructed by contractors familiar with paver installation. For design purposes the following criteria should be met:

- > Concrete pavers should be interlocking pavers with a minimum of 3 1/8 inch in thickness with No. 8 aggregate in openings overlying
- > A minimum 2-inch bedding course consisting of no. 8 aggregate over
- > 8 inches of No. 57 stone over
- > 15 inches of No. 2 stone subbase
- > Geotextile Class "ST" conforming to Section 921.09 of the MD SHA Standard Specifications should be placed on top of the existing subgrade for separation.

The recommended pavement section is total of 28 inches

A typical cross section showing permeable pavement above the recommended aggregate is attached as Drawing No. 5.

7.2 Earthworks

Based on existing and planned grades minor fill and cut on the order of ± 4 feet is anticipated in localized areas of the site. Deeper excavations may be required to construct the proposed building on the eastern portion of the site. TLB understands that the excavated material is scheduled to be reused in fill areas across the site.

Subgrade Preparation

Prior to the placement of new fill, the construction area should be stripped to remove pavements, topsoil, and organic matter. Topsoil on the order of 4 to 7 inches in thickness was encountered within the borings.

Prior to placing any granular subbase, base course materials, or select backfill materials we recommend the exposed subgrades be densified in place and then proof-rolled. The exposed subgrades shall be inspected by a Geotechnical Engineer or a Technician working under his direction. Depending on the space constraints of the excavation, the exposed subgrades shall be proof-rolled with an approved roller. Proof-rolling shall be inspected by the Geotechnical Engineer or an experienced engineering technician. Upon completion of proof-rolling, the following criteria and remedial measures shall be adopted to confirm the adequacy of the subgrades:

No yielding in the subgrades: Subgrade is acceptable and ready to receive pavement layers.

Yield of 1.0-inch or less:

Undercut 6 inches of the subgrade and replace

with select fill material.

Yield between 1.0 and 6.0 Undercut 12 inches of the subgrade and replace inches: with select fill material.

Yield of 6.0 inches or more: Undercut 24 inches of the subgrade and replace with select fill material.

Compaction

All fill materials must be compacted in a controlled manner. Unless otherwise approved by the engineer based on in-situ tests of the compacted fill, we recommend the following compaction categories:

- Subgrade Fills -- All fills placed directly below foundations, slabs, or within the zone of influence 95% AASHTO T-99 (ASTM D-698).
- Pavement subgrade and Trench Backfills should be compacted to at least 95% of AASHTO T-180 (ASTM D-1557).

Regardless of the category, we recommend that all site fills be placed in essentially horizontal layers or lifts having a minimum loose lift thickness commensurate with the equipment being utilized to perform the compaction. In no case however, should those lifts exceed eight (8) inches. Each lift should be uniformly compacted to equal or exceed the specified minimum percentage of the maximum dry unit weight.

Care should be taken not to over-compact subgrade soils in areas that will be used for infiltration techniques. Over-compaction of the soils will result in low infiltration rates.

On-Site Soils

As previously discussed, the on-site silty and clayey sand soils are expected to be excavated in some locations during subgrade preparation. These soils are generally considered suitable for use as backfill, and may be used as structural fill with approval by the Geotechnical Engineer. The contractor should refrain from using excavated clay/silt soils as backfill beneath and/or around the proposed structures and utilities.

It should be noted that the on-site soils are very moisture-sensitive as exhibited by the swelling on the CBR test results. Construction during 'dry' season such as the summer months is highly recommended. Construction during 'wet' seasons will result in poor workability and poor subgrade support of the existing soils. If construction during 'wet' season cannot be avoided, a contingency should be put in place for offsite borrow material.

Borrow Material

Offsite materials <u>will</u> be required for use as structural fill. Imported material may be borrow material, select fill or other approved material. Fill material shall contain no rocks or stones larger than 3 inches. These material shall meet Unified Soil Classification System (USCS) of SC or coarser (i.e. SC through GW). Material used in backfill shall have a Liquid Limit (LL) not greater than 40 and a maximum Plasticity Index (PI) of 10. In addition, swell tests should be performed on the potential borrow material to ensure low swell potential. The swell tests should be performed in accordance with ASTM D 4546 or determined from soaked CBR tests.

All soil materials that fall within the USCS type ML, CL, CL-ML, OL, MH, CH, OH, PT, as well as material containing organic matter, ashes, cinders, refuse, frozen or other unsuitable materials are prohibited for use as backfill for subgrades. Soils classified as CL and ML are anticipated in large quantities on site. Therefore, the contractor should be prepared to bring in offsite granular material for use as backfill.

Removal and replacement is expensive, and stabilization of such materials in-situ with appropriate additives, their engineering and construction properties can be improved. Subgrades can be stabilized by the addition of lime, cement, or a combination of these materials with flyash. A mixture design is required to determine the optimum percentage of additive or combination of additives to be used. TLB would be glad to assist in the mixture design to provide a stabilized subgrade.

Where the amount of swell is less than about 3 percent, special consideration will not normally be needed. However, where a subgrade includes interspersed patches of soil with different swell characteristics, even amounts of swell less than 3 percent may require special consideration, such as removal and replacement with non-swelling soils, or stabilization in-situ with additives as mentioned previously.

Specifications should require slopes of exposed surfaces be maintained to facilitate surface runoff away from load bearing areas and to prevent ponding of surface water. If ponding of surface water does occur, it should be removed by pumping, ditching or as otherwise directed by the inspecting geotechnical engineer. During periods of anticipated inclement weather, exposed surfaces shall be graded and sealed to preclude infiltration of surface water. Subgrades, which become disturbed due to inclement weather or construction traffic and require over-excavation, should be reworked at no additional cost to the project.

Use of Existing Topsoil Material

Based on the chemical test results and analyses the existing topsoil is considered suitable for use in lawn/low shrub areas with improvements. Improvements would consist of bringing the existing pH levels, which range from 5.5 to 7.8 to the recommended level of 6.2 with the use of lime to increase pH or ammonium sulfate to decrease pH. However, if localized area pH value is equal to or lower than 7.2 it is not recommended to add ammonium sulfate to decrease pH. Recommended amounts of lime range from 0 to 80 pounds, but should be limited to 50 pounds per 1,000 square feet. Recommended amounts of ammonium sulfate to decrease pH are 2.5 pounds of elemental sulfur per 1,000 square feet for every 0.1 of pH unit above 7.2. Recommended amounts of lime or sulfur based on the individual pH levels are reported for each test result attached in Appendix B. In addition to raising/lowering the pH levels, a recommended fertilization program is outlined for each topsoil sample tested. These recommendations should be used for planning purposes only. The existing topsoil should be stockpiled and mixed thoroughly during stripping and grubbing procedures. At that time a composite sample of topsoil should be tested and analyzed to provide a more accurate depiction of the composite topsoil stockpile and necessary improvements for use in lawn areas.

7.3 Groundwater Considerations

Groundwater was not encountered in any of the borings during drilling operations. Subsequent to drilling, boreholes remained dry above cave-in depths of 6.0 to 12.3 feet below existing grades. Groundwater is not expected to impact construction. However the contractor should be prepared to manage/divert run-off away from construction areas, as the on-site soils are moisture sensitive and will require increased effort to compact, or remove and replace if allowed to get wet.

7.4 Support of Foundations

As previously discussed, a triangular shaped building is proposed on the eastern portion of the site and is proposed to be bunkered into the hill side with the roof at grade (from east to west). Details regarding the building construction to include type of construction and anticipated building loads were not known during the preparation of this report.

Boring B-19 was performed in the area of the proposed building. The soils anticipated to support foundations at the anticipated foundation depth will consist of very stiff to hard sandy clay soils.

The triangular building may be supported on **shallow spread footings with an allowable bearing capacity of 2,500 psf.** The recommended maximum allowable bearing pressure is based on total settlements being limited to less than 1.0 inch. Differential settlements are anticipated to be less than 0.5 inch. Footings should be founded within natural soils or within structural fill prepared in accordance with the Earthwork Section of this report. Care shall be exercised to ensure that the soils encountered at the founding grade remain dry.

Foundation Construction

Stepped down and/or adjacent column footings should be positioned outside of a 3H:1V slope line extending outward from the underside of the nearest adjacent footings. Competent undisturbed natural soil and/or compacted structural fill should exist everywhere within this zone of footing influence. Strict adherence to the 'Earthwork' section of this report is recommended with regard to structural fill. Regardless of the computed footing sizes, we recommend all continuous footings have a minimum width of 18 inches. Isolated footings should be a minimum of 24 inches in least dimension. It is recommended that the footings be founded a minimum 30 inches below final exterior grades and within the natural material or structural fill.

We recommend that surface runoff be directed away from footings.

7.5 Slab-on-Grade

It is our recommendation to support the at-grade floors of the triangular building on proof-rolled exposed subgrades and/or new structural fill. In regard to subgrade preparation, strict adherence to Earthwork section of this report is recommended.

We recommend that a minimum 5.0-inch layer of porous stone, consisting of gravel or crushed stone, be placed immediately beneath the at-grade slabs. A polyethylene membrane or similar vapor barrier should be used to separate the concrete from porous stone or subgrade. It is recommended that the heavily loaded at-grade concrete slabs/pads should be designed based on a **modulus of subgrade reaction of 75 psi/in.**

Where at-grade slabs of the building will be placed below the finished exterior grades and of the potential for water to rise seasonally, it is recommended to install perimeter footing drains. Exterior perimeter drains should be provided adjacent to the lowest subgrade wall or isolated foundations. The drainage trenches and the perimeter drains should have a positive slope and should discharge into sump pump pits. Pumps should be installed unless the drains will be provided with a gravity discharge to the proposed drainage fields or daylighting at the ground surface. The discharge pipe for sump pumps, should discharge at least ten feet away from any foundation, to an area of positive drainage away from the foundation.

7.6 Below Grade Walls

Based on the provided information, the triangular building will be bunkered into the existing slope so that the roof is at grade looking from east to west. Based on Boring B-19, drilled in the vicinity of the proposed building, the soils anticipated behind the building walls will consist of

medium dense silty sand soils. For design of lateral pressures the following soil parameters are recommended:

Table No. 8

Recommended Parameters for Below Grade Walls and Temporary Support of Structures

Structures									
Soil Type	Friction Angle	Ka	Ka Ko		Unit Wt.	Maximum Equivalent Fluid Wt. (pcf)			
	(°)				(pcf)	Active	At-Rest	Passive	
On-site Silty Sand	30	0.33	0.50	3.0	125	42	63	375	
On-site Sandy Clay	26	0.39	0.56	2.6	125	49	70	320	
Off-site Structural Fill	32	0.31	0.47	3.3	130	40	61	423	

These recommendations are for walls backfilled with granular soils. The onsite silty or clayey sand soils are suitable for use as backfill material behind the walls in non-load bearing areas. The at-rest pressure coefficient (Ko) should be used if the wall is restrained from lateral movement prior to backfilling. Otherwise, if the wall is unbraced and free to move laterally, the active pressure coefficient (Ka) should be used.

The recommended earth pressure coefficients and unit weights assume drained conditions will exist behind the walls during construction and after final grading. If undrained conditions are to be designed for, then hydrostatic pressure should be superimposed on the lateral earth pressure that is computed using effective unit weight of the supported soils. Prefabricated vertical drains such as Miradrain G100W or equal, together with perimeter drains below the subgrade elevation may be installed in lieu of the free-draining material. The drains should daylight at approved locations or be connected to approved stormdrains. Additional recommendations may be necessary if submerged conditions are to be included in the design.

Surcharge, dead or live loads occurring within a horizontal distance equal to the height of the wall should be superimposed on the earth pressure loads over the entire height of the wall. The surcharge should be computed as uniform lateral pressure equal to the appropriate lateral earth pressure coefficient provided hereinbefore times the magnitude of the surcharge.

7.7 Temporary Support of Excavation

Excavation for the planned building and utilities will likely require temporary slopes or support of excavation (SOE). Soils expected to be supported are predominantly clay and silts although sands and gravels (to include fractured rock and quartzites) were occasionally logged. Where unsupported, we believe temporary slopes for excavations are feasible based on available space,

and we recommend they be no steeper than 1.5H:1.0V. As for installation of utilities, a trench mule or other approved movable structures could suffice in the support of the excavation trenches as long as they can support the subgrade materials to the invert depths of the planned utilities.

Because of the planned construction of the triangular building, temporary support of excavation may be required. We recommend all temporary excavation supports be designed for the appropriate active and/or passive earth pressure conditions. The temporary support system should be designed using the design parameters shown in the table above. Soldier piles and lagging, sheet piles, soils nails, or other approved methods could be considered for temporary support, if deep excavations are contemplated.

A unit weight of 125 pcf is recommended for the determination of the lateral forces. Surcharge loads occurring within a horizontal distance equal to the height of the excavation should also be superimposed on the recommended earth pressure loads. It is recommended that the contractor should be required to engage a professional engineer registered in the State of Maryland to design and seal the plans and drawings for all of his temporary structures.

7.8 Stormwater Management

The planned **stormwater management facilities** will incorporate infiltration techniques and will consist of permeable pavements.

As discussed, infiltration tests were performed at 'IT' series boring locations to assess the viability for infiltration techniques to be incorporated into the SWM design.

Based on Maryland Department of the Environment (MDE) guidelines, a vertical buffer of 2 to 4 feet should exist between the infiltration invert and groundwater or rock. In addition, the minimum acceptable average infiltration rate for stormwater management and water quality applications, as indicated by borehole infiltration testing, is 0.52 inches per hour. Soils meeting this criterion are generally classified in accordance with the United States Department of Agriculture (USDA) textural classification as Loam or more granular.

Based on the subsurface investigation, USDA classifications of Sandy Loam, Loam, and Silt Loam were commonly encountered throughout the site with infiltration rates ranging from less than 0.12 inch per hour to over 26 inches per hour. Infiltration rates were generally between 0.93 in/hr to 3.78 in/hr. We have reviewed the field infiltration data vis-à-vis soil texture and opine the results of B-1/IT-1 are anomalous. The results are markedly higher than those from the other tests performed in similar soils, Sandy Loam. Whereas published data indicates minimum infiltration rate for Sandy Loan is 1.02 inches per hour, the recorded rate is way higher than anticipated for Sandy Loam. This may lead one to believe material tested at the offset location from the SPT boring was a lot coarser than the soils observed in the SPT boring, thus exhibiting a condition that is not representative of the site. Therefore, the results of Boring B-1/IT-1 should be disregarded and this location should be treated no different than the other locations with Sandy Loam soils.

The site has infiltration potential. Because of the variable infiltration rates obtained in the infiltration tests, TLB recommends that SWM facilities incorporating *infiltration techniques be designed using an infiltration rate of 0.52 in/hr*. Infiltration techniques may not be feasible in localized areas due to cementation, or the presence of significant amounts of rock, silt and/or clay.

7.9 Percolation Test Results

As narrated in Section 4.4, informal percolation testing was conducted at multiple locations at P-1 near planned septic field location. Stabilized Percolation rates were calculated from the field tests and are summarized as follows:

Table No. 9
Stabilized Percolation Rates

Test Location	Rate (Min/In)
P-1A	30.00
P-1B	20.00
P-1C	5.26
P-1D	3.57
P-1E	6.25

It is recommended to use these rates for planning purposes and confirm the rates during the County stipulated testing period.

8.0 LIMITATIONS

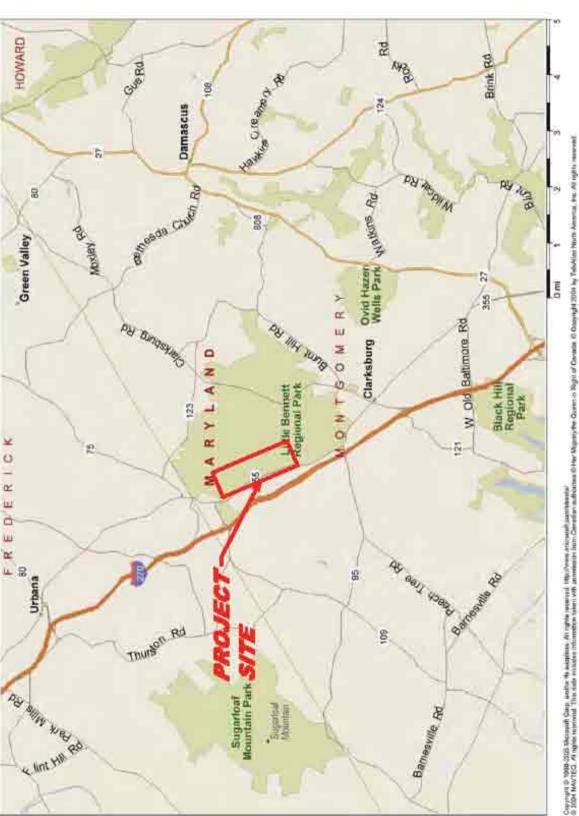
All subsurface and field investigations require the extrapolation of limited amounts of data based on general geologic knowledge. This report has been prepared to aid in the evaluation of the site. This report is intended to assist Maryland-National Capital Park and Planning Commission and A. Morton Thomas and Associates, Inc. with the design aspects of the proposed renovations together with the earthwork related portions of the project based upon our understanding of the design details, criteria, and utilization of the planned facilities as outlined herein. The water level observations and geologic descriptions presented on the accompanying logs have been made with reasonable care and accuracy, but must be considered only an approximate representation of subsurface conditions to be encountered beyond a particular exploratory location.

We recommend that a Geotechnical Engineer or a technician under his direction be retained during construction to monitor subgrade preparation and construction and to evaluate general construction techniques as they may affect existing and planned facilities at the site. The Engineer or Technician should be instructed to monitor subsurface conditions encountered during construction to see that those conditions are compatible with the findings of this study. If significant variations are encountered or if the proposed locations or designs are altered, we should be contacted and provided the opportunity to appropriately review and/or modify these recommendations.

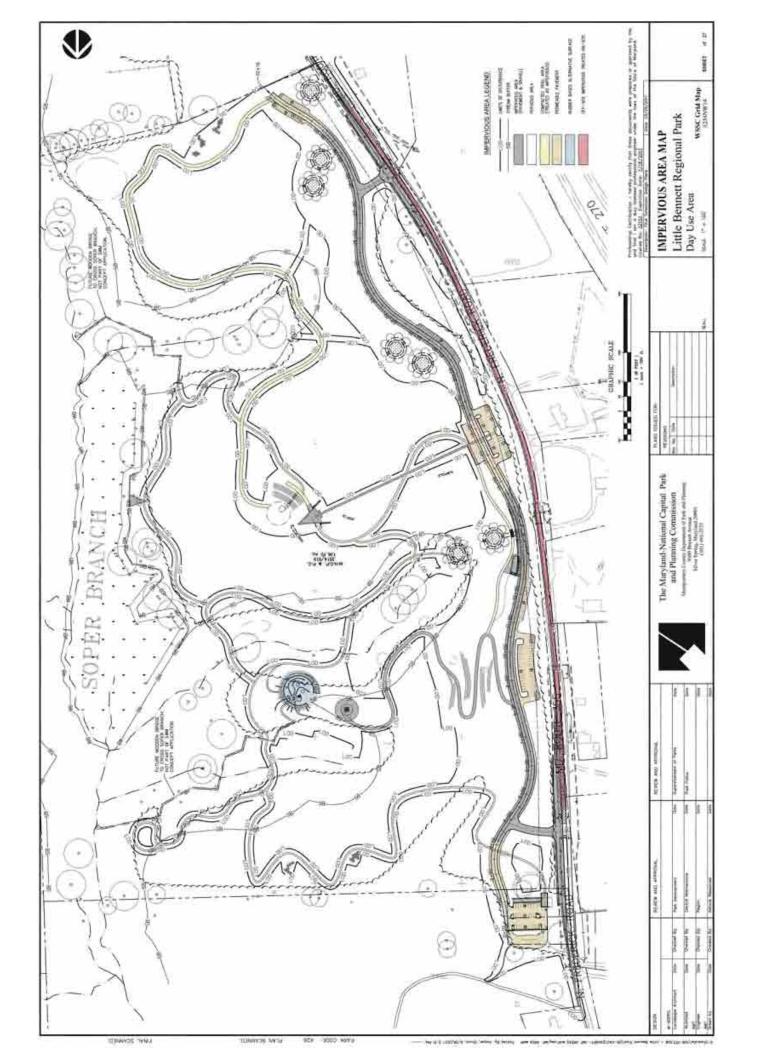
SITE LOCATION PLAN

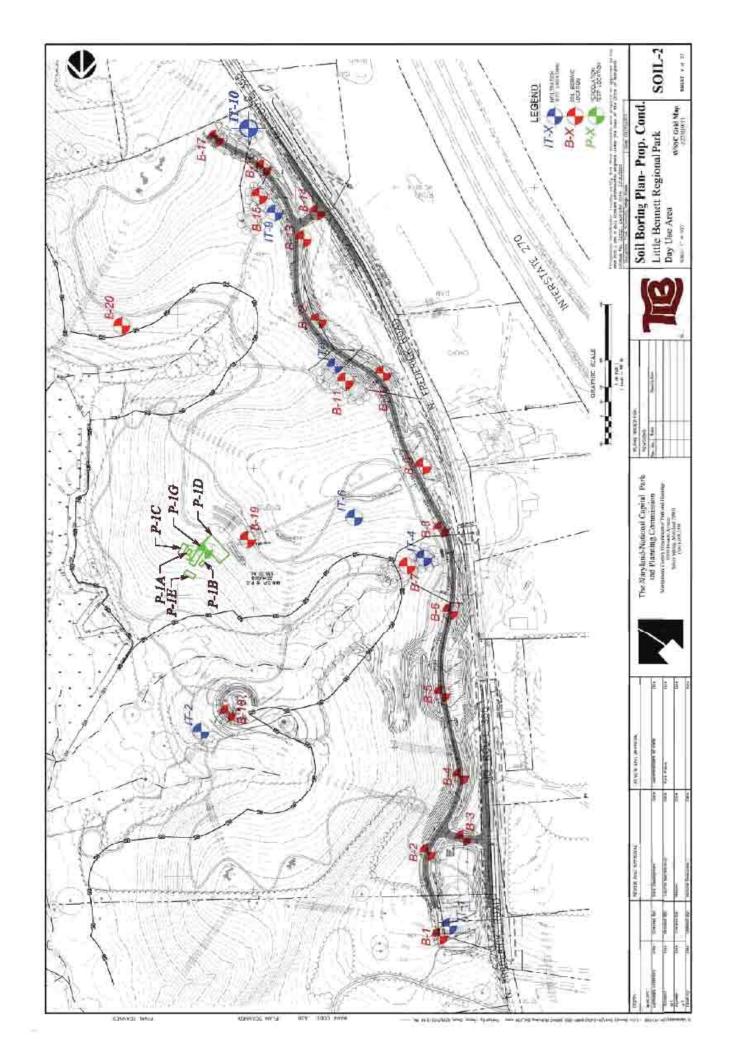
DRAWN BY: KNB
CHBCKED BY: SN

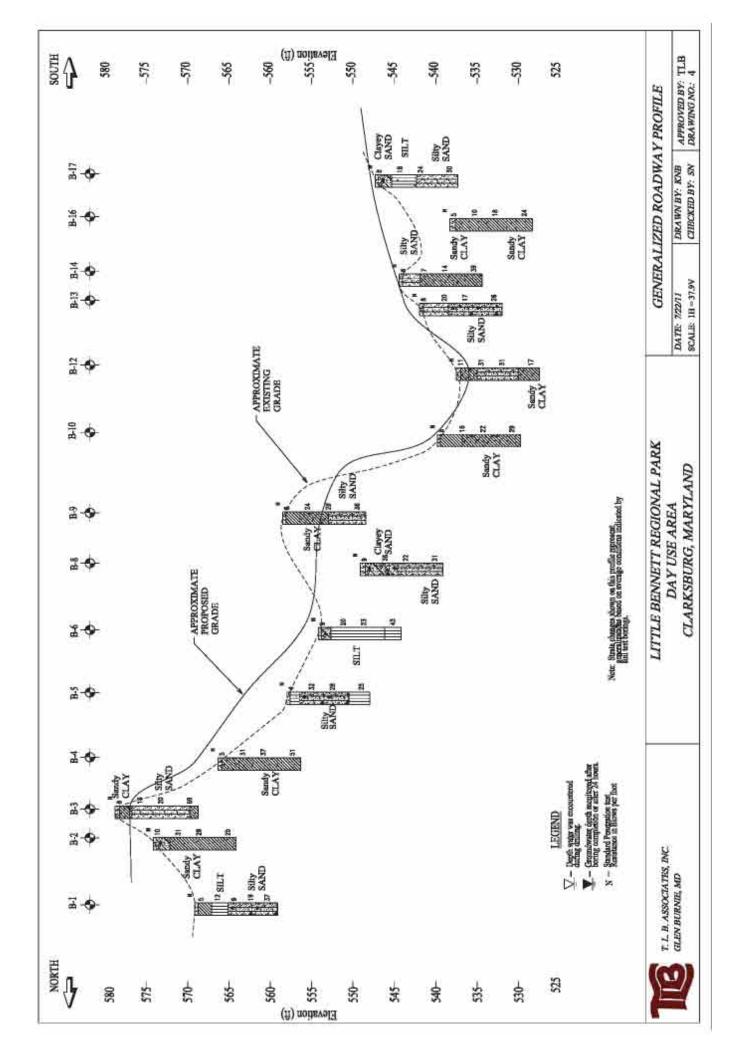
DATE: 7/24/11 SCALE: NTS

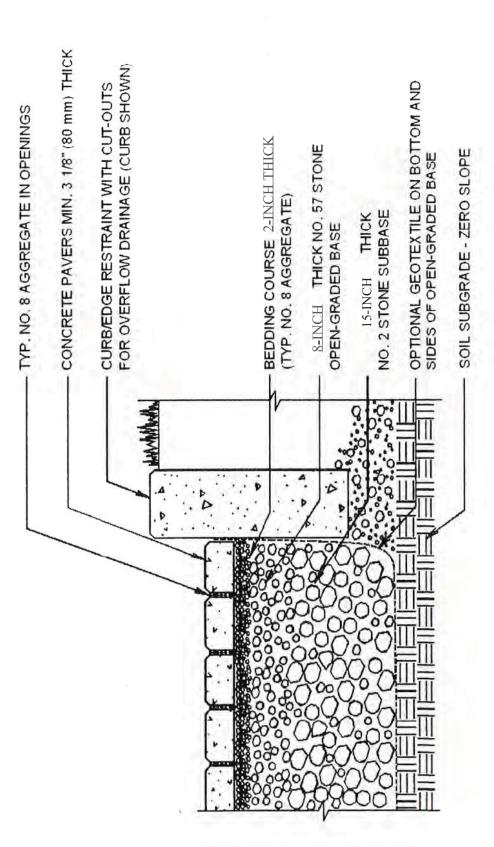


LITTLE BENNETT REGIONAL PARK DAY USE AREA CLARKSBURG, MD









TYPICAL SECTION BY INTERLOCKING CONCRETE PAVEMENT INSTITUTE

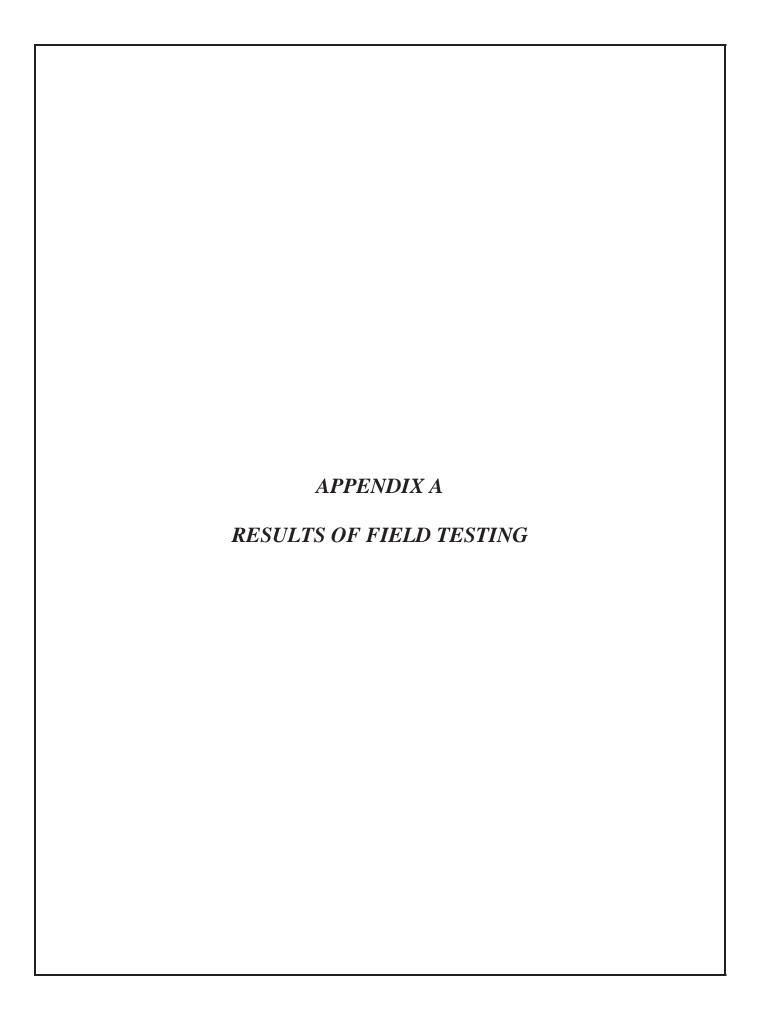


LITTLE BENNETT REGIONAL PARK DAY USE AREA CLARKSBURG, MD

TYPICAL PERMEABLE PAVEMENT SECTION DRAWN BY: KNB CHECKED BY: SN DATE: 7/27/11

SCALE: NTS

APPROVED BY: TLB DRA WING NO.: 5



GENERAL CLASSIFICATION SUMMARY FOR SOIL & ROCK EXPLORATION SOIL

	Particle Size Identification	Relative Proportions
Boulders	- 12 inch diameter or more	Descriptive Term Percent
Cobbles	- 3 to 12 inch diameter	
Gravel	- Coarse - 3/4 to 3 inches	In accordance with ASTM D 2487 and
	- Fine - 4.75mm to 3/4 inch	ASTM D 2488
Sand	- Coarse - 2.00mm to 4.75 mm [Sieve #10 to #4]	
	- Medium - 0.4mm to 2.00mm [Sieve #40 to #10]	
	- Fine - 0.075mm to 0.4mm [Sieve #200 to #40]	
Silt/Clay	- less than 0.075mm (Cannot see particles)	
Silt	- Atterberg limits plot below "A" line	
Clay	- Atterberg limits plot above "A" line	

COHESIONLESS SOILS

DensityN-ValueVery loose0-4 blows/ft.Loose5-10 blows/ft.Medium Dense11-30 blows/ft.Dense31-50 blows/ft.Very Dense>50 blows/ft.

COHESIVE SOILS

Consistency	Λ	<i>I-Value</i>
Very Soft	0-1	blows/ft.
Soft	2-4	blows/ft.
Medium Stiff	5-8	blows/ft.
Stiff	9-15	blows/ft.
Very Stiff	16-30) blows/ft.
Hard	>30	blows/ft.

Classifications on logs are made by visual inspection.

Standard Penetration Test - Driving a 2.0" O.D., 1 3/8" I.D., sampler a distance of 1.0 foot into undisturbed soil with a 140 pound hammer free falling a distance of 30.0 inches. It is customary for us to drive the spoon 6.0 inches of penetration to seat into undisturbed soil, and then perform the test. The number of hammer blows for seating the spoon and making the tests are recorded for each 6.0 inches of penetration on the drill log (Example: 6-8-9). The standard penetration test resistance or "N"-value can be obtained by adding the last two figures (i.e., 8 + 9 = 17 blows/ft.).

Strata Changes - In the column "Soil Descriptions" on the drill log, the horizontal lines represent estimated strata changes.

<u>Groundwater</u> observations were made at the times indicated. Porosity of soil strata, weather conditions, site topography, etc., may cause changes in the water levels indicated on the logs.

ROCK

<u>Rock Quality Designation</u> (RQD) - The sum of the lengths of pieces of recovered core which are greater than four inches in length, expressed as a percentage of the total length of the core run. If the core has been broken by the drilling process, it is considered to be intact provided the broken fragments are cumulatively greater than 4 inches in length. For this investigation, vertical separations which split the core have not been considered discontinuities when determining RQD.

Recovery (REC) - The total length of core recovered expressed as a percentage of the total length of that coring run.

ROCK CLASSIFICATION

Fresh- No visible signs of discoloration or decomposition.

Slightly weathered - Slightly discolored. Lower in strength than fresh rock. Dull under hammer.

Moderately weathered - Significant portions show discoloration and weakening (softening, lighter color). Shows loss of weight. Rock fabric evident.

Highly weathered - Almost all of the rock shows severe discoloration and weathering. Rock fabric evident in majority of the rock. **Completely weathered (Saprolite) -** Rock fabric discernible in a few scattered locations. Effectively reduced to soil and can be broken by hand.

Residual Soil- Reduced to soil. Rock fabric not discernible. Can be easily broken by hand.

RECORD OF SOIL / ROCK EXPLORATION

Contracted With	A. Morton Thomas & Associates, Inc.	Borina# _	B-1
Project Name	Little Bennett Regional Park	Job #	10-030-A

Clarksburg, MD Location __

SAMPLER

Datum	Hammer Wt140 lb	Hole Diameter 8 in	ForemanW. Seward
Surf. Elev 569.1 ft	Hammer Drop30 in	Rock Core Dia. N/A	Inspector K. Bullock
Date Started5/25/11	Spoon Size 2 in	Boring MethodHSA	Date Completed5/25/11

	SOIL DESCRIPTION	STRA	OL O	Ξщ		SAN	/IPLE			DODINO A CAMPLE	
ELEV. (ft)	Color, Moisture, Density, Plasticity, Size Proportions	DEPTH (ft)	SOIL	DEPTH	Cond	Blows/6"	No.	Туре	Rec (in)	BORING & SAMPLE NOTES	
568.7	5" of TOPSOIL	0.4	7/1/2 7/1/							No water encountered.	H
	Brown, moist, medium stiff, lean CLAY, trace sand and topsoil			-	I/D	1-2-3-3	1	DS	12	Grab bag of topsoil taken.	_
567.1	Brown, moist, stiff, SILT, trace mica	2.0			I/D	4-5-7-6	2	DS	20	3. 4-inch PVC pipe installed in offset borehole at a depth of 4.0 ft. for infiltration test.	
565.1	Tan and gray, moist, medium stiff to hard, silty SAND, with rock fragments, trace mica, (USCS:SM; USDA: SANDY LOAM)	4.0		<u>5</u>	I/D	4-4-5-7	3	DS	18	Boring drilled using a Simco 2800 All Terrain Vehicle (ATV) mounted drill rig with safety hammer.	
	Average Infiltration Rate = 26.01 in/hr (SAPROLITE)				I/D	6-9-10-11	4	DS	13	5. Boring Coordinates: N: 580668.6918 E: 1226836.3347	
	(SAFROLITE)				,,,		·		10		
559.1		10.0		- 10	ı	13-15-22-41	5	DS	18		
	Bottom of Boring at 10.0 ft			_							
				_	-						
				_							
				<u>15</u>							
				_	_						
				_							
				_							
				20							

SAMPLER TYPE

DS - DRIVEN SPLIT SPOON

PT - PRESSED SHELBY TUBE CA - CONTINUOUS FLIGHT AUGER RC - ROCK CORE

SAMPLE CONDITIONS

D - DISINTEGRATED I - INTACT

U - UNDISTURBED L - LOST

GROUNDWATER DEPTH

AT COMPLETION dry ft
AFTER HRS. ft
AFTER 24 HRS. ft
CAVED AT 8.0 ft

BORING METHOD

HSA - HOLLOW STEM AUGERS CFA - CONTINUOUS FLIGHT AUGERS
DC - DRIVING CASING
MD - MUD DRILLING

STANDARD PENETRATION TEST DRIVING 2" OD SAMPLER 1' WITH 140# HAMMER FALLING 30": COUNT MADE AT 6" INTERVALS

T.L.B. ASSOCIATES, INC.

Glen Burnie, Maryland 21061

Page 1 of 1

RECORD OF SOIL / ROCK EXPLORATION

Contracted With _	A. Morton Thomas & Associates, Inc.	Boring #	B-2
Project Name	Little Bennett Regional Park	Job #	10-030-A
Location	Clarksburg, MD		
	SAMPI FR		

Datum	Hammer Wt140 lb	Hole Diameter 8 in	ForemanW. Seward
Surf. Elev. 574.1 ft	Hammer Drop 30 in	Rock Core Dia. N/A	Inspector K. Bullock
Date Started 5/25/11	Spoon Size 2 in	Boring Method HSA	Date Completed 5/25/11

	E. E. (SOIL DESCRIPTION	STRA	٦,	Ξщ		SAM	MPLE			DODING A GAMES E	
	ELEV. (ft)	Color, Moisture, Density, Plasticity, Size Proportions	DEPTH (ft)	SYMBOL	DEPTH SCALE	Cond	Blows/6"	No.	Туре	Rec (in)	BORING & SAMPLE NOTES	
_	573.8	4" of TOPSOIL Grayish-brown, moist, loose, clayey SAND, trace gravel and organics	0.3			1	5-5-5	1	DS	8	No water encountered. 1 bag sample collected from 1.0 - 5.0 ft.	
_	572.1	Gray, moist, hard to very stiff, sandy, lean CLAY, trace mica, (USCS: CL; USDA: LOAM)	2.0		-	I/D	11-7-24	2	DS	18	3. Boring drilled using a Simco 2800 All Terrain Vehicle (ATV) mounted drill rig with safety hammer. 4. Boring Coordinates:	
					<u>5</u> -	I/D	10-16-13	3	DS	12	N: 580369.1963 E: 1226879.1958	
_	564.1	Bottom of Boring at 10.0 ft	10.0		_ _ 10_	D	9-13-12	4	DS	10		_
- - -	_				_							_
_	_											_
					_							
ION LOGS, GFU PROVER	_				20							

SAMPLER TYPE

DS - DRIVEN SPLIT SPOON

PT - PRESSED SHELBY TUBE CA - CONTINUOUS FLIGHT AUGER RC - ROCK CORE

D - DISINTEGRATED I - INTACT U - UNDISTURBED

L - LOST

SAMPLE CONDITIONS

AT COMPLETION dry ft
AFTER HRS. ft
CAVED AT 7.0 ft

GROUNDWATER DEPTH

BORING METHOD

HSA - HOLLOW STEM AUGERS CFA - CONTINUOUS FLIGHT AUGERS
DC - DRIVING CASING
MD - MUD DRILLING

STANDARD PENETRATION TEST DRIVING 2" OD SAMPLER 1' WITH 140# HAMMER FALLING 30": COUNT MADE AT 6" INTERVALS

Location ___

RECORD OF SOIL / ROCK EXPLORATION

Contracted With	A. Morton Thomas & Associates, Inc.	Borina#_	B-3
Project Name	Little Bennett Regional Park	Job#	10-030-A
Location	Clarksburg, MD		

SAMPLER

Datum	Hammer Wt140 lb	Hole Diameter 8 in	ForemanW. Seward
Surf. Elev 578.7 ft	Hammer Drop30 in	Rock Core Dia. N/A	Inspector K. Bullock
Date Started5/25/11	Spoon Size2 in	Boring MethodHSA	Date Completed5/25/11

	SOIL DESCRIPTION	STD A	7	ΞШ		SAI	MPLE				٦
ELEV. (ft)	Color, Moisture, Density, Plasticity, Size Proportions	STRA DEPTH (ft)	SYMBOL	DEPTH SCALE	Cond	Blows/6"	No.	Туре	Rec (in)	BORING & SAMPLE NOTES	
578.2	6" of TOPSOIL	0.5	7/1/2 7/1							No water encountered.	
	Brown, moist, medium stiff, sandy, lean CLAY , trace gravel and organics			-	I/D	1-3-5	1	DS	12	Grab bag of topsoil taken.	_
576.7	Brown, moist, medium dense to very dense, silty, fine SAND	2.0		_	D	8-13-5	2	DS	14	3. Boring drilled using a Simco 2800 All Terrain Vehicle (ATV) mounted drill rig with safety hammer.	
				<u>5</u>	D	10-10-10	3	DS	12	4. Boring Coordinates: N: 580320.9693 E: 1226750.5609	
				_							
500.7				_							
569.7	Gray, moist, hard, sandy lean CLAY, trace rock fragments and mica, (SAPROLITE)	9.0		10_	D	13-32-37	4	DS	18		
	Bottom of Boring at 10.0 ft										
				_	-						
				_							
				_							
				_	-						
				15							
				-	-						F
				_							L
				_							
				_	-						
				20							

SAMPLER TYPE

DS - DRIVEN SPLIT SPOON

RC - ROCK CORE

PT - PRESSED SHELBY TUBE CA - CONTINUOUS FLIGHT AUGER

SAMPLE CONDITIONS D - DISINTEGRATED

I - INTACT U - UNDISTURBED L - LOST

AT COMPLETION dry ft
AFTER HRS. ft
AFTER 24 HRS. ft
CAVED AT 7.5 ft

GROUNDWATER DEPTH

BORING METHOD

HSA - HOLLOW STEM AUGERS CFA - CONTINUOUS FLIGHT AUGERS
DC - DRIVING CASING
MD - MUD DRILLING

Contracted With	A. Morton Thomas & Associates, Inc.	Borina#_	B-4
Project Name	Little Bennett Regional Park	Job#	10-030-A
Location	Clarksburg, MD		

SAMPLER

Datum	Hammer Wt140 lb	Hole Diameter 8 in	ForemanW. Seward
Surf. Elev566.3 ft	Hammer Drop30 in	Rock Core Dia. N/A	Inspector K. Bullock
Date Started 5/26/11	Spoon Size2 in	Boring MethodHSA	Date Completed5/26/11

	SOIL DESCRIPTION	CTDA	7	ΙШ		SAI	MPLE				٦
ELEV. (ft)	Color, Moisture, Density, Plasticity, Size Proportions	STRA DEPTH (ft)	SYMBOL	DEPTH SCALE	Cond	Blows/6"	No.	Туре	Rec (in)	BORING & SAMPLE NOTES	
565.8	6" of TOPSOIL	0.5	7/1/2 7/1							No water encountered.	
	Brown, moist, medium stiff, sandy lean CLAY, trace organics			-	I/D	1-2-3	1	DS	12	Grab bag of topsoil taken.	
563.8	Gray with brown, moist, hard, sandy lean CLAY, some rock fragments, trace mica, (SAPROLITE)	2.5		_	D	7-15-16	2	DS	13	Boring drilled using a Simco 2800 All Terrain Vehicle (ATV) mounted drill rig with safety hammer.	
				5	D	9-16-21	3	DS	14	4. Boring Coordinates: N: 580099.6747 E: 1226760.3946	
				_							
				_	D	10-22-29	4	DS	18		
556.3	Bottom of Boring at 10.0 ft	10.0		<u>10</u> _							
				_							
				_							
				_							
				_							
				<u>15</u>							

SAMPLER TYPE

DS - DRIVEN SPLIT SPOON

PT - PRESSED SHELBY TUBE
CA - CONTINUOUS FLIGHT AUGER RC - ROCK CORE

SAMPLE CONDITIONS D - DISINTEGRATED I - INTACT

U - UNDISTURBED L - LOST

GROUNDWATER DEPTH AT COMPLETION dry ft
AFTER HRS. ft
CAVED AT 6.3 ft

BORING METHOD HSA - HOLLOW STEM AUGERS

CFA - CONTINUOUS FLIGHT AUGERS
DC - DRIVING CASING
MD - MUD DRILLING

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RECORD OF SOIL / ROCK EXPLORATION

Contracted With	A. Morton Thomas & Associates, Inc.	Borina#_	B-5
Project Name	Little Bennett Regional Park	Job#	10-030-A
Location	Clarksburg, MD		

SAMPLER

Datum	Hammer Wt. 140 lb	Hole Diameter 8 in	Foreman W. Seward
Surf. Elev. 558.0 ft	Hammer Drop 30 in	Rock Core Dia. N/A	Inspector K. Bullock
Date Started 5/26/11	Spoon Size 2 in	Boring Method HSA	Date Completed 5/26/11

EL E. (SOIL DESCRIPTION	STRA	79	Ξщ		SAI	MPLE			DODING & GAMPLE
ELEV. (ft)	Color, Moisture, Density, Plasticity, Size Proportions	DEPTH (ft)	SOIL	DEPTH SCALE	Cond	Blows/6"	No.	Туре	Rec (in)	BORING & SAMPLE NOTES
557.6	5" of TOPSOIL Brown, moist, loose, silty SAND, trace gravel and organics Brown, moist, dense to medium dense, silty SAND, trace gravel	0.4	<u> </u>	-	I/D	1-2-2	1	DS	18	No water encountered. Boring drilled using a Simco 2800 All Terrain Vehicle (ATV) mounted drill rig with safety hammer.
				- -	I/D	16-16-16	2	DS	16	3. Boring Coordinates: N: 579807.3579 E: 1226828.4320
				<u>5</u> –	D	15-13-15	3	DS	18	
550.5	Tan, moist, very stiff, SILT, trace rock fragments, (SAPROLITE)	7.5		_		7.40.40	4	D0	16	
548.0	Bottom of Boring at 10.0 ft	10.0		10_	D	7-12-13	4	DS	10	
				_						
				_						
_				<u>15</u>						
				_						
				_						

SAMPLER TYPE

DS - DRIVEN SPLIT SPOON

PT - PRESSED SHELBY TUBE CA - CONTINUOUS FLIGHT AUGER RC - ROCK CORE

I - INTACT U - UNDISTURBED L - LOST

SAMPLE CONDITIONS

D - DISINTEGRATED

GROUNDWATER DEPTH AT COMPLETION dry ft
AFTER HRS. ft
AFTER 24 HRS. ft
CAVED AT 6.5 ft

BORING METHOD

HSA - HOLLOW STEM AUGERS CFA - CONTINUOUS FLIGHT AUGERS
DC - DRIVING CASING
MD - MUD DRILLING

T.L.B. ASSOCIATES, INC.

Glen Burnie, Maryland 21061

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RECORD OF SOIL / ROCK EXPLORATION

Contracted With	A. Morton Thomas & Associates, Inc.	Boring#_	B-6
Project Name	Little Bennett Regional Park	Job#	10-030-A
Location	Clarksburg, MD		

SAMPLER

Datum	Hammer Wt140 lb	Hole Diameter 8 in	ForemanW. Seward
Surf. Elev 554.2 ft	Hammer Drop30 in	Rock Core Dia. N/A	Inspector K. Bullock
Date Started5/25/11	Spoon Size 2 in	Boring Method HSA	Date Completed5/25/11

		SOIL DESCRIPTION	STRA	٦.	ΞШ		SAI	MPLE]
	ELEV. (ft)	Color, Moisture, Density, Plasticity, Size Proportions	DEPTH (ft)	SOIL	DEPTH SCALE	Cond	Blows/6"	No.	Туре	Rec (in)	BORING & SAMPLE NOTES	
_	553.8 552.7	5" of TOPSOIL Brown, moist, stiff, sandy lean CLAY, trace organics Brown, moist, very stiff, SILT, some sand	1.5			D	2-5-4	1	DS	8	No water encountered. Boring drilled using a Simco 2800 All Terrain Vehicle (ATV) mounted drill rig with safety hammer.	
					-	D	6-9-11	2	DS	14	3. Boring Coordinates: N: 579511.5123 E: 1226798.9983	_
		Quartzites observed at 5 ft			<u>5</u>	D	11-12-11	3	DS	18		
	<u>546.2</u>	Tan, moist, hard, SILT, some rock fragments, (SAPROLITE)	8.0		- - -	D	21-23-20	4	DS	18		_
	544.2	Bottom of Boring at 10.0 ft	10.0		10_							
					_	-						_
					 15							_
					_	-						_
					_							
					20	_						

SAMPLER TYPE

DS - DRIVEN SPLIT SPOON

PT - PRESSED SHELBY TUBE
CA - CONTINUOUS FLIGHT AUGER RC - ROCK CORE

SAMPLE CONDITIONS

D - DISINTEGRATED I - INTACT

U - UNDISTURBED L - LOST

GROUNDWATER DEPTH

AT COMPLETION dry ft
AFTER HRS. ft
AFTER 24 HRS. ft
CAVED AT 7.5 ft

BORING METHOD

HSA - HOLLOW STEM AUGERS CFA - CONTINUOUS FLIGHT AUGERS
DC - DRIVING CASING
MD - MUD DRILLING

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RECORD OF SOIL / ROCK EXPLORATION

Contracted With	A. Morton Thomas & Associates, Inc.	Borina#_	B-7
Project Name	Little Bennett Regional Park	Job#	10-030-A
Location	Clarksburg, MD		

SAMPLER

Datum	Hammer Wt140 lb	Hole Diameter 8 in	ForemanW. Seward
Surf. Elev534.2 ft	Hammer Drop 30 in	Rock Core Dia. N/A	Inspector K. Bullock
Date Started5/24/11	Spoon Size 2 in	Boring Method HSA	Date Completed5/24/11

		SOIL DESCRIPTION	STRA	٦-	Ξщ		SAM	ИPLE]
	ELEV. (ft)	Color, Moisture, Density, Plasticity, Size Proportions	DEPTH (ft)	SYMBOL	DEPTH SCALE	Cond	Blows/6"	No.	Туре	Rec (in)	BORING & SAMPLE NOTES	
	533.8	4" of TOPSOIL Brown, moist, medium stiff, lean CLAY, with sand, trace organics in top foot	0.3	<u> </u>		ı	2-3-3	1	DS	12	No water encountered. Grab bag of topsoil taken.	
_	532.2	Tan with gray, moist, medium dense to very dense, silty, fine to medium SAND, trace gravel and quartzites	2.0			I/D	5-9-11	2	DS	15	3. Boring drilled using a Simco 2800 All Terrain Vehicle (ATV) mounted drill rig with safety hammer. 4. Boring Coordinates:	_
 - -					<u>5</u> -	D	21-32-26	3	DS	15	N: 579349.6995 E: 1226951.8956	
- -	524.2	Bottom of Boring at 10.0 ft	10.0		_ _ 	D	15-15-13	4	DS	10		_
_ _ _	-				_							_
_					<u>15</u>							_
					_							_
		AMPLED TYPE			20						DING METHOD	_

SAMPLER TYPE

DS - DRIVEN SPLIT SPOON

PT - PRESSED SHELBY TUBE
CA - CONTINUOUS FLIGHT AUGER RC - ROCK CORE

SAMPLE CONDITIONS

D - DISINTEGRATED I - INTACT

U - UNDISTURBED L - LOST

GROUNDWATER DEPTH AT COMPLETION dry ft
AFTER HRS. ft
CAVED AT 8.0 ft

BORING METHOD

HSA - HOLLOW STEM AUGERS CFA - CONTINUOUS FLIGHT AUGERS
DC - DRIVING CASING
MD - MUD DRILLING

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RECORD OF SOIL / ROCK EXPLORATION

Contracted With _	A. Morton Thomas & Associates, Inc.	Boring #	B-8	
Project Name	Little Bennett Regional Park	Job #	10-030-A	
Location	Clarksburg, MD			

Location ___

SAMPLER

Datum	Hammer Wt140 lb	Hole Diameter 8 in	Foreman W. Seward
Surf. Elev549.1 ft	Hammer Drop30 in	Rock Core Dia. N/A	Inspector K. Bullock
Date Started5/24/11	Spoon Size 2 in	Boring MethodHSA	Date Completed5/24/11

ELE) (SOIL DESCRIPTION	STRA	٦٦	王叫		SAM	MPLE			DODING & CAMPLE]
ELEV. (ft)	Color, Moisture, Density, Plasticity, Size Proportions	STRA DEPTH (ft)	SOIL	DEPTH SCALE	Cond	Blows/6"	No.	Туре	Rec (in)	BORING & SAMPLE NOTES	
548.5	7" of TOPSOIL Brown, moist, loose to medium dense, clayey SAND , trace organics in top	0.6	<u>1</u>		I/D	2-5-4	1	DS	10	No water encountered. 1 bag sample collected from 1.0 - 5.0 ft.	
544.6	foot	4.5			I/D	9-16-20	2	DS	18	3. Boring drilled using a Simco 2800 All Terrain Vehicle (ATV) mounted drill rig with safety hammer. 4. Boring Coordinates:	_
344.0	Reddish brown, moist, medium dense to dense, silty, fine to medium SAND , trace gravel and quartzites	4.0		5	1	12-12-10	3	DS	14	N: 579219.9220 E: 1226831.3011	
539.1	Bottom of Boring at 10.0 ft	10.0		10_	D	14-11-20	4	DS	10		_
_				_							_
_				15							_
_				_							
				20							

SAMPLER TYPE

DS - DRIVEN SPLIT SPOON PT - PRESSED SHELBY TUBE CA - CONTINUOUS FLIGHT AUGER

RC - ROCK CORE

SAMPLE CONDITIONS D - DISINTEGRATED

I - INTACT U - UNDISTURBED L - LOST

GROUNDWATER DEPTH AT COMPLETION dry ft
AFTER HRS. ft
CAVED AT 6.0 ft **BORING METHOD**

HSA - HOLLOW STEM AUGERS CFA - CONTINUOUS FLIGHT AUGERS
DC - DRIVING CASING
MD - MUD DRILLING

RECORD OF SOIL EXPLORATION LOGS.GPJ PROJECT.GDT 7/28/11

T.L.B. ASSOCIATES, INC.

Glen Burnie, Maryland 21061

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RECORD OF SOIL / ROCK EXPLORATION

Contracted With	A. Morton Thomas & Associates, Inc.	Borina#_	B-9
Project Name	Little Bennett Regional Park	Job #	10-030-A
Location	Clarksburg, MD		

SAMPLER

Datum	Hammer Wt140 lb	Hole Diameter 8 in	ForemanW. Seward
Surf. Elev 558.5 ft	Hammer Drop 30 in	Rock Core Dia. N/A	Inspector K. Bullock
Date Started5/24/11	Spoon Size 2 in	Boring Method HSA	Date Completed5/24/11

	SOIL DESCRIPTION	STRA	٦.	ΞШ		SAI	MPLE]
ELEV. (ft)	Color, Moisture, Density, Plasticity, Size Proportions	STRA DEPTH (ft)	SYMBOL	DEPTH SCALE	Cond	Blows/6"	No.	Туре	Rec (in)	BORING & SAMPLE NOTES	
558.1	5" of TOPSOIL Red, moist, medium stiff, lean CLAY	0.4	<u> </u>		ı	2-4-4	1	DS	10	No water encountered. Grab bag of topsoil taken.	_
556.5	Red, moist, very stiff, sandy, lean CLAY, trace gravel	2.0		5	I/D	8-11-13	2	DS	12	 Boring drilled using a Simco 2800 All Terrain Vehicle (ATV) mounted drill rig with safety hammer. Boring Coordinates: 578993.9963 1226895.8472 	_
553.0	Red, moist, medium dense to dense, silty, fine to medium SAND , trace gravel and mica	5.5			D	10-13-15	3	DS	12	E. 1220090.0472	_
548.5	Bottom of Boring at 10.0 ft	10.0		10_	ı	11-16-20	4	DS	10		_
				_							_
				 -							

SAMPLER TYPE

DS - DRIVEN SPLIT SPOON

PT - PRESSED SHELBY TUBE
CA - CONTINUOUS FLIGHT AUGER RC - ROCK CORE

SAMPLE CONDITIONS D - DISINTEGRATED I - INTACT

U - UNDISTURBED L - LOST

GROUNDWATER DEPTH AT COMPLETION dry ft
AFTER HRS. ft
AFTER 24 HRS. ft
CAVED AT 8.5 ft

BORING METHOD HSA - HOLLOW STEM AUGERS

CFA - CONTINUOUS FLIGHT AUGERS
DC - DRIVING CASING
MD - MUD DRILLING

Contracted With	A. Morton Thomas & Associates, Inc.	Boring # _	B-10
Project Name	Little Bennett Regional Park	Job#	10-030-A
Location	Clarksburg, MD		

Location ___

SAMPLER

Datum	Hammer Wt140 lb	Hole Diameter 8 in	ForemanW. Seward
Surf. Elev539.8 ft	Hammer Drop30 in	Rock Core Dia. N/A	Inspector K. Bullock
Date Started5/24/11	Spoon Size 2 in	Boring MethodHSA	Date Completed5/24/11

	SOIL DESCRIPTION	STDA	٦.	ΤШ		SAI	MPLE				٦
ELEV. (ft)	Color, Moisture, Density, Plasticity, Size Proportions	STRA DEPTH (ft)	SOIL	DEPTH SCALE	Cond	Blows/6"	No.	Туре	Rec (in)	BORING & SAMPLE NOTES	
539.5	4" of TOPSOIL	0.3	11/2. · <u>1</u> /1/2.							No water encountered.	ŀ
536.8	Reddish-brown, moist, medium stiff to stiff, lean CLAY , trace organics in top 6-inches	3.0		_	1	2-2-4	1	DS	10	Boring drilled using a Simco 2800 All Terrain Vehicle (ATV) mounted drill rig with safety hammer.	
330.0	Reddish-brown, moist, very stiff, sandy, lean CLAY	3.0			ı	3-7-9	2	DS	14	3. Boring Coordinates: N: 578665.6501 E: 1227037.9422	
				<u>5</u> - -	I/D	9-12-10	3	DS	12		
529.8		10.0		_ _ 10_	D	12-14-15	4	DS	10		
	Bottom of Boring at 10.0 ft			_							
				_							ŀ
				_							
				<u>15</u>							-
				_							
				_							
				_							
				_							
				20							

SAMPLER TYPE

DS - DRIVEN SPLIT SPOON

PT - PRESSED SHELBY TUBE CA - CONTINUOUS FLIGHT AUGER RC - ROCK CORE

SAMPLE CONDITIONS

D - DISINTEGRATED I - INTACT U - UNDISTURBED

L - LOST

GROUNDWATER DEPTH AT COMPLETION dry ft
AFTER HRS. ft
CAVED AT 7.0 ft **BORING METHOD**

HSA - HOLLOW STEM AUGERS CFA - CONTINUOUS FLIGHT AUGERS
DC - DRIVING CASING
MD - MUD DRILLING

Page 1 of 1

RECORD OF SOIL / ROCK EXPLORATION

Contracted With	A. Morton Thomas & Associates, Inc.	Boring # _	B-11
Project Name	Little Bennett Regional Park	Job #	10-030-A
Location	Clarksburg, MD		

SAMPLER

Datum	Hammer Wt140 lb	Hole Diameter 8 in	ForemanW. Seward
Surf. Elev533.0 ft	Hammer Drop30 in	Rock Core Dia. N/A	Inspector K. Bullock
Date Started5/24/11	Spoon Size2 in	Boring MethodHSA	Date Completed5/24/11

	SOIL DESCRIPTION	STDV	. പ	ΤШ		SAI	MPLE				7
ELEV. (ft)	Color, Moisture, Density, Plasticity, Size Proportions	STRA DEPTH (ft)	SOIL	DEPTH SCALE	Cond	Blows/6"	No.	Туре	Rec (in)	BORING & SAMPLE NOTES	
532.5	6" of TOPSOIL	0.5	711/2 711							No water encountered.	
	Brown to red, moist, medium stiff to very stiff, lean CLAY , trace organics			-	I	2-3-5	1	DS	10	Grab bag of topsoil taken.	F
_				_	I/D	5-8-9	2	DS	18	3. Boring drilled using a Simco 2800 All Terrain Vehicle (ATV) mounted drill rig with safety hammer.	_
528.0		5.0		5 5						4. Boring Coordinates: N: 578692.7561 E: 1227171.5647	
	Red, moist, very stiff to hard, sandy SILT , with quartzites			_	I/D	7-8-11	3	DS	18		
				_							
				_							
523.0		10.0		10_	D	18-21-18	4	DS	16		
	Bottom of Boring at 10.0 ft										
				_							
				_							H
				_							_
				<u>15</u>							
1				_							
-				_							
				_							
				20							

SAMPLER TYPE

DS - DRIVEN SPLIT SPOON PT - PRESSED SHELBY TUBE CA - CONTINUOUS FLIGHT AUGER

RC - ROCK CORE

SAMPLE CONDITIONS

D - DISINTEGRATED I - INTACT

U - UNDISTURBED L - LOST

GROUNDWATER DEPTH AT COMPLETION dry ft
AFTER HRS. ft
CAVED AT 7.0 ft

BORING METHOD

HSA - HOLLOW STEM AUGERS CFA - CONTINUOUS FLIGHT AUGERS
DC - DRIVING CASING
MD - MUD DRILLING

Contracted With	A. Morton Thomas & Associates, Inc.	Boring # _	B-12
Project Name	Little Bennett Regional Park	Job#	10-030-A
Location	Clarksburg, MD		

SAMPLER

Datum	Hammer Wt140 lb	Hole Diameter 8 in	Foreman W. Seward
Surf. Elev537.5 ft	Hammer Drop30 in	Rock Core Dia. N/A	Inspector K. Bullock
Date Started5/18/11	Spoon Size 2 in	Boring MethodHSA	Date Completed5/19/11

ELEV.	SOIL DESCRIPTION	STRA	ر ق	프끸		SAI	MPLE			BORING & SAMPLE
(ft)	Color, Moisture, Density, Plasticity, Size Proportions	DEPTH (ft)	SYMBOL	DEPTH SCALE	Cond	Blows/6"	No.	Туре	Rec (in)	NOTES
537.0	6" of TOPSOIL	0.5	7/1/2 7/1/							No water encountered.
535.0	Reddish brown with gray, moist, stiff, sandy, lean CLAY , with wood fragments	2.5		- -	I/D	2-3-8	1	DS	12	Boring drilled using a Simco 2800 All Terrain Vehicle (ATV) mounted drill rig with safety hammer.
	Reddish brown, moist, dense, silty, fine to medium SAND , trace gravel			-	D	9-19-12	2	DS	18	3. Boring Coordinates: N: 578475.6091 E: 1227266.9725
				<u>5</u> –	D	14-17-14	3	DS	18	
530.0	Reddish brown, moist, very stiff,	7.5		_						
	CLAY, with sand, trace mica			_		8-8-9	4	DS	18	
527.5	Bottom of Boring at 10.0 ft	10.0		10_						
	Bottom of Borning at 10.0 it			_						
				_						
				_						
				_						
				<u>15</u>						
				_						
				_						
				_						
				20						

SAMPLER TYPE

DS - DRIVEN SPLIT SPOON PT - PRESSED SHELBY TUBE

CA - CONTINUOUS FLIGHT AUGER RC - ROCK CORE

SAMPLE CONDITIONS

D - DISINTEGRATED I - INTACT

U - UNDISTURBED L - LOST

GROUNDWATER DEPTH

AT COMPLETION dry ft
AFTER HRS. ft
CAVED AT 8.3 ft

BORING METHOD

HSA - HOLLOW STEM AUGERS CFA - CONTINUOUS FLIGHT AUGERS
DC - DRIVING CASING
MD - MUD DRILLING

T.L.B. ASSOCIATES, INC.

Glen Burnie, Maryland 21061

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RECORD OF SOIL / ROCK EXPLORATION

Contracted With	A. Morton Thomas & Associates, Inc.	Boring# _	B-13
	Little Bennett Regional Park	Job#	10-030-A
Location	Clarksburg, MD		

SAMPLER

Datum	Hammer Wt140 lb	Hole Diameter 8 in	ForemanW. Seward
Surf. Elev542.0 ft	Hammer Drop30 in	Rock Core Dia. N/A	Inspector K. Bullock
Date Started5/17/11	Spoon Size2 in	Boring MethodHSA	Date Completed5/17/11

ELEV.	SOIL DESCRIPTION	STRA	J Z	王끡		SAI	MPLE			PODING & SAMDLE	
(ft)	Color, Moisture, Density, Plasticity, Size Proportions	STRA DEPTH (ft)	SOIL	DEPTH SCALE	Cond	Blows/6"	No.	Туре	Rec (in)	BORING & SAMPLE NOTES	
541.5	6" of TOPSOIL	0.5	\(\frac{1}{2}\frac{1}{2}\), \(\frac{1}{2}\frac{1}{2}\).							No water encountered.	
	Reddish-brown, moist, loose to medium dense, silty, fine SAND , with organics			· -	I	1-3-5	1	DS	15	Boring drilled using a Simco 2800 All Terrain Vehicle (ATV) mounted drill rig with safety hammer.	
538.5	Reddish-brown to tan, moist, medium	3.5		-	D	7-9-11	2	DS	15	3. Boring Coordinates: N: 578184.9968 E: 1227321.3083	
	dense, SAND , with some silt and gravel, quartzites			5						L. 1227 321.3003	
					ı	8-8-9	3	DS	18		
				_							L
				_	-						
				-	,	10-12-14	4	DS	18		_
532.0	Bottom of Boring at 10.0 ft	10.0		10_							
				_	-						_
				_							
				_	-						
				15							
				_	-						
				_							L
				_							
				_							
				20							

SAMPLER TYPE

DS - DRIVEN SPLIT SPOON PT - PRESSED SHELBY TUBE

CA - CONTINUOUS FLIGHT AUGER RC - ROCK CORE

SAMPLE CONDITIONS

D - DISINTEGRATED I - INTACT

U - UNDISTURBED L - LOST

GROUNDWATER DEPTH AT COMPLETION dry ft
AFTER HRS. ft
CAVED AT 7.0 ft

BORING METHOD

HSA - HOLLOW STEM AUGERS CFA - CONTINUOUS FLIGHT AUGERS
DC - DRIVING CASING
MD - MUD DRILLING

Contracted With	A. Morton Thomas & Associates, Inc.	Boring # _	B-14
Project Name	Little Bennett Regional Park	Job#	10-030-A
Location	Clarksburg, MD		

Location ___

SAMPLER

Datum	Hammer Wt140 lb	Hole Diameter 8 in	Foreman W. Seward
Surf. Elev. 544.4 ft	Hammer Drop30 in	Rock Core Dia. N/A	Inspector K. Bullock
Date Started 5/17/11	Spoon Size 2 in	Boring Method HSA	Date Completed5/17/11

ELEV.	SOIL DESCRIPTION	STRA	그럴	프핔		SAI	MPLE			BORING & SAMPLE	
(ft)	Color, Moisture, Density, Plasticity, Size Proportions	DEPTH (ft)	SYMBOL	DEPTH SCALE	Cond	Blows/6"	No.	Туре	Rec (in)	NOTES	
544.0	5" of TOPSOIL	0.4	71 1/2 VI							No water encountered.	H
	Reddish-brown, moist, loose, silty, fine SAND , with organics			-	1	2-3-3	1	DS	15	Grab bag of topsoil taken.	
544.0		0.5		_						Boring drilled using a	-
541.9	Reddish-brown, moist, medium stiff to hard, CLAY , with fine sand	2.5		-	ı	2-3-4	2	DS	15	Simco 2800 All Terrain Vehicle (ATV) mounted drill rig with safety hammer.	
				5						4. Boring Coordinates: N: 578090.2502 E: 1227271.3937	
				-	I/D	5-6-8	3	DS	6		
				_							
534.4		10.0		10	I/D	11-17-22	4	DS	18		
554.4	Bottom of Boring at 10.0 ft	10.0	<i>\````</i>	10_							ł
				_							
				_							
				_							-
				_							-
				<u>15</u>							
				_							
				_							
				_							
				_							
				20							

SAMPLER TYPE

DS - DRIVEN SPLIT SPOON PT - PRESSED SHELBY TUBE CA - CONTINUOUS FLIGHT AUGER

RC - ROCK CORE

SAMPLE CONDITIONS D - DISINTEGRATED

I - INTACT U - UNDISTURBED L - LOST

AT COMPLETION dry ft
AFTER HRS. ft
CAVED AT 7.0 ft

GROUNDWATER DEPTH

BORING METHOD

HSA - HOLLOW STEM AUGERS CFA - CONTINUOUS FLIGHT AUGERS
DC - DRIVING CASING
MD - MUD DRILLING

Location ___

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RECORD OF SOIL / ROCK EXPLORATION

Contracted With	A. Morton Thomas & Associates, Inc.	Boring#_	B-15
Project Name	Little Bennett Regional Park	Job#	10-030-A
Location	Clarksburg, MD		

SAMPLER

Datum	Hammer Wt140 lb	Hole Diameter 8 in	ForemanW. Seward
Surf. Elev535.7 ft	Hammer Drop 30 in	Rock Core Dia. N/A	Inspector K. Bullock
Date Started5/16/11	Spoon Size 2 in	Boring MethodHSA	Date Completed5/16/11

	SOIL DESCRIPTION	STRA	٦٩	王끡		SAI	MPLE			DODING & CAMPLE
ELEV. (ft)	Color, Moisture, Density, Plasticity, Size Proportions	STRA DEPTH (ft)	SYMBOL	DEPTH SCALE	Cond	Blows/6"	No.	Туре	Rec (in)	BORING & SAMPLE NOTES
535.3	5" of TOPSOIL Brown, moist, medium stiff, lean	0.4	<u> </u>			1-3-4	1	DS	12	No water encountered. 2 jars of topsoil taken.
533.2	CLAY, trace organics	2.5								Boring drilled using a Simco 2800 All Terrain Vehicle (ATV) mounted
	Gray with white, moist, stiff to very stiff, sandy CLAY , trace quartzites			_	ı	5-5-7	2	DS	16	drill rig with safety hammer.
				<u>5</u>						4. Boring Coordinates: N: 578029.3701 E: 1227479.5631
				-	ı	7-8-10	3	DS	15	
				_						
				_	ı	7-11-16	4	DS	16	
525.7	Bottom of Boring at 10.0 ft	10.0		10_						
				_						
				_						
				<u>15</u>						
				_						
				_						
				_						
				20						

SAMPLER TYPE

DS - DRIVEN SPLIT SPOON

PT - PRESSED SHELBY TUBE
CA - CONTINUOUS FLIGHT AUGER RC - ROCK CORE

D - DISINTEGRATED I - INTACT U - UNDISTURBED L - LOST

SAMPLE CONDITIONS

GROUNDWATER DEPTH

AT COMPLETION dry ft
AFTER HRS. ft
AFTER 24 HRS. ft
CAVED AT 7.5 ft

BORING METHOD

HSA - HOLLOW STEM AUGERS CFA - CONTINUOUS FLIGHT AUGERS
DC - DRIVING CASING
MD - MUD DRILLING

Page 1 of 1

RECORD OF SOIL / ROCK EXPLORATION

Contracted With _	A. Morton Thomas & Associates, Inc.	Boring #	B-16	
Project Name	Little Bennett Regional Park	Job #	10-030-A	
Location	Clarksburg, MD			

Location ___

SAMPLER

Datum	Hammer Wt. 140 lb	Hole Diameter 8 in	Foreman W. Seward
Surf. Elev. 538.3 ft	Hammer Drop30 in	Rock Core Dia. N/A	Inspector K. Bullock
Date Started5/16/11	Spoon Size 2 in	Boring MethodHSA	Date Completed <u>5/16/11</u>

ELE) (SOIL DESCRIPTION	STRA	٦٦	프백		SAI	MPLE			DODING & CAMPLE]
ELEV. (ft)	Color, Moisture, Density, Plasticity, Size Proportions	STRA DEPTH (ft)	SYMBOL	DEPTH SCALE	Cond	Blows/6"	No.	Туре	Rec (in)	BORING & SAMPLE NOTES	
537.6	8" of TOPSOIL	0.7	7/1/2	П						No water encountered.	
	Brown, moist, medium stiff to very stiff, lean CLAY , with sand and organics	0.7		_	-	1-2-3	1	DS	8	2. 2 jars of of topsoil taken. 3. Boring drilled using a Simco 2800 All Terrain Vehicle (ATV) mounted	
				-	ı	2-4-6	2	DS	8	drill rig with safety hammer.	_
				_ <u>5</u>						4. Boring Coordinates: N: 577929.5411 E: 1227463.8440	
	To gray with white, trace mica and quartzites			-	ı	7-8-10	3	DS	18		_
				_	-						
528.3		10.0		- 10	ı	8-11-13	4	DS	18		-
520.3	Bottom of Boring at 10.0 ft	10.0	<i>\//////</i>		-						
				_	-						
				_	-						
				<u>15</u>	-						
				_	-						
				_	_						
-				20							

SAMPLER TYPE

DS - DRIVEN SPLIT SPOON PT - PRESSED SHELBY TUBE

CA - CONTINUOUS FLIGHT AUGER RC - ROCK CORE

SAMPLE CONDITIONS

D - DISINTEGRATED I - INTACT

U - UNDISTURBED L - LOST

GROUNDWATER DEPTH

AT COMPLETION dry ft
AFTER HRS. ft
CAVED AT 7.0 ft

BORING METHOD

HSA - HOLLOW STEM AUGERS CFA - CONTINUOUS FLIGHT AUGERS
DC - DRIVING CASING
MD - MUD DRILLING

Location ___

Page 1 of 1

RECORD OF SOIL / ROCK EXPLORATION

Contracted With	A. Morton Thomas & Associates, Inc.	Boring#_	B-17
Project Name	Little Bennett Regional Park	Job#	10-030-A
Location	Clarksburg, MD		

SAMPLER

Datum	Hammer Wt140 lb	Hole Diameter 8 in	ForemanW. Seward
Surf. Elev547.3 ft	Hammer Drop30 in	Rock Core Dia. N/A	Inspector K. Bullock
Date Started5/16/11	Spoon Size2 in	Boring MethodHSA	Date Completed5/16/11

	ELE) /	SOIL DESCRIPTION	STRA		돈백		SAI	MPLE			DODING A GAMBLE]
	ELEV. (ft)	Color, Moisture, Density, Plasticity, Size Proportions	DEPTH (ft)	SOIL	DEPTH SCALE	Cond	Blows/6"	No.	Туре	Rec (in)	BORING & SAMPLE NOTES	
	546.9	5" of TOPSOIL	0.4	7/1/N. 7/1/							No water encountered.	
_		Brown, moist, loose, clayey SAND , with silt and organics			-	D	2-3-5	1	DS	14	Grab bag of topsoil taken.	
_	545.3	Tan, moist, very stiff, SILT , with sand, trace mica,	2.0		_						3. 1 bag sample collected from 1.5 - 5.0 ft.	_
	540.0	(USCS: ML; USDA: SILT LOAM)	5.0			D	6-9-9	2	DS	18	Boring drilled using a Simco 2800 All Terrain Vehicle (ATV) mounted drill rig with safety hammer.	
_	542.3	White, moist, medium dense, silty, fine SAND , with gravel and quartzites	5.0			I/D	8-12-12	3	DS	18	5. Boring Coordinates: N: 577823.3396 E: 1227632.7622	
_												_
_		Calcium staining observed	40.0			ı	11-15-15	4	DS	12		_
	537.3	Bottom of Boring at 10.0 ft	10.0		10_							
_					_							
					_							
_					_							
					<u>15</u>							
_					-							
_					-							
_					_							
_					-							
<u></u>		AMBI ED TABE			20			<u> </u>				

SAMPLER TYPE

DS - DRIVEN SPLIT SPOON

PT - PRESSED SHELBY TUBE CA - CONTINUOUS FLIGHT AUGER RC - ROCK CORE

SAMPLE CONDITIONS D - DISINTEGRATED I - INTACT U - UNDISTURBED

L - LOST

AT COMPLETION dry ft
AFTER HRS. ft
CAVED AT 7.0 ft

GROUNDWATER DEPTH

BORING METHOD

HSA - HOLLOW STEM AUGERS CFA - CONTINUOUS FLIGHT AUGERS
DC - DRIVING CASING
MD - MUD DRILLING

Location ___

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RECORD OF SOIL / ROCK EXPLORATION

Contracted With	A. Morton Thomas & Associates, Inc.	Boring#	B-18
Project Name	Little Bennett Regional Park	Job#	10-030-A
Location	Clarksburg, MD		

SAMPLER

Datum	Hammer Wt. 140 lb	Hole Diameter 8 in	Foreman W. Seward
Surf. Elev. 503.4 ft	Hammer Drop 30 in	Rock Core Dia. N/A	Inspector K. Bullock
Date Started 5/26/11	Spoon Size 2 in	Boring Method HSA	Date Completed 5/26/11

ELEV.	SOIL DESCRIPTION	STRA	70	Ξ끡		SAM	MPLE			DODING & CAMPLE]
(ft)	Color, Moisture, Density, Plasticity, Size Proportions	DEPTH (ft)	SYMBOL	DEPTH SCALE	Cond	Blows/6"	No.	Туре	Rec (in)	BORING & SAMPLE NOTES	
502.9	6" of TOPSOIL	0.5	7/1/2 1/1/							No water encountered.	
	Brown, moist, loose, silty SAND , trace organics and mica, some cementation observed			-	ı	2-3-4	1	DS	10	Grab bag of topsoil taken.	-
500.9	Reddish-brown, moist, medium dense to very dense, silty SAND , with rock fragments, trace mica	2.5		-	D	9-9-11	2	DS	12	3. Boring drilled using a Simco 2800 All Terrain Vehicle (ATV) mounted drill rig with safety hammer. 4. Boring Coordinates: N: 579873.4144	_
				J -	D	12-12-12	3	DS	18	E: 1227589.8721	
					I/D	16-15-21	4	DS	18		_
	(SAPROLITE)			10	D	20-34-51/4"	5	DS	10		
491.4	Reddish-brown, moist, hard, CLAY, trace rock fragments, (SAPROLITE)	12.0		_							_
488.4	Bottom of Boring at 15.0 ft	15.0		_ 15	D	40-51/4"	6	DS	10		
				_							-
				_							
				_							
		1	1	1				i .			

SAMPLER TYPE

SAMPLE CONDITIONS D - DISINTEGRATED

GROUNDWATER DEPTH

BORING METHOD

DS - DRIVEN SPLIT SPOON PT - PRESSED SHELBY TUBE CA - CONTINUOUS FLIGHT AUGER

I - INTACT U - UNDISTURBED L - LOST

STANDARD PENETRATION TEST DRIVING 2" OD SAMPLER 1' WITH 140# HAMMER FALLING 30": COUNT MADE AT 6" INTERVALS

AT COMPLETION _______ft AFTER ________ ft AFTER 24 HRS. _______ ft CAVED AT ________ft

HSA - HOLLOW STEM AUGERS CFA - CONTINUOUS FLIGHT AUGERS
DC - DRIVING CASING
MD - MUD DRILLING

RECORD OF SOIL EXPLORATION LOGS.GPJ PROJECT.GDT 7/28/11

Contracted With	A. Morton Thomas & Associates, Inc.	Boring#_	B-19
Project Name	Little Bennett Regional Park	Job#	10-030-A
Location	Clarksburg, MD		

Location ___

SAMPLER

Datum	Hammer Wt140 lb	Hole Diameter 8 in	ForemanW. Seward
Surf. Elev535.9 ft	Hammer Drop30 in	Rock Core Dia. N/A	Inspector K. Bullock
Date Started5/26/11	Spoon Size 2 in	Boring Method HSA	Date Completed5/26/11

	FI FV	SOIL DESCRIPTION	STRA	79	王끡		SAM	MPLE			DODING & CAMPLE]
	ELEV. (ft)	Color, Moisture, Density, Plasticity, Size Proportions	DEPTH (ft)	SOIL	DEPTH SCALE	Cond	Blows/6"	No.	Туре	Rec (in)	BORING & SAMPLE NOTES	
_	535.4	6" of TOPSOIL Brown, moist, loose, silty SAND , trace gravel and organics	0.5		_	D	2-2-3	1	DS	18	No water encountered. Boring drilled using a Simco 2800 All Terrain Vehicle (ATV) mounted	_
-	-	Tan to brown, moist, medium dense, silty SAND , minor cementation observed	2.0		_	D	9-8-9	2	DS	16	drill rig with safety hammer. 3. Boring Coordinates: N: 579256.6527 E: 1227520.9029	_
	528.9	Some quartzites at 5 ft.	7.0		<u>5</u>	D	11-9-10	3	DS	14		_
_		Brown, moist, very stiff to hard, CLAY, some sand, trace rock fragments, (SAPROLITE)			_ _ _	D	12-14-18	4	DS	18		_
- <u>-</u>					<u>10</u> -	D	25-22-18	5	DS	18		
_	520.9		15.0		_ - 15	D	20-21-24	6	DS	18		_
_		Bottom of Boring at 15.0 ft			_							_
					_							_
_												_
<u> </u>		AMBI ED TADE			20							

SAMPLER TYPE

SAMPLE CONDITIONS D - DISINTEGRATED

GROUNDWATER DEPTH

BORING METHOD

DS - DRIVEN SPLIT SPOON PT - PRESSED SHELBY TUBE CA - CONTINUOUS FLIGHT AUGER

I - INTACT U - UNDISTURBED L - LOST

AT COMPLETION dry ft
AFTER HRS. ft
CAVED AT 12.3 ft

HSA - HOLLOW STEM AUGERS CFA - CONTINUOUS FLIGHT AUGERS
DC - DRIVING CASING
MD - MUD DRILLING

RECORD OF SOIL EXPLORATION LOGS.GPJ PROJECT.GDT 7/27/11

Location ___

T.L.B. ASSOCIATES, INC.

Glen Burnie, Maryland 21061

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RECORD OF SOIL / ROCK EXPLORATION

Contracted With _	A. Morton Thomas & Associates, Inc.	Borina #	B-20	
Project Name	Little Bennett Regional Park	Job#	10-030-A	
Location	Clarksburg, MD			

SAMPLER

Datum	Hammer Wt. 140 lb	Hole Diameter 8 in	Foreman W. Seward
Surf. Elev. 512.3 ft	Hammer Drop 30 in	Rock Core Dia. N/A	Inspector K. Bullock
Date Started 5/17/11	Spoon Size 2 in	Boring Method HSA	Date Completed5/17/11

	OOU DECODIDATION	OTD A	٦	T		SAN	//PLE				٦
ELEV. (ft)	SOIL DESCRIPTION Color, Moisture, Density, Plasticity, Size Proportions	STRA DEPTH (ft)	SOIL	DEPTH SCALE	Cond	Blows/6"	No.	Туре	Rec (in)	BORING & SAMPLE NOTES	
511.8	6" of TOPSOIL	0.5	7/1/2 7/1/							No water encountered.	
510.8	Brown, moist, medium stiff, lean CLAY, with organics	1.5		-	D	2-4-3	1	DS	8	Grab bag of topsoil taken.	_
	Reddish-brown, moist, stiff to hard, CLAY, with sand, trace quartzites			_	I	4-6-9	2	DS	18	3. Boring drilled using a Simco 2800 All Terrain Vehicle (ATV) mounted drill rig with safety hammer.	
				<u>5</u>	ı	8-9-10	3	DS	18	4. Boring Coordinates: N: 578495.3516 E: 1227970.7900	
				_							
				10	I/D	10-12-13	4	DS	12		
				<u>10</u> –	D	13-21-23	5	DS	8		
				_							
497.3		15.0		- 15	I/D	19-23-29	6	DS	18		
	Bottom of Boring at 15.0 ft	10.0	12////								
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				_							\vdash
				_							F
				_							
				20							

SAMPLER TYPE

SAMPLE CONDITIONS D - DISINTEGRATED

GROUNDWATER DEPTH

BORING METHOD

DS - DRIVEN SPLIT SPOON PT - PRESSED SHELBY TUBE CA - CONTINUOUS FLIGHT AUGER

I - INTACT U - UNDISTURBED L - LOST

AT COMPLETION dry ft
AFTER HRS. ft
AFTER 24 HRS. ft
CAVED AT 7.5 ft

HSA - HOLLOW STEM AUGERS CFA - CONTINUOUS FLIGHT AUGERS
DC - DRIVING CASING
MD - MUD DRILLING

RC - ROCK CORE

Contracted With	A. Morton Thomas & Associates, Inc.	Boring#_	IT-2
	Little Bennett Regional Park	Job #	10-030-A

Clarksburg, MD Location __

SAMPLER

Datum	Hammer Wt140 lb	Hole Diameter 8 in	ForemanW. Seward
Surf. Elev493.2 ft			Inspector K. Bullock
Date Started5/25/11	Spoon Size 2 in	Boring Method HSA	Date Completed5/25/11

	SOIL DESCRIPTION STRA 그렇 돈띡 SAMPLE							DODING & CAMPLE			
ELEV. (ft)	Color, Moisture, Density, Plasticity, Size Proportions	DEPTH (ft)	SOIL	DEPTH SCALE	Cond	Blows/6"	No.	Туре	Rec (in)	BORING & SAMPLE NOTES	
492.8	5" of TOPSOIL	0.4	<u> </u>							No water encountered.	
	Reddish brown, moist, medium stiff, sandy, lean CLAY , trace organics			-	D	2-3-5-6	1	DS	16	Grab bag of topsoil taken.	_
490.7		2.5		_						3. 4-inch PVC pipe	H
490.7	Tan and gray-brown and red, moist, medium dense, silty, fine to medium SAND, trace gravel	2.5			I/D	7-10-10-13	2	DS	18	installed in offset borehole at a depth of 5.0 ft. for infiltration test.	
489.2	Reddish brown, moist, medium dense,	4.0								4. Boring drilled using a Simco 2800 All Terrain	H
	silty ROCK FRAGMENTS, with sand, (USCS: GM; USDA: SANDY LOAM) Average Infiltration Rate = 0.93 in/hr			5	D	12-12-11-12	3	DS	20	Vehicle (ATV) mounted drill rig with safety hammer.	
	(SAPROLITE)									5. Boring Coordinates: N: 579938.5781	H
485.7	Red and tan, moist, medium dense.	7.5			D	4-7-7-6	4	DS	24	E: 1227685.5524	
	silty SAND, (SAPROLITE)										
				-	D	9-10-10-16	5	DS	18		
483.2	Bottom of Boring at 10.0 ft	10.0	(018M0)	10_							
				_							
				_							
				_							F
				_							
				<u>15</u>							
				_							
				_	_						
				_							
				_							

SAMPLER TYPE

DS - DRIVEN SPLIT SPOON

PT - PRESSED SHELBY TUBE CA - CONTINUOUS FLIGHT AUGER RC - ROCK CORE

SAMPLE CONDITIONS

D - DISINTEGRATED I - INTACT

U - UNDISTURBED L - LOST

GROUNDWATER DEPTH

AT COMPLETION dry ft
AFTER HRS. ft
AFTER 24 HRS. ft
CAVED AT 8.5 ft

BORING METHOD

HSA - HOLLOW STEM AUGERS CFA - CONTINUOUS FLIGHT AUGERS
DC - DRIVING CASING
MD - MUD DRILLING

T.L.B. ASSOCIATES, INC.

Glen Burnie, Maryland 21061

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RECORD OF SOIL / ROCK EXPLORATION

Contracted With	A. Morton Thomas & Associates, Inc.	Borina#_	IT-4
Project Name	Little Bennett Regional Park	Job #	10-030-A
Location	Clarksburg, MD		

SAMPLER

Datum	Hammer Wt140 lb	Hole Diameter 8 in	ForemanW. Seward
Surf. Elev538.4 ft	Hammer Drop30 in	Rock Core Dia. N/A	Inspector K. Bullock
Date Started5/24/11	Spoon Size 2 in	Boring MethodHSA	Date Completed5/24/11

EL EV	SOIL DESCRIPTION	STRA	79	포백		SAN	/IPLE			DODING A GAMPI F]
ELEV. (ft)	Color, Moisture, Density, Plasticity, Size Proportions	DEPTH (ft)	SOIL	DEPTH	Cond	Blows/6"	No.	Туре	Rec (in)	BORING & SAMPLE NOTES	
537.9	6" of TOPSOIL	0.5	7/1/2 1/1/							No water encountered.	
-	Brown, moist, soft to very stiff, lean CLAY, trace sand and organics in top foot			_	ı	3-3-6-5	1	DS	13	4-inch PVC pipe installed in offset borehole at a depth of 6.0 ft. for infiltration test.	_
-					I/D	0-1-2-2	2	DS	10	3. Boring drilled using a Simco 2800 All Terrain Vehicle (ATV) mounted drill rig with safety hammer.	_
532.4		6.0		<u>5</u> _	I/D	5-12-10-15	3	DS	16	4. Boring Coordinates: N: 579322.0863 E: 1226893.2995	
-	Tan and gray, moist, medium dense, silty, fine SAND, with rock fragments, (USCS: SM; USDA: LOAM) Average Infiltration Rate = <0.12 in/hr				D	5-9-11-9	4	DS	16		
_		10.0			D	10-10-11-20	5	DS	17		_
528.4	Bottom of Boring at 10.0 ft	10.0		10_							_
				_	_						_
				_	_						_
											_
				_	_						_
-				<u>15</u>							
				_	_						_
				_	_						
-				_	-						
				_	_						
				20							

SAMPLER TYPE

DS - DRIVEN SPLIT SPOON PT - PRESSED SHELBY TUBE

CA - CONTINUOUS FLIGHT AUGER RC - ROCK CORE

SAMPLE CONDITIONS

D - DISINTEGRATED I - INTACT

U - UNDISTURBED L - LOST

GROUNDWATER DEPTH

AT COMPLETION dry ft
AFTER HRS. ft
CAVED AT 7.0 ft

BORING METHOD

HSA - HOLLOW STEM AUGERS CFA - CONTINUOUS FLIGHT AUGERS
DC - DRIVING CASING
MD - MUD DRILLING

Location __

Page 1 of 1

RECORD OF SOIL / ROCK EXPLORATION

Contracted With	A. Morton Thomas & Associates, Inc.	Boring# _	IT-6
Project Name	Little Bennett Regional Park	Job#	10-030-A
Location	Clarksburg, MD		

SAMPLER

Datum	Hammer Wt140 lb	Hole Diameter 8 in	Foreman W. Seward
Surf. Elev 551.1 ft	Hammer Drop30 in	Rock Core Dia. N/A	Inspector K. Bullock
Date Started5/24/11	Spoon Size 2 in	Boring Method HSA	Date Completed5/24/11

	SOIL DESCRIPTION	STRA	٦.	ΞЩ		SAN	ИPLE]
ELEV. (ft)	Color, Moisture, Density, Plasticity, Size Proportions	DEPTH (ft)	SYMBOL	DEPTH	Cond	Blows/6"	No.	Туре	Rec (in)	BORING & SAMPLE NOTES	
550.7	5" of TOPSOIL	0.4	7/1/2 V/1	7						No water encountered.	
549.1	Reddish-brown, moist, loose, silty, fine SAND , trace organics	2.0		_	I/D	3-4-5-8	1	DS	14	4-inch PVC pipe installed in offset borehole at a depth of 5.0 ft. for infiltration test.	_
	Red, moist, medium dense, silty SAND, with rock fragments, (USCS: SM; USDA: SANDY LOAM)				I/D	7-10-11-10	2	DS	22	Boring drilled using a Simco 2800 All Terrain Vehicle (ATV) mounted drill rig with safety hammer.	
	Average Infiltration Rate = 3.78 in/hr			<u>5</u>	I/D	7-10-10-11	3	DS	24	4. Boring Coordinates: N: 579177.0651 E: 1227140.6428	
E42.4		8.0			D	9-13-14-14	4	DS	20		
543.1	Red, moist, hard, CLAY , some rock fragments	8.0	1111111	 	D	11-20-32-37	5	DS	24		
541.1		10.0		10							
	Bottom of Boring at 10.0 ft		1								
_				_	_						L
				_	-						F
				_	-						F
_				_	_						_
				<u>15</u>	_						L
				_	-						
				_	_						L
				_							L
				_	_						L
				20							

SAMPLER TYPE

DS - DRIVEN SPLIT SPOON

PT - PRESSED SHELBY TUBE CA - CONTINUOUS FLIGHT AUGER RC - ROCK CORE

SAMPLE CONDITIONS D - DISINTEGRATED

I - INTACT U - UNDISTURBED L - LOST

GROUNDWATER DEPTH AT COMPLETION dry ft
AFTER HRS. ft
AFTER 24 HRS. ft
CAVED AT 8.0 ft **BORING METHOD**

HSA - HOLLOW STEM AUGERS CFA - CONTINUOUS FLIGHT AUGERS
DC - DRIVING CASING
MD - MUD DRILLING

T.L.B. ASSOCIATES, INC.

Glen Burnie, Maryland 21061

Page 1 of 1

RECORD OF SOIL / ROCK EXPLORATION

Contracted With	A. Morton Thomas & Associates, Inc.	Borina#_	IT-8
Project Name	Little Bennett Regional Park	Job#	10-030-A
Location	Clarksburg, MD		

SAMPLER

Datum	Hammer Wt140 lb	Hole Diameter 8 in	ForemanW. Seward
Surf. Elev529.9 ft	Hammer Drop30 in	Rock Core Dia. N/A	Inspector K. Bullock
Date Started5/24/11	Spoon Size 2 in	Boring Method HSA	Date Completed5/24/11

Γ		SOIL DESCRIPTION	STRA	٦.	ΞШ		SAN	MPLE				
	ELEV. (ft)	Color, Moisture, Density, Plasticity, Size Proportions	DEPTH (ft)	SYMBOL	DEPTH	Cond	Blows/6"	No.	Туре	Rec (in)	BORING & SAMPLE NOTES	
+	529.6	_ 4" of TOPSOIL	0.3	.71 1×. 1/1	ļ —						No water encountered.	
		Brown, moist, medium stiff to stiff, lean CLAY, with sand			-	I	2-3-4-5	1	DS	18	4-inch PVC pipe installed in offset borehole at a depth of 4.0 ft. for infiltration test.	_
	527.4	Reddish brown, moist, medium dense, clayey SAND , with silt, trace gravel	4.0			ı	2-8-6-8	2	DS	16	3. Boring drilled using a Simco 2800 All Terrain Vehicle (ATV) mounted drill rig with safety	_
\top	020.0	Reddish brown, moist, stiff to very	7.0								hammer.	_
		stiff, sandy SILT, (USCS: ML; USDA: LOAM) Average Infiltration Rate = 1.35 in/hr			5_	I	4-5-6-8	3	DS	13	4. Boring Coordinates: N: 578636.9381 E: 1227209.4095	
		Trace gravel and quartzites at 5.0 ft										
-						I/D	8-11-13-14	4	DS	24	-	_
	521.9		8.0									
		Reddish brown, moist, hard, CLAY , with fine sand				ı	15-18-20-25	5	DS	24		_
	519.9		10.0		10							
		Bottom of Boring at 10.0 ft										
-					_							_
					_							_
_					_						-	_
					_							
-					15							
					_							_
\dashv					-						-	_
					_							_
+					_							_
					20							

SAMPLER TYPE

DS - DRIVEN SPLIT SPOON PT - PRESSED SHELBY TUBE

CA - CONTINUOUS FLIGHT AUGER RC - ROCK CORE

D - DISINTEGRATED I - INTACT U - UNDISTURBED

L - LOST

SAMPLE CONDITIONS

GROUNDWATER DEPTH AT COMPLETION dry ft
AFTER HRS. ft
AFTER 24 HRS. ft
CAVED AT 8.0 ft **BORING METHOD**

HSA - HOLLOW STEM AUGERS CFA - CONTINUOUS FLIGHT AUGERS
DC - DRIVING CASING
MD - MUD DRILLING

Contracted With	A. Morton Thomas & Associates, Inc.	Boring # _	IT-9
	130 0 00 1 10 1	Job#	10-030-A
Location	Clarksburg, MD		

Location ___

SAMPLER

Datum	Hammer Wt140 lb	Hole Diameter 8 in	ForemanW. Seward
Surf. Elev534.5 ft	Hammer Drop 30 in	Rock Core Dia. N/A	Inspector K. Bullock
Date Started5/16/11	Spoon Size 2 in	Boring Method HSA	Date Completed5/16/11

		SOIL DESCRIPTION	STRA	٦,	ΞЩ		SAM	MPLE				
	ELEV. (ft)	Color, Moisture, Density, Plasticity, Size Proportions	DEPTH (ft)	SOIL	DEPTH SCALE	Cond	Blows/6"	No.	Туре	Rec (in)	BORING & SAMPLE NOTES	
	534.0	6" of TOPSOIL	0.5	71 1/2 /1							No water encountered.	
_	-	Brown, moist, medium stiff, sandy SILT, trace organics in top 1.5 ft., (USCS: ML; USDA: SILTY LOAM)			_	I	1-2-3-2	1	DS	20	4-inch PVC pipe installed in offset borehole at a depth of 4.0 ft. for infiltration test.	_
_	530.0	Average Infiltration Rate = 3.51 in/hr	4.5	•		I	1-3-2-2	2	DS	18	3. Boring drilled using a Simco 2800 All Terrain Vehicle (ATV) mounted drill rig with safety hammer.	_
	530.0	Reddish-brown, moist, loose, clayey SAND			5		3-3-4-5	3	DS	24	Boring Coordinates: -	
_	529.0	Brown, moist, stiff to very stiff, lean CLAY	5.5								N: 578090.6041 E: 1227424.0709	
-						I	2-5-5-5	4	DS	12	_	_
	526.0	Reddish-brown, moist, medium	8.5									
_		dense, silty, fine SAND , iron staining observed				I	9-9-10-12	5	DS	24		_
	524.5	Bottom of Boring at 10.0 ft	10.0		10_							
-	_				_							_
_	-				_							_
_	-				-							_
_					-							_
					15							
_					_							_
_	-				-						-	_
-					_							_
- Kor												
1068.64	1				_							_
5					20							

SAMPLER TYPE

DS - DRIVEN SPLIT SPOON PT - PRESSED SHELBY TUBE
CA - CONTINUOUS FLIGHT AUGER

RC - ROCK CORE

SAMPLE CONDITIONS

D - DISINTEGRATED I - INTACT

U - UNDISTURBED L - LOST

GROUNDWATER DEPTH

AT COMPLETION dry ft
AFTER HRS. ft
AFTER 24 HRS. ft
CAVED AT 8.0 ft

BORING METHOD

HSA - HOLLOW STEM AUGERS CFA - CONTINUOUS FLIGHT AUGERS
DC - DRIVING CASING
MD - MUD DRILLING

Location ___

Page 1 of 1

RECORD OF SOIL / ROCK EXPLORATION

Contracted With	A. Morton Thomas & Associates, Inc.	Boring # _	IT-10
Project Name	Little Bennett Regional Park	Job#	10-030-A
Location	Clarksburg, MD		

SAMPLER

Datum	Hammer Wt140 lb	Hole Diameter 8 in	ForemanW. Seward
Surf. Elev 543.2 ft	Hammer Drop 30 in	Rock Core Dia. N/A	Inspector K. Bullock
Date Started5/16/11	Spoon Size2 in	Boring MethodHSA	Date Completed5/16/11

	ELEV/	SOIL DESCRIPTION	STRA	ا ا	王백		SAM	//PLE			DODING A CAMPLE]
	ELEV. (ft)	Color, Moisture, Density, Plasticity, Size Proportions	DEPTH (ft)	SOIL	DEPTH SCALE	Cond	Blows/6"	No.	Туре	Rec (in)	BORING & SAMPLE NOTES	
	542.7	6" of TOPSOIL	0.5	7/1/2 7/1							No water encountered.	
_		Tan, moist, medium stiff, lean CLAY , trace sand			-	ı	2-2-4-5	1	DS	20	Grab bag of topsoil taken.	
_	<u>541.2</u>	Gray with white, moist, stiff to very stiff, lean CLAY, with mica	2.0			I	3-5-7-7	2	DS	24	3. Boring drilled using a Simco 2800 All Terrain Vehicle (ATV) mounted drill rig with safety hammer.	
					<u>5</u>	I/D	4-6-8-8	3	DS	18	4. Boring Coordinates: N: 577793.5967 E: 1227517.3757	
_	-					I	3-5-7-8	4	DS	24		_
_	533.2		10.0		- 10	ı	6-9-16-16	5	DS	20		_
	333.2	Bottom of Boring at 10.0 ft	10.0	*/////								_
_	-				_							_
_	-				_							_
	-				15							
_					_							
					_							
					_							
<u></u>		AAMDI ED TYDE			20						DING METHOD	

SAMPLER TYPE

DS - DRIVEN SPLIT SPOON

PT - PRESSED SHELBY TUBE CA - CONTINUOUS FLIGHT AUGER RC - ROCK CORE

SAMPLE CONDITIONS

D - DISINTEGRATED I - INTACT

U - UNDISTURBED L - LOST

GROUNDWATER DEPTH

AT COMPLETION dry ft
AFTER HRS. ft
CAVED AT 7.0 ft

BORING METHOD

HSA - HOLLOW STEM AUGERS CFA - CONTINUOUS FLIGHT AUGERS
DC - DRIVING CASING
MD - MUD DRILLING

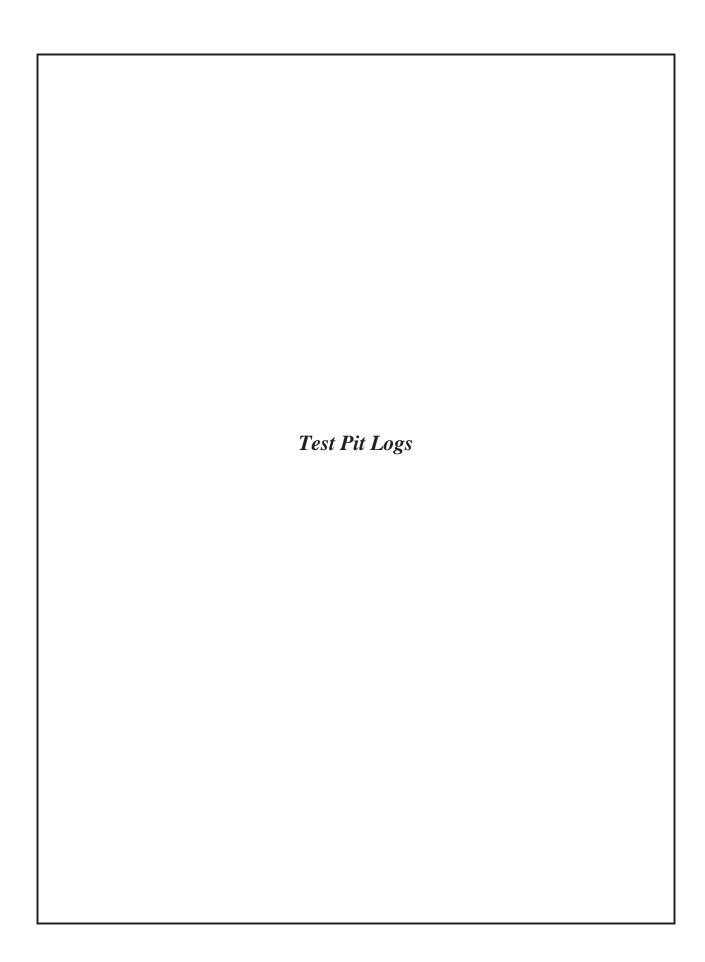
SUMMARY OF INFILTRATION TEST RESULTS

LITTLE BENNETT REGIONAL PARK CLARKSBURG, MARYLAND

JOB NO. 10-030-A

Boring Number	Casing Depth	Water Reading After 24 hours Presoak	After 1st Hour Infiltration oak Rate	2nd Hour Infiltration Rate	3rd Hour Infiltration Rate	4th Hour Infiltration Average Rate Infiltration R	Average Infiltration Rate
	(ft)	(ft)	(in./hr)	(in./hr)	(in./hr)	(in./hr)	(in./hr)
IT-1/B-01	4.0	Dry	20.52	33.12	30.24	20.16	26.01
IT-2	5.0	0.12	1.44	0.72	0.84	0.72	0.93
IT-4	0.9	99.0	<0.12	<0.12	<0.12	<0.12	<0.12
9-II	5.0	Dry	5.76	4.20	2.76	2.40	3.78
8-II	4.0	Dry	1.56	0.96	1.32	1.56	1.35
6-LI	4.0	Dry	3.96	4.20	3.60	2.28	3.51

T. L. B. ASSOCIATES, INC. Clarksburg, Maryland





Contracted With	A. Morton	Thomas & Associate	s, Inc.	Test Pit #	P-1A
Project Name	Little Benn	nett Regional Park		Job#	10-030-A
Location	Clarksburg	g, Maryland		,	
Date 6/29/2011 Inspector Khadija B		Groundwater	No water o	observed	-
Depth 1 2 3 4	Soil Descr 6 inches of	*			-
5			n test perfo. 5/30/11 after		ar surface
7	Percolatio	n Test Data			
8	Original W	ater Level (from grou	and surface	1.167	ft
9	Measured	Drop from Original W	Vater Level	(ft)	•
10	1st Readin	g 1.75			-
11	2nd Readin	ng 1.5			
12	3rd Readin	ng 1.5			
13	4th Readin	g 1.5			
14 —— 15 ——	5th Readin	g 1.375			
16	6th Readin	g 1.25			
17	7th Readin	g 1.25			
17	8th Readin	g 1.25			



Contracted With	A. Morton	Thomas & Associate	es, Inc.	Test Pit #	P-1B
Project Name	Little Ben	nett Regional Park		Job#	10-030-A
Location	Clarksburg	g, Maryland		•	
Date 6/29/2011 Inspector Khadija B		Groundwater	No water o	observed	-
Depth	Soil Descr	iption			
0 —	6 inches of	*			-
1 T		test hole			
2 3					
4		Test depth at 2.5 ft			
5		Test perfo	ormed at nea	ır surface g	grade
6		on 6/30/1	1 after preso	oak	
7					
8		on Test Data			-
9	_	Vater Level (from gro			<u>ft</u>
10		Drop from Original V		(ft)	_
11	1st Readin	•			
12	2nd Readi	•			
13	3rd Readin	•			
14 —	4th Readir	•			
15	5th Readir	•			
16	6th Readir	•			
17	7th Readir	•			
18	8th Readir	ng 1.375)		



Date Clarksburg, Maryland Clarksburg, Maryland Date G/29/2011 Groundwater No water observed Groundwater No water observed Groundwater No water observed Groundwater No water observed Clarksburg, Maryland Clarksburg, Marylan	Contracted With	A. Morton Thomas & Associates, I	nc. Test Pit #	P-1C
Date Inspector Soil Description Toronto	Project Name	Little Bennett Regional Park	Job #	10-030-A
Depth Soil Description 7 inches of TOPSOIL Root mat observed to extend to 17 inches / 1.4 ft Grayish-brown, Silty SAND with gravel Reddish Brown, Silty SAND trace clay and rock fragments Test depth at 5.65 ft Test performed on 6/30/11 after presoak Percolation Test Data Original Water Level (from ground surface 4.4 ft Measured Drop from Original Water Level (ft) Ist Reading 5.25 2nd Reading 4.875 15 4th Reading 4.875 16 5th Reading 4.875 17 6th Reading 4.875	Location	Clarksburg, Maryland		
7 inches of TOPSOIL Root mat observed to extend to 17 inches / 1.4 ft Grayish-brown, Silty SAND with gravel Reddish Brown, Silty SAND trace clay and rock fragments Test depth at 5.65 ft Test performed on 6/30/11 after presoak Percolation Test Data Original Water Level (from ground surface) Addition Reading Section 1.4 ft Measured Drop from Original Water Level (ft) Ist Reading Section 1.5 ft Test depth at 5.65 ft Test performed on 6/30/11 after presoak Percolation Test Data Original Water Level (from ground surface) Section 1.4 ft Measured Drop from Original Water Level (ft) Section 1.5 ft Ath Reading Se			o water observed	
7th Reading 4.875 8th Reading 4.875	1 —— 2 —— 3 —— 4 —— 5 —— 6 —— 7 —— 8 —— 9 —— 10 —— 11 —— 12 —— 13 —— 14 —— 15 —— 16 ——	7 inches of TOPSOIL Root mat observed to extend to 17 inches of TOPSOIL Root mat observed to extend to 17 inches of ToPSOIL Reddish Brown, Silty SAND with good test hole Test depth at 5.65 ft Test performed Percolation Test Data Original Water Level (from ground Measured Drop from Original Water 1st Reading 5.25 2nd Reading 4.875 3rd Reading 4.875 4th Reading 4.875 5th Reading 4.875 6th Reading 4.875 7th Reading 4.875	clay and rock ottom of pit at 3.15 ed on 6/30/11 after surface 4.4	3.15 ft ft presoak



RECORD OF TEST PIT EXPLORATION

Contracted With A. Morton Thomas & Associates, Inc. Test Pit # P-1D Project Name Little Bennett Regional Park Job# 10-030-A Location Clarksburg, Maryland Date 6/29/2011 Groundwater No water observed Inspector Khadija Bullock Depth Soil Description 6.5 inches of TOPSOIL 1 Gray-brown to reddish-brown Silty SAND with gravel 2 3 3 ft 4 Reddish-brown, Sandy CLAY with silt and shale 5 fragments, trace quartz and flint 6 (SAPROLITE) 7 8 9 9 ft Brown, Silty SAND with quartz and rock fragments 10 some mica 11 (SAPROLITE) 12 large rock fragments at 12 ft 13 13.8 ft Bottom of Pit at 13.8 ft test hole 15 16 test depth at 16 ft test performed on 6/30/11 after presoak 18 **Percolation Test Data** Original Water Level (from ground surface) 15.05 ft Measured Drop from Original Water Level (ft) 1st Reading 15.75 2nd Reading 15.75 3rd Reading 15.75 4th Reading 15.75

15.75

15.75

15.75

15.75

5th Reading

6th Reading

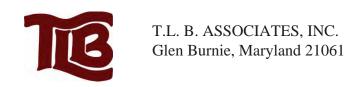
7th Reading

8th Reading



RECORD OF TEST PIT EXPLORATION

Contracted With Project Name Location	A. Morton Thomas Little Bennett Regi Clarksburg, Maryla	onal Park	Test Pit # Job #	P-1E 10-030-A
Date 6/29/2011 Inspector Khadija I		vater No water	er observed	-
Depth 0 1 2 3 4 Bottom o		OIL, Gray-brown silty O with gravel and quantest hole		- gravel - 1.4 ft - 3.65 ft
6 — 7 — 8 — 9 — 10 —	Percolation Test D			<u>-</u>
11 ———————————————————————————————————	_	rel (from ground surfa m Original Water Lev 5.5 5.3 5.35 5.125 5.15 5.125 5.125 5.125		n -



Contracted With	A. Morton Thomas & Associates	s, Inc.	Test Pit #	
Project Name	Little Bennett Regional Park		Job#	10-030-A
Location	Clarksburg, Maryland		_	
Date 6/29/2011 Inspector Khadija B		No water during exc 24-hours a	avation or	
Depth	Soil Description			
1 =	6 inches of TOPSOIL, Root mat	extends to	1ft	-
2 — 3 — 4 —	Brown to tan with orange, silty S and quartzites	SAND with	gravel	
5 —— 6 —— 7 ——	trace rock fragments at 7 ft			
8 9 10	Brown, sandy CLAY with rock f	ragments		8.5 ft
11 —	(SAPROLITE)			
12 —	excavating became di	fficult at 12	2.5 feet	
14 —	(bucket scraping on la	arge rock fi	ragments)	
15	Bottom of Pit at 15 ft.			-
17				
18				

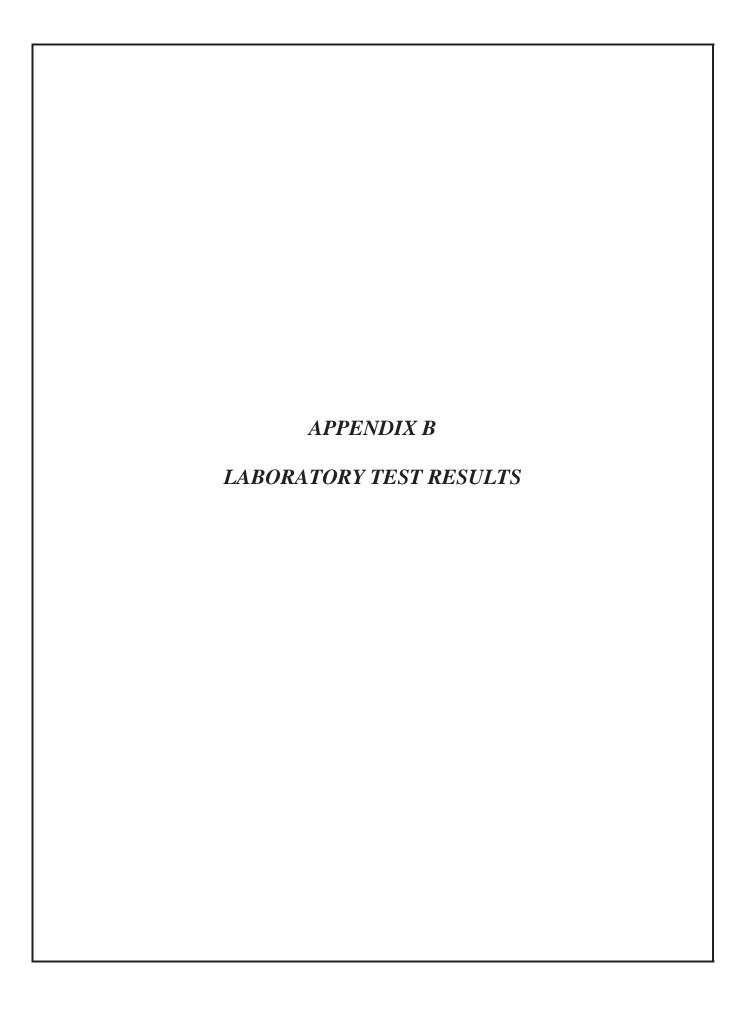
Little Bennet Regional Park Summary of Informal Percolation Test Data

Toef	Test Depth	Original Water	1st	2nd	3rd	444	544	4+9	4+/	844
Location	from Surface (ft)	Level (from surface in ft)	Reading	Reading	Reading	Reading	Reading Reading Reading Reading Reading Reading Reading	Reading	Reading	Reading
P-1A	2.250	1.167	1.750	1.500	1.500	1.500	1.750 1.500 1.500 1.500 1.375 1.375 1.250 1.250	1.375	1.250	1.250
P-1B	2.500	1.250	1.500	1.625	1.625	1.750	1.500 1.625 1.625 1.750 1.500 1.500	1.500	1.375	1.375
P-1C	5.650	4.400	5.250	4.875	5.375	4.875	5.250 4.875 5.375 4.875 4.750	4.875	4.875	4.875
P-1D	16.000	15.050	15.750	15.750	15.750	15.750	15.750 15.750 15.750 15.750 15.750 15.750 15.750 15.750 15.750	15.750	15.750	15.750
P-1E	6.150	4.725	5.500	5.300	5.350	5.125	5.500 5.300 5.350 5.125 5.150 5.125 5.125	5.125	5.125	5.125

Note: Caving conditions observed in Test Holes P-1C and P-1E at 3rd Reading

Summary of Informal Percolation Test Results

				Measured	Measured Drop in 30-Minute Intervals (in)	-Minute In	tervals (in			
Test Location	Test Depth from	1st	2nd	3rd	4th	5th	6th	7th	8th	Percolation Rate
	Surface (IL)	Reading	Reading	Reading	Reading Reading Reading Reading Reading Reading Reading	Reading	Reading	Reading	Reading	(Min/Inch)
P-1A	2.25	7.0	4.0	4.0	4.0	2.5	2.5	1.0	1.0	30.00
P-1B	2.50	3.0	4.5	4.5	6.0	3.0	3.0	1.5	1.5	20.00
P-1C	5.65	10.2	5.7	11.7	5.7	4.2	5.7	5.7	5.7	5.26
P-1D	16.00	8.4	8.4	8.4	8.4	8.4	8.4	8.4	8.4	3.57
P-1E	6.15	9.3	6.9	7.5	4.8	5.1	4.8	4.8	4.8	6.25



											Sheet 1 of 1
Depth (ft)	Liquid I Limit (%)	Plastic Limit (%)	Plastic Plasticity Limit Index (%) (%)	%<#4 Sieve	% < #200 Sieve	Classification	Water Content (%)	Dry Density (pcf)	Optimum Water Content (%)	Soaked CBR @ 95% Density @ 0.1 in (%)	Swelling (%)
4.0-6.0	40	34	9	69	22	SM	10.3				
1.5-5.0	34	26	80	97	72	ML	7.9	118.2	13.5	0.8	11.1
5.0-6.5				61	21		10.8				
7.5-9.0				23	8		10.3				
1.0-5.0	35	23	12	96	52	CL	4.4	129.2	11.0	6.0	16.0
4.0-6.0	34	30	4	58	22	В	7.8				
6.0-8.0	36	29	7	73	36	SM	13.8				
4.0-6.0	34	28	9	83	34	SM	17.0				
4.0-6.0	42	34	8	9/	20	ML	24.4				
2.0-4.0	31	25	9	100	99	ML	33.9				

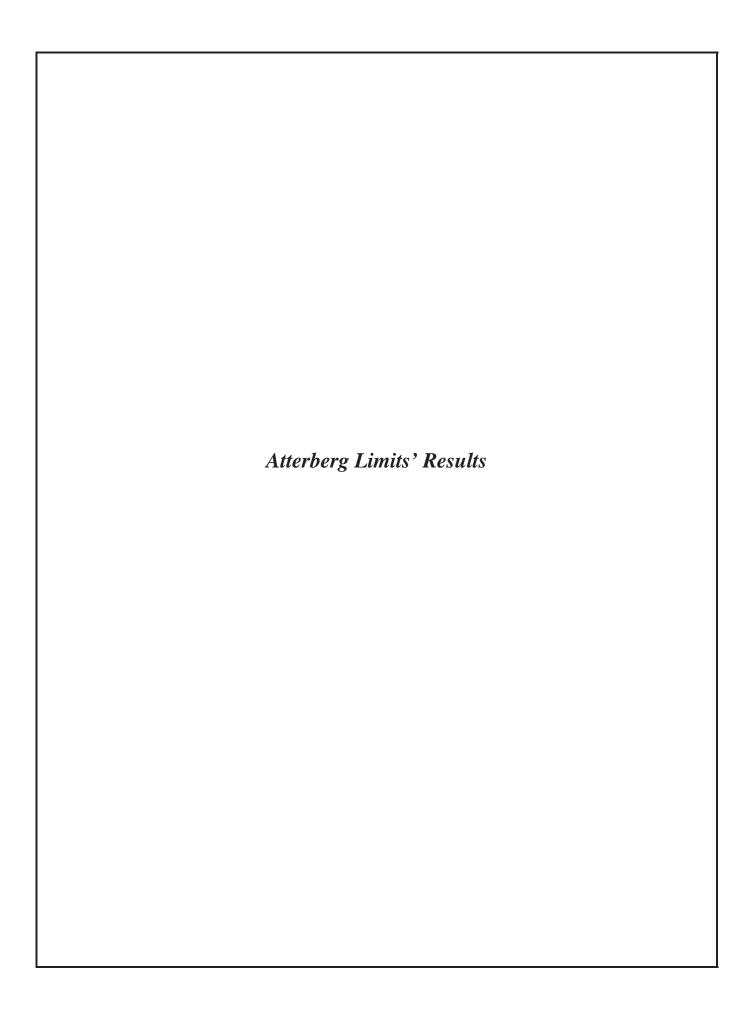
Clarksburg, MD

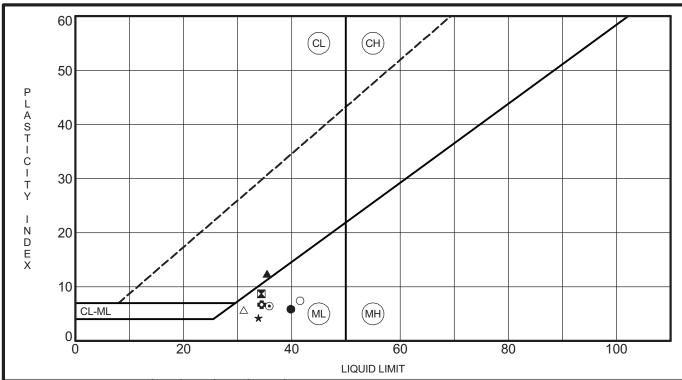
Summary of Laboratory Results

Little Bennett Regional Park

Project Number: 10-030-A

T.L.B. ASSOCIATES, INC. Glen Burnie, Maryland 21061





	Boring	Depth	LL	PL	PI	Fines	Classification
•	B-1	4.0-6.0	40	34	6	22	Tan, silty SAND with gravel (rock fragments) [USCS: SM; USDA: Sandy Loam]
M	B-17	1.5-5.0	34	26	8	72	Tan, SILT with sand [USCS: ML; USDA: Silt Loam]
A	B-2	1.0-5.0	35	23	12	52	Gray, sandy lean Clay [USDA: CL; USCS: Loam]
*	IT-2	4.0-6.0	34	30	4	22	Reddish brown, silty GRAVEL(rock fragments) with sand [USCS: GM; USDA: Sandy Loan
•	IT-4	6.0-8.0	36	29	7	36	Tan, silty SAND with gravel (rock fragments) [USCS: SM; USDA: Loam]
0	IT-6	4.0-6.0	34	28	6	34	Reddish brown, silty SAND with gravel (rock fragments) [USCS: SM; USDA: Sandy Loam
0	IT-8	4.0-6.0	42	34	8	50	Reddish brown, sandy SILT with gravel [USCS: ML; USDA: Loam]
Δ	IT-9	2.0-4.0	31	25	6	65	Dark brown, sandy SILT [USCS: ML; USDA: Silty Loam]

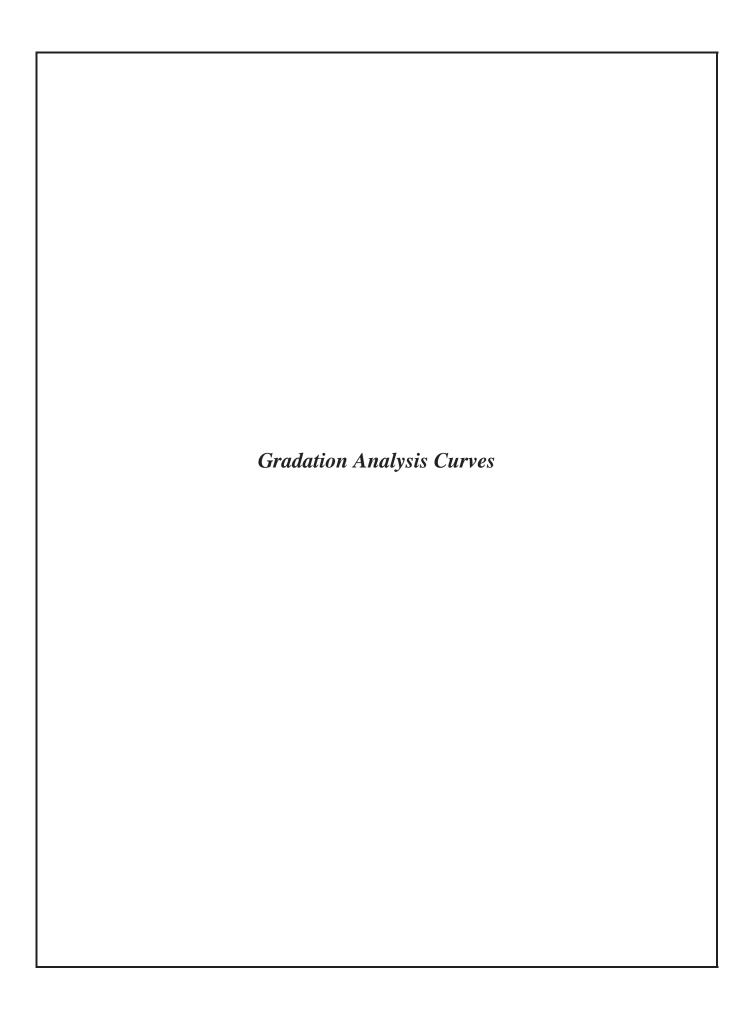
 Test Method: ASTM D4318
 Tested By: ____MG
 Date: ___6/7/2011

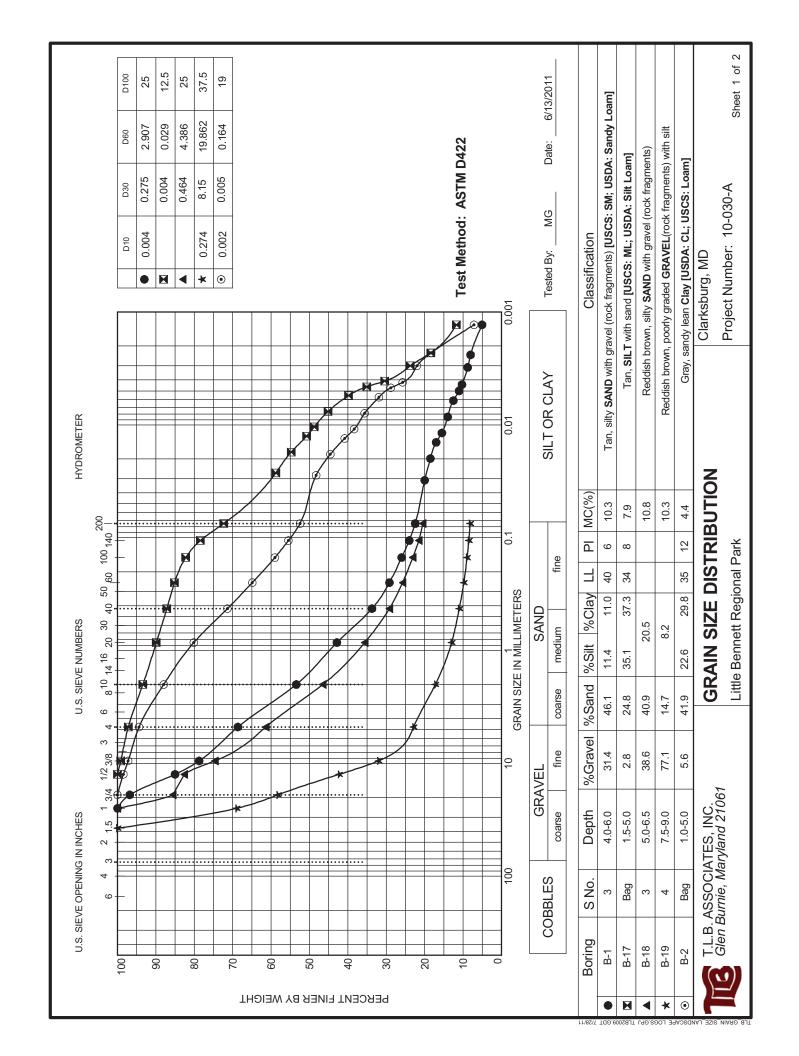


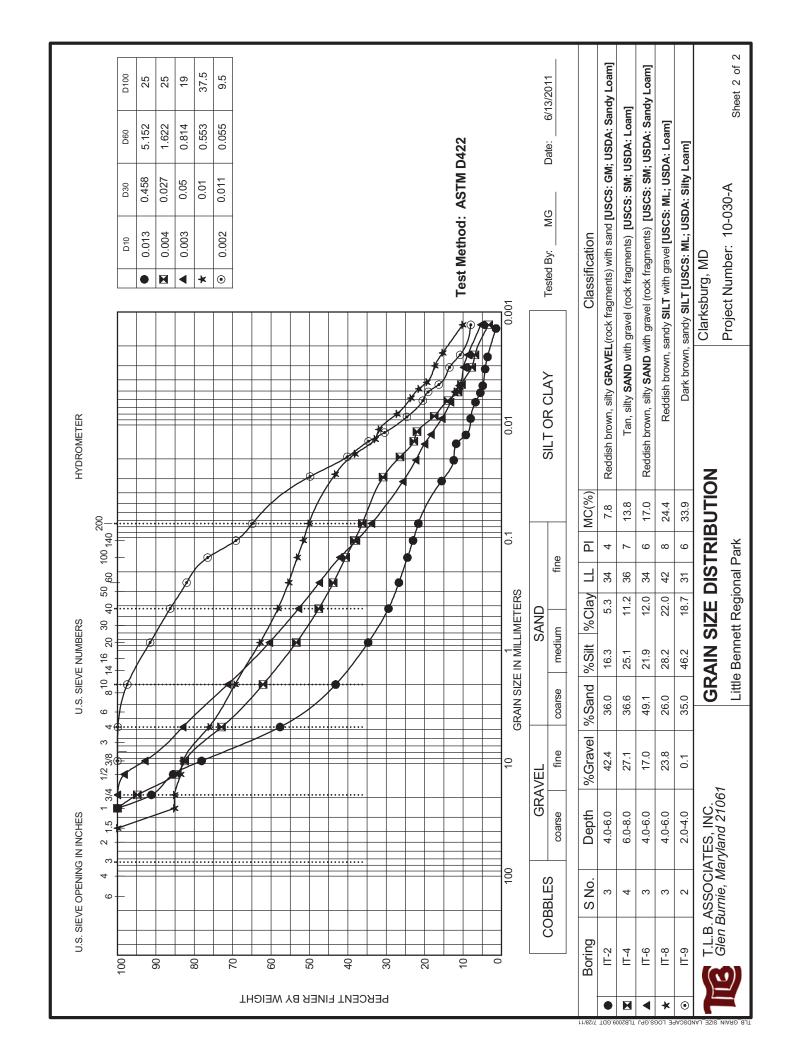
T.L.B. ASSOCIATES, INC. Glen Burnie, Maryland 21061

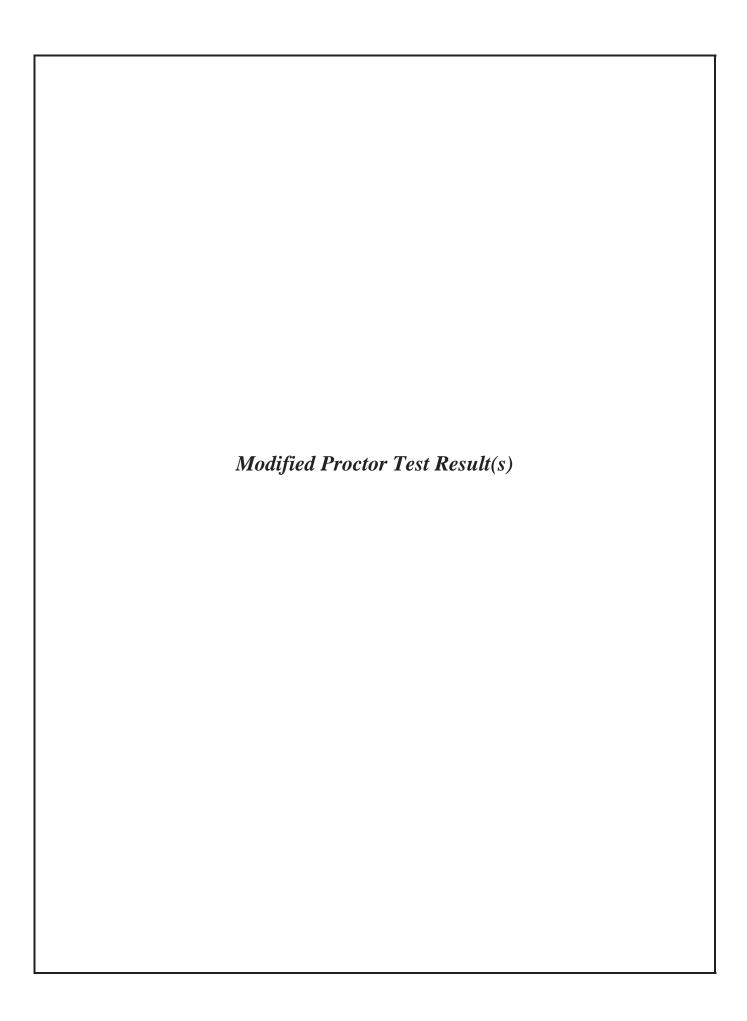
ATTERBERG LIMITS' RESULTS

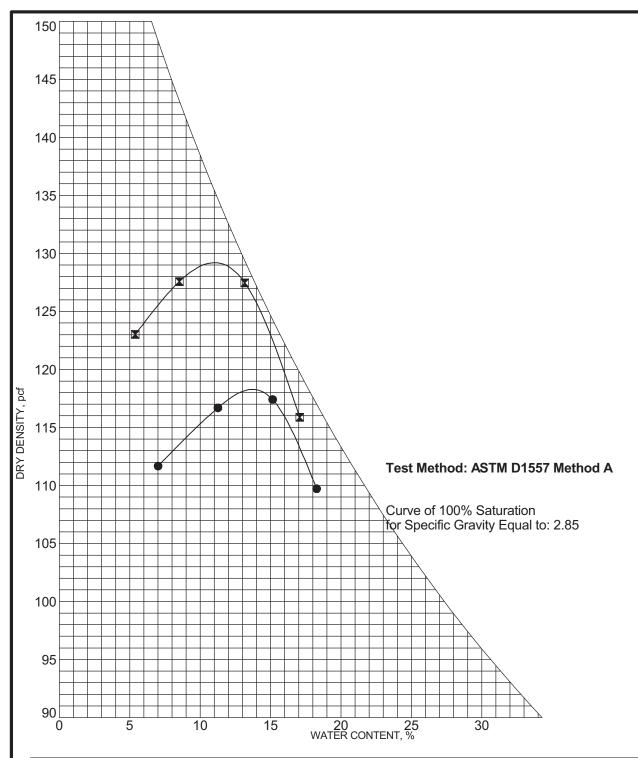
Project: Little Bennett Regional Park











	Boring	Sample No.	Depth, Ft	Classification	Max. Dry Density (pcf)	Opt. MC %	LL	PI
	B-17	Bag	1.5-5.0	Tan, SILT with sand [USCS: ML; USDA: Silt Loam]	118.2	13.5	34	8
X	B-2	Bag	1.0-5.0	Gray, sandy lean Clay [USDA: CL; USCS: Loam]	129.2	11.0	35	12

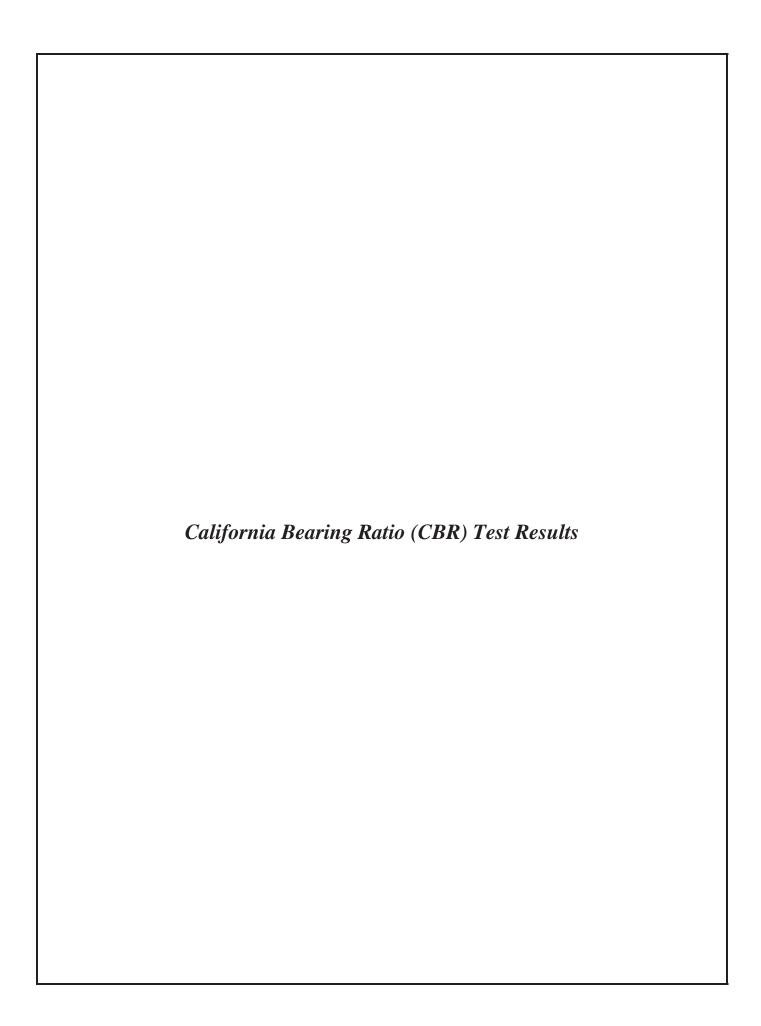
Tested By: MG Date: 6/9/2011

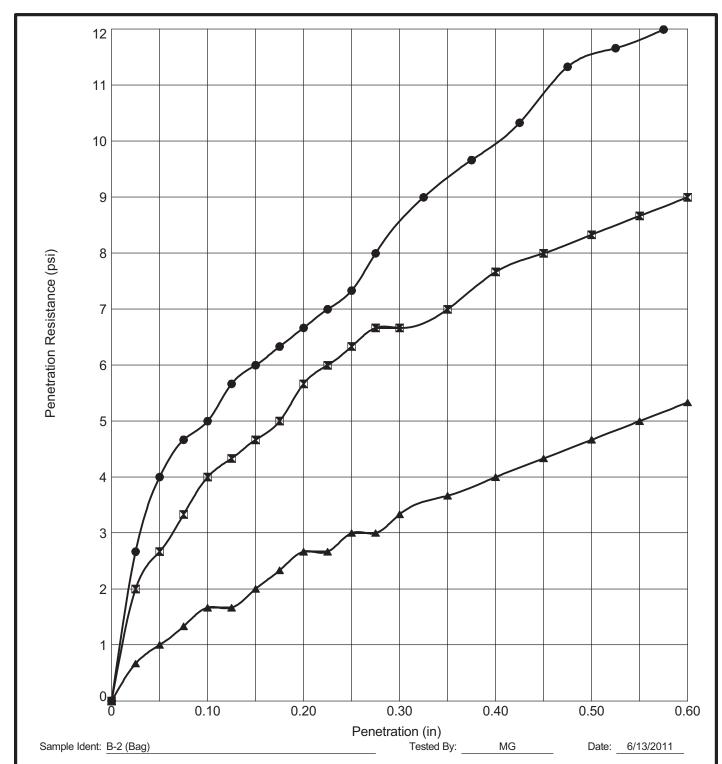


T.L.B. ASSOCIATES, INC. Glen Burnie, Maryland 21061

MOISTURE-DENSITY RELATIONSHIP

Project: Little Bennett Regional Park





Material Description: Gray, sandy lean Clay [USDA: CL; USCS: Loam]

	Density (pcf)	Molded % Max Density	% Moisture	Density (pcf)	Soaked % Max Density	% Moisture	0.1 in	BR 0.2 in	Pen. Surcharge (lbs)	% Swell
•	127.86	98.99	9.5	124.75	96.55	22.2	0.5	0.4	10.0	16.0
	121.59	94.12	9.5	118.63	91.82	23.8	0.4	0.4	10.0	15.0
	119.85	92.72	9.5	117.18	90.70	24.1	0.2	0.2	10.0	13.3

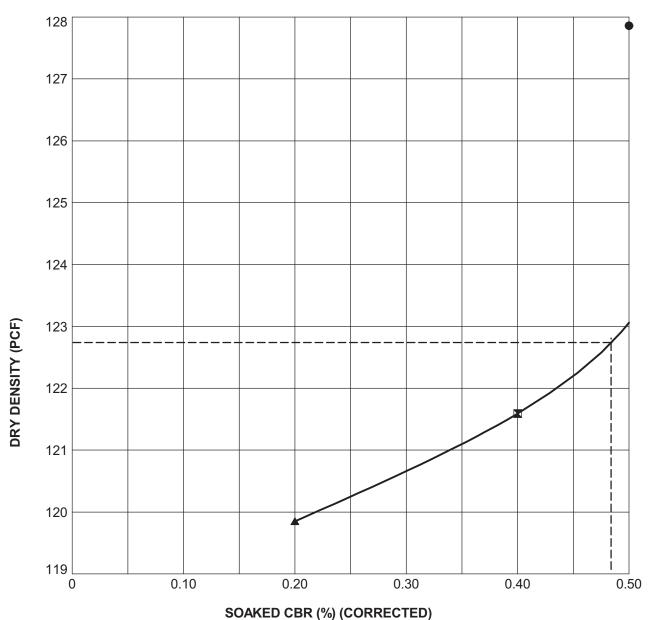
TEST METHOD: ASTM D1883



T.L.B. ASSOCIATES, INC. Glen Burnie, Maryland 21061

PENETRATION VS STRESS

Project: Little Bennett Regional Park



Remark: Soaked CBR values based on % of Modified Proctor (ASTM D1557 Method A)

Sample Ident: B-2 (Bag) Tested By: MG Date: 6/13/2011 Material Description: Gray, sandy lean Clay [USDA: CL; USCS: Loam]

Dry Density @ 95% ___122.7 _ pcf __CBR @ 95% Density ____0.5 ___ LL ____35____ PL ____23____

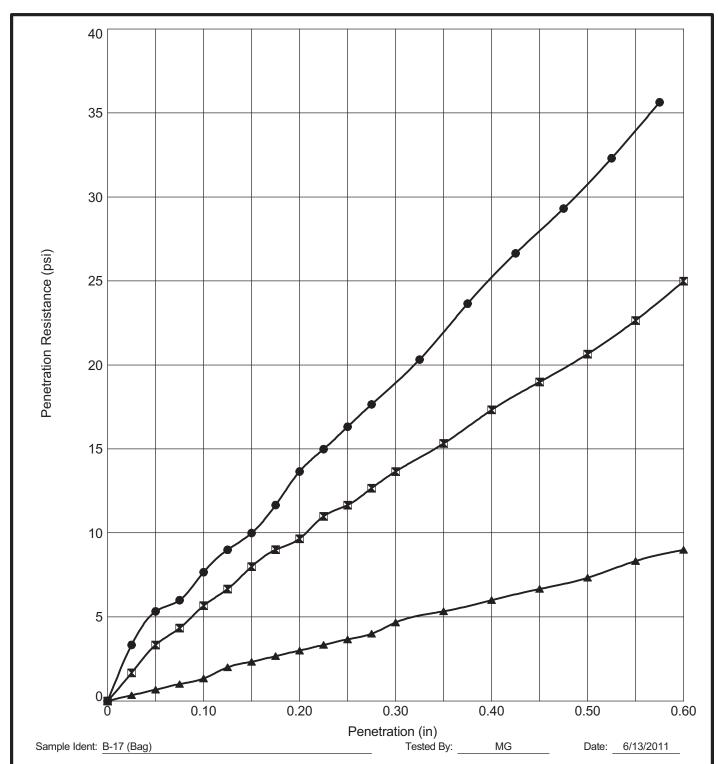
	% Compaction	Dry Density (pcf)	0.1 in	3R 0.2 in	% Swell
•	99.0	127.9	0.5	0.4	16.0
×	94.1	121.6	0.4	0.4	15.0
	92.7	119.8	0.2	0.2	13.3

TEST METHOD: ASTM D1883



DRY DENSITY VS SOAKED CBR

Project: Little Bennett Regional Park



Material Description: Tan, SILT with sand [USCS: ML; USDA: Silt Loam]

	Molded Density (pcf) % Max Density % Moisture			Soaked Density (pcf) % Max Density % Moisture				BR 0.2 in	Pen. Surcharge (lbs)	% Swell
•	113.86	96.36	13.3	111.00	93.91	25	0.8	0.9	10.0	11.1
	105.98	89.68	13.3	104.79	88.65	26.3	0.6	0.6	10.0	7.9
	93.89	79.44	13.3	94.87	80.26	28.3	0.1	0.2	10.0	4.6

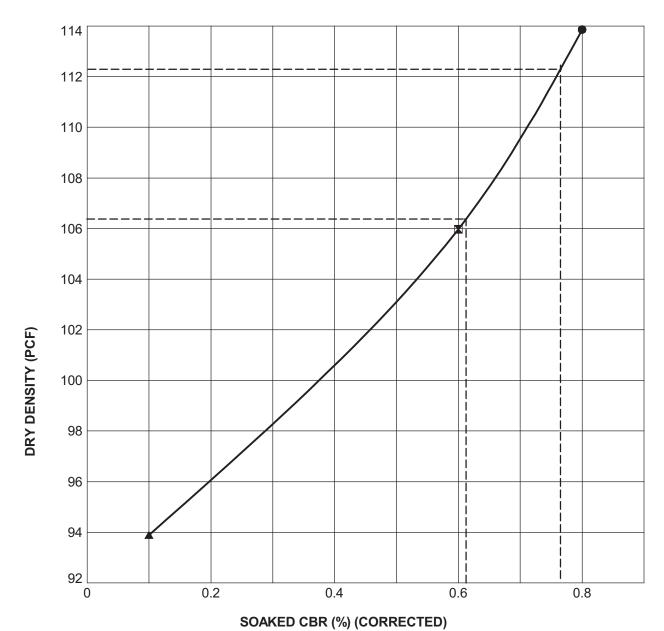
TEST METHOD: ASTM D1883



T.L.B. ASSOCIATES, INC. Glen Burnie, Maryland 21061

PENETRATION VS STRESS

Project: Little Bennett Regional Park



Remark: Soaked CBR values based on % of Modified Proctor (ASTM D1557 Method A)

Sample Ident: B-17 (Bag) Tested By: MG Date: 6/13/2011

Material Description: Tan, SILT with sand [USCS: ML; USDA: Silt Loam]

Dry Density @ 95% ___112.3 _pcf CBR @ 95% Density ___0.8 ___ LL ___34 ___ PL __26 ___ PI ___8

	% Compaction	Dry Density (pcf)	0.1 in	3R 0.2 in	% Swell
•	96.4	113.9	0.8	0.9	11.1
×	89.7	106.0	0.6	0.6	7.9
	79.4	93.9	0.1	0.2	4.6

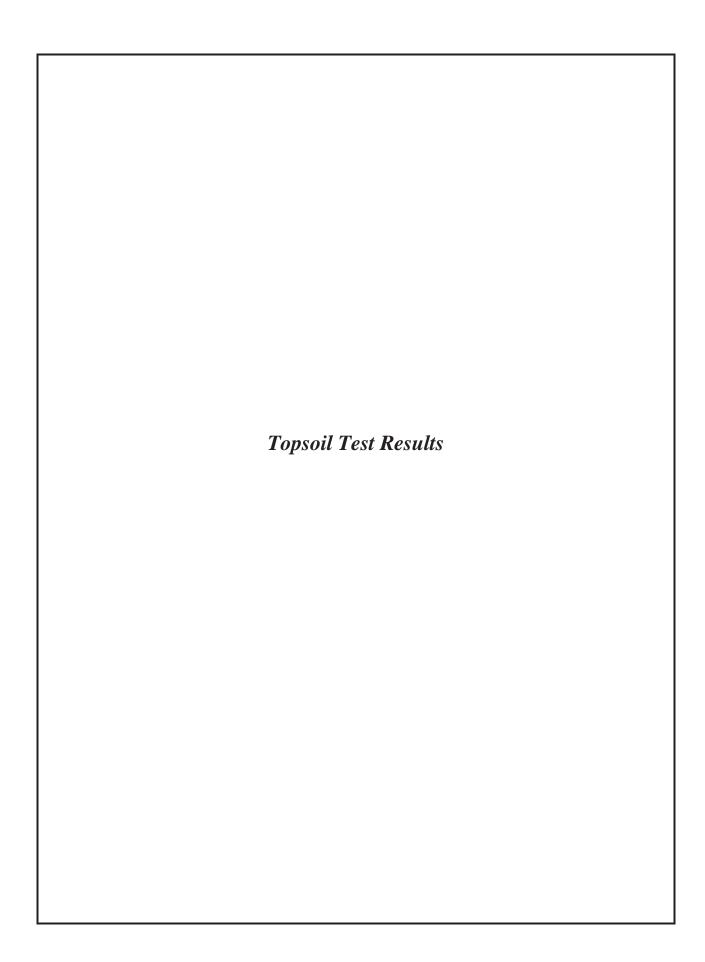
TEST METHOD: ASTM D1883



Glen Burnie, Maryland 21061

DRY DENSITY VS SOAKED CBR

Project: Little Bennett Regional Park





7621 Whitepine Road Richmond, Virginia 23237 (804) 743-9401 Fax (804) 271-6446

TEXTURE ANALYSIS

Client:

T.L.B. ASSOCIATES INC 7280 BALTIMORE ANNAPOLIS BLVD

GLEN BURNIE, MD 21061

Grower:

T.L.B. ASSOCIATES INC

Report No: 11-159-0530 Cust No : 02323

Date Printed : 06/10/2011

1 of 1 Page: K BULLOCK Submitted By:

		Farr	m:			Received : 06/08/2011
<u>Lab</u> <u>No</u>	Field ID	Sample Identification	Percent Sand	Percent Silt	Percent Clay	<u>Textural</u> Classification
19368		B-1	36.0	42.8	21.2	Loam
19369		B-3	44.0	38.8	17.2	Loam
19370		B-4	38.0	38.8	23.2	Loam
19371		B-5	42.0	40.8	17.2	Loam
19372		B-6	44.6	39.1	16.1	Loam
19373		B-7	40.0	42.8	17.2	Loam
19374		B-9	34.0	48.8	17.2	Loam
19375		B-11	32.0	46.8	21.2	Loam
19377		B-14	30.0	48.8	21.2	Loam
19378		B-15	26.0	50.8	23.2	Silt Loam
19379		B-16	18.0	58.8	23.2	Silt Loam
19380		B-17	26.0	48.8	25.2	Loam
19381		B-18	38.0	44.8	17.2	Loam
19382		B-19	40.3	43.5	16.2	Loam
19383		B-20	44.0	44.8	11.2	Loam
19384		IT-2	44.0	42.8	13.2	Loam
19385		IT-3	42.0	42.8	15.2	Loam
19386		IT-5	48.0	36.8	15.2	Loam
19387		IT-6	34.0	44.8	21.2	Loam
19388		IT-10	32.0	16.8	51.2	Clay



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Submitted By: K BULLOCK

SOIL ANALYSIS

11-159-0530 Client: Report No: T.L.B. ASSOCIATES INC Cust No: 02323 T.L.B. ASSOCIATES INC Date Printed: 06/10/2011 THOMAS BROWN 7280 BALTIMORE ANNAPOLIS BLVD Date Received: 06/08/2011 GLEN BURNIE MD 21061 PO: Page: 1 of 20

Lab Number: 19368 Field Id: Sample Id: B-1

				SOI	L TEST RATIN	NGS		Calculated Cation	
Test	Method	Results	Very Low	Low	Medium	Optimum	Very High	Exchange	e Capacity
Soil pH	1:1	5.1						7	.7
Buffer pH		6.64							/100g
Phosphorus (P)	M3	140 ppm		•	<u> </u>	•			ed Cation
Potassium (K)	M3	167 ppm		•				Satu	ration
Calcium (Ca)	МЗ	717 ppm					Γ	%K	5.6
Magnesium (Mg)	МЗ	97 ppm						%Ca	46.6
Sulfur (S)					Γ			%Mg	10.5
Boron (B)	M3	0.1 ppm						%H	37.7
Copper (Cu)								Hmeq	2.9
Iron (Fe)								-	
Manganese (Mn)									
Zinc (Zn)								K · Mc	Ratio
Sodium (Na)								0.5	
Soluble Salts		0.19 mmhos/cm						0.5	, , , , , , , , , , , , , , , , , , ,
Organic Matter	WB	2.0 % ENR 79							
Nitrate Nitrogen									
Soil Classification		0							

^{*} Additional results to follow

SOIL FERTILITY GUIDELINES

Crop: Bedding Plants Rec Units: LB/1000 SF

(lbs)	LIME (tons)	N	P ₂ O ₅	K ₂O	Mg	S	В	Cu	Mn	Zn	Fe
80		2.5	0	2.0	0		0.50				
Crop:								Rec U	nits:		

Comments:

CUSTOMERS NOT LISTED ON SUBMITTAL FORM Bedding Plants

- · Apply dolomitic lime to raise pH and improve the magnesium level.
- · Broadcast boron using Borax and mix into the soil to raise boron level. Note boron should not be applied in the band near the plant.
- All recommended fertilizers are on actual elemental basis. To convert to product basis, divide the recommended quantity in the first page by the percentage of the active ingredient then multiply by 100.
- For best result, if there are no existing plants, broadcast all lime then till and mix 6 inches into the soil. Limit the lime application to 50 pounds per 1000 sq. ft. for existing plants, apply every 4-6 months until the recommended amount is fulfilled.
- · Apply the amount of lime recommended in first page to raise pH
- · For more in depth explanation, go to our website www.al-labs-eastern.com and select the "Lawn and Garden" tab at the top of home page. At the bottom of the "Lawn and Garden" page, you find information explaining a soil test report and fertilizer recommendations. http://al-labs-eastern.com/forms/LawnGardenSoilTestExplained.pdf



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Submitted By: K BULLOCK

SOIL ANALYSIS

11-159-0530 Client : Report No: T.L.B. ASSOCIATES INC Cust No: T.L.B. ASSOCIATES INC 02323 Date Printed: THOMAS BROWN 06/10/2011 7280 BALTIMORE ANNAPOLIS BLVD Date Received: 06/08/2011 GLEN BURNIE MD 21061 PO: Page: 2 of 20

Lab Number: 19369 Field Id: Sample Id: B-3

				SOI	L TEST RATII	NGS		Calculated Cation	
Test	Method	Results	Very Low	Low	Medium	Optimum	Very High	Exchange	e Capacity
Soil pH	1:1	5.5						5	.9
Buffer pH		6.78							/100g
Phosphorus (P)	M3	40 ppm							ed Cation
Potassium (K)	M3	43 ppm						Satu	ration
Calcium (Ca)	M3	712 ppm						%K	1.9
Magnesium (Mg)	M3	81 ppm						%Ca	60.3
Sulfur (S)								%Mg	11.4
Boron (B)	M3	0.1 ppm						%Н	25.6
Copper (Cu)								Hmeq	1.5
Iron (Fe)								·	
Manganese (Mn)									
Zinc (Zn)								K · Ma	Ratio
Sodium (Na)								0.1	
Soluble Salts		0.08 mmhos/cm						0.1	10
Organic Matter	WB	2.3 % ENR 88							
Nitrate Nitrogen									
Soil Classification		0]						

^{*} Additional results to follow

SOIL FERTILITY GUIDELINES

Crop: Bedding Plants Rec Units: LB/1000 SF

(lbs)	LIME (tons)	N	P ₂ O ₅	K ₂O	Mg	S	В	Cu	Mn	Zn	Fe
41		2.5	1.0	6.0	0		0.50				
Crop:								Rec U	nits:		

Comments:

Bedding Plants

- · Apply dolomitic lime to raise pH and improve the magnesium level.
- Broadcast boron using Borax and mix into the soil to raise boron level. Note boron should not be applied in the band near the plant.
- · All recommended fertilizers are on actual elemental basis. To convert to product basis, divide the recommended quantity in the first page by the percentage of the active ingredient then multiply by 100.
- For best result, if there are no existing plants, broadcast all lime then till and mix 6 inches into the soil. Limit the lime application to 50 pounds per 1000 sq. ft. for existing plants, apply every 4-6 months until the recommended amount is fulfilled.
- Phosphate is more efficient if applied near the plant, apply all phosphate beside the row. Broadcast N and/or K2O then mix into the soil. If there is no fertilizer meets the ratio, you can use single element fertilizer such as Urea, Triplesuper Phosphate and Muriate of Potash to achieve the requirements. Consult the enclosed instruction sheet on lime and fertilizer application.
- · Apply the amount of lime recommended in first page to raise pH
- For more in depth explanation, go to our website www.al-labs-eastern.com and select the "Lawn and Garden" tab at the top of home page. At the bottom of the "Lawn and Garden" page, you find information explaining a soil test report and fertilizer recommendations. http://al-labs-eastern.com/forms/LawnGardenSoilTestExplained.pdf



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Submitted By: K BULLOCK

SOIL ANALYSIS

11-159-0530 Client : Report No: T.L.B. ASSOCIATES INC Cust No: T.L.B. ASSOCIATES INC 02323 Date Printed: THOMAS BROWN 06/10/2011 7280 BALTIMORE ANNAPOLIS BLVD Date Received: 06/08/2011 GLEN BURNIE MD 21061 PO: Page: 3 of 20

Lab Number: 19370 Field Id: Sample Id: B-4

_				SOII	L TEST RATI	NGS		Calculated Cation	
Test	Method	Results	Very Low	Low	Medium	Optimum	Very High	Exchange	e Capacity
Soil pH	1:1	5.9						6	.3
Buffer pH		6.82							/100g
Phosphorus (P)	M3	40 ppm							ed Cation
Potassium (K)	M3	39 ppm						Satu	ration
Calcium (Ca)	M3	828 ppm						%K	1.6
Magnesium (Mg)	M3	113 ppm						%Ca	65.7
Sulfur (S)								%Mg	14.9
Boron (B)	M3	0.2 ppm						%Н	17.0
Copper (Cu)								Hmeq	1.1
Iron (Fe)									
Manganese (Mn)									
Zinc (Zn)								K · M	g Ratio
Sodium (Na)								0.4	
Soluble Salts		0.16 mmhos/cm						0.	
Organic Matter	WB	2.8 % ENR 97						1	
Nitrate Nitrogen									
Soil Classification		0							

^{*} Additional results to follow

SOIL FERTILITY GUIDELINES

Crop: Bedding Plants Rec Units: LB/1000 SF

(lbs)	LIME (tons)	N	P ₂ O ₅	K ₂O	Mg	S	В	Cu	Mn	Zn	Fe
25		2.5	1.0	6.0	0		0.50				
Crop:	Crop:										

Comments:

Bedding Plants

- · Apply dolomitic lime to raise pH and improve the magnesium level.
- Broadcast boron using Borax and mix into the soil to raise boron level. Note boron should not be applied in the band near the plant.
- · All recommended fertilizers are on actual elemental basis. To convert to product basis, divide the recommended quantity in the first page by the percentage of the active ingredient then multiply by 100.
- For best result, if there are no existing plants, broadcast all lime then till and mix 6 inches into the soil. Limit the lime application to 50 pounds per 1000 sq. ft. for existing plants, apply every 4-6 months until the recommended amount is fulfilled.
- Phosphate is more efficient if applied near the plant, apply all phosphate beside the row. Broadcast N and/or K2O then mix into the soil. If there is no fertilizer meets the ratio, you can use single element fertilizer such as Urea, Triplesuper Phosphate and Muriate of Potash to achieve the requirements. Consult the enclosed instruction sheet on lime and fertilizer application.
 - · Apply the amount of lime recommended in first page to raise pH
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Submitted By: K BULLOCK

SOIL ANALYSIS

11-159-0530 Client : Report No: T.L.B. ASSOCIATES INC Cust No: T.L.B. ASSOCIATES INC 02323 Date Printed: THOMAS BROWN 06/10/2011 7280 BALTIMORE ANNAPOLIS BLVD Date Received: 06/08/2011 GLEN BURNIE MD 21061 PO: Page: 4 of 20

Lab Number: 19371 Field Id: Sample Id: B-5

	l	- ··		SOII	L TEST RATIN	NGS		Calculate	ed Cation
Test	Method	Results	Very Low	Low	Medium	Optimum	Very High	Exchange	e Capacity
Soil pH	1:1	5.7						5	.2
Buffer pH		6.82							- <u>-</u> /100g
Phosphorus (P)	M3	29 ppm							ed Cation
Potassium (K)	M3	42 ppm						Satu	ration
Calcium (Ca)	M3	634 ppm						%K	2.1
Magnesium (Mg)	M3	98 ppm						%Ca	61.0
Sulfur (S)								%Mg	15.7
Boron (B)	M3	0.1 ppm						%H	21.0
Copper (Cu)								Hmeq	1.1
Iron (Fe)									
Manganese (Mn)									
Zinc (Zn)								K · Ma	Ratio
Sodium (Na)								0.1	
Soluble Salts		0.09 mmhos/cm						0.1	
Organic Matter	WB	2.0 % ENR 83							
Nitrate Nitrogen									
Soil Classification		0							

^{*} Additional results to follow

SOIL FERTILITY GUIDELINES

Crop: Bedding Plants Rec Units: LB/1000 SF

(lbs)	LIME (tons)	N	P ₂ O ₅	K ₂O	Mg	S	В	Cu	Mn	Zn	Fe
35		2.5	2.0	6.0	0		0.50				
Crop:								Rec U	nits:		

Comments:

Bedding Plants

- · Broadcast boron using Borax and mix into the soil to raise boron level. Note boron should not be applied in the band near the plant.
- All recommended fertilizers are on actual elemental basis. To convert to product basis, divide the recommended quantity in the first page by the percentage of the active ingredient then multiply by 100.
- For best result, if there are no existing plants, broadcast all lime then till and mix 6 inches into the soil. Limit the lime application to 50 pounds per 1000 sq. ft. for existing plants, apply every 4-6 months until the recommended amount is fulfilled.
- Phosphate is more efficient if applied near the plant, apply all phosphate beside the row. Broadcast N and/or K2O then mix into the soil. If there is no fertilizer meets the ratio, you can use single element fertilizer such as Urea, Triplesuper Phosphate and Muriate of Potash to achieve the requirements. Consult the enclosed instruction sheet on lime and fertilizer application.
- Apply the amount of lime recommended in first page to raise pH
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Submitted By: K BULLOCK

SOIL ANALYSIS

11-159-0530 Client: Report No: T.L.B. ASSOCIATES INC Cust No: 02323 T.L.B. ASSOCIATES INC Date Printed: 06/10/2011 THOMAS BROWN 7280 BALTIMORE ANNAPOLIS BLVD Date Received: 06/08/2011 GLEN BURNIE MD 21061 PO: Page: 5 of 20

Lab Number: 19372 Field Id: Sample Id: B-6

				SOII	L TEST RATIN	IGS		Calculate	ed Cation
Test	Method	Results	Very Low	Low	Medium	Optimum	Very High	Exchange	Capacity
Soil pH	1:1	6.2						5	.5
Buffer pH		6.86							.0 /100g
Phosphorus (P)	M3	17 ppm							ed Cation
Potassium (K)	M3	30 ppm						Satu	ration
Calcium (Ca)	M3	805 ppm						%K	1.4
Magnesium (Mg)	M3	86 ppm						%Ca	73.2
Sulfur (S)								%Mg	13.0
Boron (B)	M3	0.1 ppm						%H	12.1
Copper (Cu)								Hmeq	0.7
Iron (Fe)								-	
Manganese (Mn)									
Zinc (Zn)								K · Ma	Ratio
Sodium (Na)								0.1	
Soluble Salts		0.05 mmhos/cm						0.1	
Organic Matter	WB	1.9 % ENR 80							
Nitrate Nitrogen									
Soil Classification		0							

^{*} Additional results to follow

SOIL FERTILITY GUIDELINES

Crop: Bedding Plants Rec Units: LB/1000 SF

(lbs) L	IME (tons)	N	P ₂ O ₅	K ₂ O	Mg	S	В	Cu	Mn	Zn	Fe
0		2.5	3.5	6.0	0		0.50				
Crop:								Rec U	nits:		

Comments:

- · Broadcast boron using Borax and mix into the soil to raise boron level. Note boron should not be applied in the band near the plant.
- All recommended fertilizers are on actual elemental basis. To convert to product basis, divide the recommended quantity in the first page by the percentage of the active ingredient then multiply by 100.
- Phosphate is more efficient if applied near the plant, apply all phosphate beside the row. Broadcast N and/or K2O then mix into the soil. If there is no fertilizer meets the ratio, you can use single element fertilizer such as Urea, Triplesuper Phosphate and Muriate of Potash to achieve the requirements. Consult the enclosed instruction sheet on lime and fertilizer application.
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Submitted By: K BULLOCK

SOIL ANALYSIS

11-159-0530 Client : Report No: T.L.B. ASSOCIATES INC Cust No: 02323 T.L.B. ASSOCIATES INC Date Printed: THOMAS BROWN 06/10/2011 7280 BALTIMORE ANNAPOLIS BLVD Date Received: 06/08/2011 GLEN BURNIE MD 21061 PO: Page: 6 of 20

Lab Number: 19373 Field Id: Sample Id: B-7

				SOI	L TEST RATII	NGS		Calculate	ed Cation
Test	Method	Results	Very Low	Low	Medium	Optimum	Very High	Exchange	Capacity
Soil pH	1:1	7.6						19	.9
Buffer pH								meq/	
Phosphorus (P)	М3	16 ppm						Calculate	
Potassium (K)	МЗ	51 ppm						Satur	ation
Calcium (Ca)	М3	3751 ppm		1		<u> </u>		%K	0.7
Magnesium (Mg)	МЗ	118 ppm						%Ca	94.2
Sulfur (S)								%Mg	4.9
Boron (B)	МЗ	1.0 ppm						%Н	0.0
Copper (Cu)								Hmeq	0.0
Iron (Fe)									
Manganese (Mn)									
Zinc (Zn)								K : Mg	Ratio
Sodium (Na)								0.1	
Soluble Salts		0.28 mmhos/cm						0.1	s <u> </u>
Organic Matter	WB	2.3 % ENR 72							
Nitrate Nitrogen									
Soil Classification		0							

^{*} Additional results to follow

SOIL FERTILITY GUIDELINES

Crop: Bedding Plants Rec Units: LB/1000 SF

(lbs) L	IME (tons)	N	P ₂ O ₅	K ₂ O	Mg	S	В	Cu	Mn	Zn	Fe
0		2.5	3.5	5.5	0		0				
Crop:								Rec U	nits:		

Comments:

- · Cation Exchange Capacity may be over-estimated due to high pH and free lime in the soil.
- All recommended fertilizers are on actual elemental basis. To convert to product basis, divide the recommended quantity in the first page by the percentage of the active ingredient then multiply by 100.
- · Phosphate is more efficient if applied near the plant, apply all phosphate beside the row. Broadcast N and/or K2O then mix into the soil. If there is no fertilizer meets the ratio, you can use single element fertilizer such as Urea, Triplesuper Phosphate and Muriate of Potash to achieve the requirements. Consult the enclosed instruction sheet on lime and fertilizer application.
- Soil pH will come down naturally, it is not necessary to reduce pH if it is below 7.2. To reduce pH, apply 2.5 pounds of elemental sulfur per 1000 square feet for every 0.1 of pH unit above 7.2. For example, pH 7.4 requires 5 pounds. Sulfur should be thoroughly mix 6 inches into the soil
- · Use ammonium sulfate as all or portion of the N requirement to reduce pH.
- · For more in depth explanation, go to our website www.al-labs-eastern.com and select the "Lawn and Garden" tab at the top of home page. At the bottom of the "Lawn and Garden" page, you find information explaining a soil test report and fertilizer recommendations. http://al-labs-eastern.com/forms/LawnGardenSoilTestExplained.pdf



7621 Whitepine Road Richmond, Virginia 23237 (804) 743-9401 Fax (804) 271-6446

Submitted By: K BULLOCK

SOIL ANALYSIS

11-159-0530 Client: Report No: T.L.B. ASSOCIATES INC Cust No: 02323 T.L.B. ASSOCIATES INC Date Printed: 06/10/2011 THOMAS BROWN 7280 BALTIMORE ANNAPOLIS BLVD Date Received: 06/08/2011 GLEN BURNIE MD 21061 PO: Page: 7 of 20

Lab Number: 19374 Field Id: Sample Id: B-9

_				SOII	L TEST RATII	NGS		Calculat	ed Cat
Test	Method	Results	Very Low	Low	Medium	Optimum	Very High	Exchange	e Capa
Soil pH	1:1	6.2						1:	2.8
Buffer pH		6.77							/100g
Phosphorus (P)	M3	19 ppm	·					Calculat	
Potassium (K)	M3	43 ppm						Satu	ration
Calcium (Ca)	M3	2066 ppm			1	•		%K	0.9
Magnesium (Mg)	M3	97 ppm						%Ca	80.7
Sulfur (S)				_				%Mg	6.3
Boron (B)	M3	0.4 ppm						%Н	12.1
Copper (Cu)				_				Hmeq	1.6
Iron (Fe)								_	
Manganese (Mn)									
Zinc (Zn)								K : Mo	g Ratio
Sodium (Na)									14
Soluble Salts		0.36 mmhos/cm						0.	·
Organic Matter	WB	2.1 % ENR 74							
Nitrate Nitrogen									
Soil Classification		0							

^{*} Additional results to follow

SOIL FERTILITY GUIDELINES

Crop: Bedding Plants Rec Units: LB/1000 SF

(lbs) L	IME (tons)	N	P ₂ O ₅	K ₂ O	Mg	S	В	Cu	Mn	Zn	Fe
0		2.5	3.0	6.0	0		0.50				
Crop:								Rec U	nits:		

Comments:

- · Broadcast boron using Borax and mix into the soil to raise boron level. Note boron should not be applied in the band near the plant.
- · All recommended fertilizers are on actual elemental basis. To convert to product basis, divide the recommended quantity in the first page by the percentage of the active ingredient then multiply by 100.
- Phosphate is more efficient if applied near the plant, apply all phosphate beside the row. Broadcast N and/or K2O then mix into the soil. If there is no fertilizer meets the ratio, you can use single element fertilizer such as Urea, Triplesuper Phosphate and Muriate of Potash to achieve the requirements. Consult the enclosed instruction sheet on lime and fertilizer application.
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Lab Number: 19375 Field Id: Sample Id: B-11

				SOI	L TEST RATIN	NGS			ed Cation
Test	Method	Results	Very Low	Low	Medium	Optimum	Very High	Exchange	Capacity
Soil pH	1:1	5.9						9	.7
Buffer pH		6.76							/100g
Phosphorus (P)	M3	26 ppm							ed Cation
Potassium (K)	M3	65 ppm						Satur	ration
Calcium (Ca)	M3	1387 ppm						%K	1.7
Magnesium (Mg)	M3	110 ppm						%Ca	71.5
Sulfur (S)								%Mg	9.5
Boron (B)	M3	0.4 ppm						%Н	17.1
Copper (Cu)				Γ				Hmeq	1.7
Iron (Fe)									
Manganese (Mn)									
Zinc (Zn)								K · Ma	Ratio
Sodium (Na)								0.1	
Soluble Salts		0.2 mmhos/cm						0.1	0
Organic Matter	WB	2.4 % ENR 84							
Nitrate Nitrogen									
Soil Classification		0							

^{*} Additional results to follow

SOIL FERTILITY GUIDELINES

Crop: Bedding Plants Rec Units: LB/1000 SF

(lbs) [IME (tons)	N	P ₂ O ₅	K ₂ O	Mg	S	В	Cu	Mn	Zn	Fe
30		2.5	2.5	5.5	0		0.50				
Crop:								Rec U	nits:		

Comments:

Bedding Plants

- · Apply dolomitic lime to raise pH and improve the magnesium level.
- Broadcast boron using Borax and mix into the soil to raise boron level. Note boron should not be applied in the band near the plant.
- · All recommended fertilizers are on actual elemental basis. To convert to product basis, divide the recommended quantity in the first page by the percentage of the active ingredient then multiply by 100.
- For best result, if there are no existing plants, broadcast all lime then till and mix 6 inches into the soil. Limit the lime application to 50 pounds per 1000 sq. ft. for existing plants, apply every 4-6 months until the recommended amount is fulfilled.
- Phosphate is more efficient if applied near the plant, apply all phosphate beside the row. Broadcast N and/or K2O then mix into the soil. If there is no fertilizer meets the ratio, you can use single element fertilizer such as Urea, Triplesuper Phosphate and Muriate of Potash to achieve the requirements. Consult the enclosed instruction sheet on lime and fertilizer application.
 - · Apply the amount of lime recommended in first page to raise pH
- For more in depth explanation, go to our website www.al-labs-eastern.com and select the "Lawn and Garden" tab at the top of home page. At the bottom of the "Lawn and Garden" page, you find information explaining a soil test report and fertilizer recommendations. http://al-labs-eastern.com/forms/LawnGardenSoilTestExplained.pdf



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Submitted By: K BULLOCK

SOIL ANALYSIS

Client:
T.L.B. ASSOCIATES INC
THOMAS BROWN
7280 BALTIMORE ANNAPOLIS BLVD
GLEN BURNIE MD 21061

T.L.B. ASSOCIATES INC

Report No: 11Cust No:
Date Printed: 0

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Lab Number: 19377 Field Id: Sample Id: B-14

_				SOI	L TEST RATIN	IGS		Calculat	ed Cation
Test	Method	Results	Very Low	Low	Medium	Optimum	Very High	Exchange	e Capacity
Soil pH	1:1	6.3						Q	.2
Buffer pH		6.83							/100g
Phosphorus (P)	M3	27 ppm	,						ed Cation
Potassium (K)	M3	115 ppm	·					Satu	ration
Calcium (Ca)	M3	1335 ppm						%K	3.2
Magnesium (Mg)	M3	154 ppm						%Ca	72.6
Sulfur (S)								%Mg	13.9
Boron (B)	M3	0.5 ppm	'					%Н	10.6
Copper (Cu)								Hmeq	1.0
Iron (Fe)									
Manganese (Mn)									
Zinc (Zn)								K · M	g Ratio
Sodium (Na)								0.2	
Soluble Salts		0.18 mmhos/cm						0.2	23
Organic Matter	WB	2.6 % ENR 89						1	
Nitrate Nitrogen									
Soil Classification		0							

^{*} Additional results to follow

SOIL FERTILITY GUIDELINES

Crop: Bedding Plants Rec Units: LB/1000 SF

(lbs)	LIME (tons)	N	P ₂ O ₅	K ₂ O	Mg	S	В	Cu	Mn	Zn	Fe
0		2.5	2.0	4.0	0		0				
Crop:								Rec U	nits:		

Comments:

- · All recommended fertilizers are on actual elemental basis. To convert to product basis, divide the recommended quantity in the first page by the percentage of the active ingredient then multiply by 100.
- Phosphate is more efficient if applied near the plant, apply all phosphate beside the row. Broadcast N and/or K2O then mix into the soil. If there is no fertilizer meets the ratio, you can use single element fertilizer such as Urea, Triplesuper Phosphate and Muriate of Potash to achieve the requirements. Consult the enclosed instruction sheet on lime and fertilizer application.
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Submitted By: K BULLOCK

SOIL ANALYSIS

11-159-0530 Client : Report No: T.L.B. ASSOCIATES INC Cust No: T.L.B. ASSOCIATES INC 02323 Date Printed: THOMAS BROWN 06/10/2011 7280 BALTIMORE ANNAPOLIS BLVD Date Received: 06/08/2011 GLEN BURNIE MD 21061 PO: Page: 10 of 20

Lab Number: 19378 Field Id: Sample Id: B-15

Test	Method	Results	SOIL TEST RATINGS						Calculated Cation	
			Very Low	Low	Medium	Optimum	Very High	Exchange	e Capacity	
Soil pH	1:1	5.9						6.7		
Buffer pH		6.81							/100g	
Phosphorus (P)	М3	24 ppm						Calculated Cation		
Potassium (K)	М3	64 ppm						Satu	ration	
Calcium (Ca)	М3	889 ppm						%K	2.4	
Magnesium (Mg)	М3	115 ppm						%Ca	66.3	
Sulfur (S)								%Mg	14.3	
Boron (B)	М3	0.3 ppm						%Н	17.2	
Copper (Cu)								Hmeq	1.2	
Iron (Fe)										
Manganese (Mn)										
Zinc (Zn)								K · Ma	g Ratio	
Sodium (Na)								0.		
Soluble Salts		0.15 mmhos/cm						0.	.,	
Organic Matter	WB	2.3 % ENR 86								
Nitrate Nitrogen										
Soil Classification		0								

^{*} Additional results to follow

SOIL FERTILITY GUIDELINES

Crop: Bedding Plants Rec Units: LB/1000 SF

(lbs) L	IME (tons)	N	P ₂ O ₅	K ₂O	Mg	S	В	Cu	Mn	Zn	Fe
25		2.5	2.5	5.5	0		0.50				
Crop: Rec Units:											

Comments:

Bedding Plants

- · Apply dolomitic lime to raise pH and improve the magnesium level.
- Broadcast boron using Borax and mix into the soil to raise boron level. Note boron should not be applied in the band near the plant.
- · All recommended fertilizers are on actual elemental basis. To convert to product basis, divide the recommended quantity in the first page by the percentage of the active ingredient then multiply by 100.
- For best result, if there are no existing plants, broadcast all lime then till and mix 6 inches into the soil. Limit the lime application to 50 pounds per 1000 sq. ft. for existing plants, apply every 4-6 months until the recommended amount is fulfilled.
- Phosphate is more efficient if applied near the plant, apply all phosphate beside the row. Broadcast N and/or K2O then mix into the soil. If there is no fertilizer meets the ratio, you can use single element fertilizer such as Urea, Triplesuper Phosphate and Muriate of Potash to achieve the requirements. Consult the enclosed instruction sheet on lime and fertilizer application.
 - · Apply the amount of lime recommended in first page to raise pH
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SOIL ANALYSIS

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Lab Number: 19379 Field Id: Sample Id: B-16

				SOI	L TEST RATII	NGS		Calculated	d Cation
Test	Method	Results	Very Low	Low	Medium	Optimum	Very High	Exchange	Capacity
Soil pH	1:1	7.4						19.	.5
Buffer pH								meq/1	
Phosphorus (P)	М3	31 ppm						Calculated	
Potassium (K)	M3	74 ppm						Satura	ation
Calcium (Ca)	M3	3652 ppm				•		%K	1.0
Magnesium (Mg)	M3	132 ppm						%Ca	93.6
Sulfur (S)								%Mg	5.6
Boron (B)	M3	1.0 ppm						%H	0.0
Copper (Cu)								Hmeq	0.0
Iron (Fe)								-	
Manganese (Mn)									
Zinc (Zn)								K : Mg	Ratio
Sodium (Na)								0.17	
Soluble Salts		0.33 mmhos/cm						0.17	
Organic Matter	WB	2.9 % ENR 84							
Nitrate Nitrogen									
Soil Classification		0							

^{*} Additional results to follow

SOIL FERTILITY GUIDELINES

Crop : Bedding PlantsRec Units:LB/1000 SF

(lbs) L	ΜΕ (tons)	N	P ₂ O ₅	K ₂O	Mg	S	В	Cu	Mn	Zn	Fe
0		2.5	2.0	5.5	0		0				
Crop:								Rec U	nits:		

Comments:

Bedding Plants

- · Cation Exchange Capacity may be over-estimated due to high pH and free lime in the soil.
- \cdot All recommended fertilizers are on actual elemental basis. To convert to product basis, divide the recommended quantity in the first page by the percentage of the active ingredient then multiply by 100.
- · Phosphate is more efficient if applied near the plant, apply all phosphate beside the row. Broadcast N and/or K2O then mix into the soil. If there is no fertilizer meets the ratio, you can use single element fertilizer such as Urea, Triplesuper Phosphate and Muriate of Potash to achieve the requirements. Consult the enclosed instruction sheet on lime and fertilizer application.
- Soil pH will come down naturally, it is not necessary to reduce pH if it is below 7.2. To reduce pH, apply 2.5 pounds of elemental sulfur per 1000 square feet for every 0.1 of pH unit above 7.2. For example, pH 7.4 requires 5 pounds. Sulfur should be thoroughly mix 6 inches into the soil.
- · Use ammonium sulfate as all or portion of the N requirement to reduce pH.
- · For more in depth explanation, go to our website www.al-labs-eastern.com and select the "Lawn and Garden" tab at the top of home page. At the bottom of the "Lawn and Garden" page, you find information explaining a soil test report and fertilizer recommendations. http://al-labs-eastern.com/forms/LawnGardenSoilTestExplained.pdf



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SOIL ANALYSIS

11-159-0530 Client: Report No: T.L.B. ASSOCIATES INC Cust No: 02323 T.L.B. ASSOCIATES INC Date Printed: 06/10/2011 THOMAS BROWN 7280 BALTIMORE ANNAPOLIS BLVD Date Received: 06/08/2011 GLEN BURNIE MD 21061 PO: Page: 12 of 20

Lab Number: 19380 Field Id: Sample Id: B-17

				SOI	L TEST RATII	NGS		Calculate	ed Cation
Test	Method	Results	Very Low	Low	Medium	Optimum	Very High	Exchange	e Capacity
Soil pH	1:1	6.2						13	3.6
Buffer pH		6.77							/100g
Phosphorus (P)	M3	41 ppm							ed Cation
Potassium (K)	M3	68 ppm						Satu	ration
Calcium (Ca)	M3	2136 ppm						%K	1.3
Magnesium (Mg)	M3	131 ppm						%Ca	78.5
Sulfur (S)								%Mg	8.0
Boron (B)	M3	0.4 ppm						%Н	12.1
Copper (Cu)								Hmeq	1.6
Iron (Fe)									
Manganese (Mn)									
Zinc (Zn)								K · Ma	Ratio
Sodium (Na)								0.1	
Soluble Salts		0.29 mmhos/cm						0.	
Organic Matter	WB	2.9 % ENR 89							
Nitrate Nitrogen									
Soil Classification		0							

^{*} Additional results to follow

SOIL FERTILITY GUIDELINES

Crop: Bedding Plants Rec Units: LB/1000 SF

(lbs) Ll	ME (tons)	N	P ₂ O ₅	K ₂O	Mg	S	В	Cu	Mn	Zn	Fe
0		2.5	1.0	5.5	0		0.50				
Crop:								Rec U	nits:		

Comments:

Bedding Plants

- · Broadcast boron using Borax and mix into the soil to raise boron level. Note boron should not be applied in the band near the plant.
- All recommended fertilizers are on actual elemental basis. To convert to product basis, divide the recommended quantity in the first page by the percentage of the active ingredient then multiply by 100.
- Phosphate is more efficient if applied near the plant, apply all phosphate beside the row. Broadcast N and/or K2O then mix into the soil. If there is no fertilizer meets the ratio, you can use single element fertilizer such as Urea, Triplesuper Phosphate and Muriate of Potash to achieve the requirements. Consult the enclosed instruction sheet on lime and fertilizer application.
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Lab Number: 19381 Field Id: Sample Id: B-18

				SOII	L TEST RATI	NGS		Calculate	ed Cation
Test	Method	Results	Very Low	Low	Medium	Optimum	Very High	Exchange	e Capacity
Soil pH	1:1	5.6						6	.4
Buffer pH		6.78							/100g
Phosphorus (P)	М3	27 ppm							ed Cation
Potassium (K)	М3	40 ppm						Satu	ration
Calcium (Ca)	М3	785 ppm						%K	1.6
Magnesium (Mg)	М3	106 ppm						%Ca	61.3
Sulfur (S)								%Mg	13.8
Boron (B)	M3	0.2 ppm						%Н	23.3
Copper (Cu)								Hmeq	1.5
Iron (Fe)									
Manganese (Mn)									
Zinc (Zn)								K · Ma	g Ratio
Sodium (Na)								0.1	
Soluble Salts		0.13 mmhos/cm						0.	12
Organic Matter	WB	2.0 % ENR 81							
Nitrate Nitrogen									
Soil Classification		0							

^{*} Additional results to follow

SOIL FERTILITY GUIDELINES

Crop: Bedding Plants Rec Units: LB/1000 SF

(lbs)	LIME (tons)	N	P ₂ O ₅	K ₂O	Mg	S	В	Cu	Mn	Zn	Fe
35		2.5	2.0	6.0	0		0.50				
Crop:								Rec U	nits:		
		1									

Comments:

Bedding Plants

- · Apply dolomitic lime to raise pH and improve the magnesium level.
- Broadcast boron using Borax and mix into the soil to raise boron level. Note boron should not be applied in the band near the plant.
- · All recommended fertilizers are on actual elemental basis. To convert to product basis, divide the recommended quantity in the first page by the percentage of the active ingredient then multiply by 100.
- For best result, if there are no existing plants, broadcast all lime then till and mix 6 inches into the soil. Limit the lime application to 50 pounds per 1000 sq. ft. for existing plants, apply every 4-6 months until the recommended amount is fulfilled.
- Phosphate is more efficient if applied near the plant, apply all phosphate beside the row. Broadcast N and/or K2O then mix into the soil. If there is no fertilizer meets the ratio, you can use single element fertilizer such as Urea, Triplesuper Phosphate and Muriate of Potash to achieve the requirements. Consult the enclosed instruction sheet on lime and fertilizer application.
- · Apply the amount of lime recommended in first page to raise pH
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SOIL ANALYSIS

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Lab Number: 19382 Field Id: Sample Id: B-19

				SOI	L TEST RATI	NGS		Calculat	ed Cation
Test	Method	Results	Very Low	Low	Medium	Optimum	Very High	Exchange	e Capacity
Soil pH	1:1	5.1						5	.2
Buffer pH		6.74							/100g
Phosphorus (P)	M3	20 ppm	'						ed Cation
Potassium (K)	M3	29 ppm						Satu	ration
Calcium (Ca)	M3	548 ppm						%K	1.4
Magnesium (Mg)	M3	50 ppm			Τ			%Ca	52.7
Sulfur (S)								%Mg	8.0
Boron (B)	M3	0.1 ppm						%Н	37.4
Copper (Cu)								Hmeq	1.9
Iron (Fe)									
Manganese (Mn)									
Zinc (Zn)								K · M	g Ratio
Sodium (Na)									18
Soluble Salts		0.11 mmhos/cm						0.	10
Organic Matter	WB	1.7 % ENR 77							
Nitrate Nitrogen									
Soil Classification		0							

^{*} Additional results to follow

SOIL FERTILITY GUIDELINES

Crop: Bedding Plants Rec Units: LB/1000 SF

(lbs)	LIME (tons)	N	P ₂ O ₅	K ₂O	Mg	S	В	Cu	Mn	Zn	Fe
60		2.5	3.0	6.0	0.69		0.50				
Crop:								Rec U	nits:		

Comments:

Bedding Plants

- · Apply dolomitic lime to raise pH and improve the magnesium level.
- If dolomitic lime is not used, apply required magnesium with magnesium oxide. Epsom Salts, K-Mag or Sul-PO-Mag.
- · Broadcast boron using Borax and mix into the soil to raise boron level. Note boron should not be applied in the band near the plant.
- · All recommended fertilizers are on actual elemental basis. To convert to product basis, divide the recommended quantity in the first page by the percentage of the active ingredient then multiply by 100.
- For best result, if there are no existing plants, broadcast all lime then till and mix 6 inches into the soil. Limit the lime application to 50 pounds per 1000 sq. ft. for existing plants, apply every 4-6 months until the recommended amount is fulfilled.
- Phosphate is more efficient if applied near the plant, apply all phosphate beside the row. Broadcast N and/or K2O then mix into the soil. If there is no fertilizer meets the ratio, you can use single element fertilizer such as Urea, Triplesuper Phosphate and Muriate of Potash to achieve the requirements. Consult the enclosed instruction sheet on lime and fertilizer application.
- · Apply the amount of lime recommended in first page to raise pH
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SOIL ANALYSIS

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Lab Number: 19383 Field Id: Sample Id: B-20

				SOI	L TEST RATII	NGS		Calculate	d Cation
Test	Method	Results	Very Low	Low	Medium	Optimum	Very High	Exchange	Capacity
Soil pH	1:1	5.3						3.	9
Buffer pH		6.81						meq/	
Phosphorus (P)	M3	75 ppm			' 			Calculate	
Potassium (K)	M3	47 ppm						Satura	ation
Calcium (Ca)	M3	402 ppm						%K	3.1
Magnesium (Mg)	M3	63 ppm						%Ca	51.5
Sulfur (S)								%Mg	13.5
Boron (B)	M3	0.1 ppm						%Н	30.6
Copper (Cu)								Hmeq	1.2
Iron (Fe)									
Manganese (Mn)									
Zinc (Zn)								K : Mg	Ratio
Sodium (Na)								0.2	
Soluble Salts		0.1 mmhos/cm						0.2	,
Organic Matter	WB	1.9 % ENR 82							
Nitrate Nitrogen									
Soil Classification		0							

^{*} Additional results to follow

SOIL FERTILITY GUIDELINES

Crop: Bedding Plants Rec Units: LB/1000 SF

(lbs)	LIME (to	ons)	N	P ₂ O ₅	K ₂O	Mg	S	В	Cu	Mn	Zn	Fe
35			2.5	0.5	6.0	0.39		0.50				
Crop:									Rec U	nits:		

Comments:

Bedding Plants

- · Apply dolomitic lime to raise pH and improve the magnesium level.
- If dolomitic lime is not used, apply required magnesium with magnesium oxide. Epsom Salts, K-Mag or Sul-PO-Mag.
- · Broadcast boron using Borax and mix into the soil to raise boron level. Note boron should not be applied in the band near the plant.
- All recommended fertilizers are on actual elemental basis. To convert to product basis, divide the recommended quantity in the first page by the percentage of the active ingredient then multiply by 100.
- For best result, if there are no existing plants, broadcast all lime then till and mix 6 inches into the soil. Limit the lime application to 50 pounds per 1000 sq. ft. for existing plants, apply every 4-6 months until the recommended amount is fulfilled.
- Phosphate is more efficient if applied near the plant, apply all phosphate beside the row. Broadcast N and/or K2O then mix into the soil. If there is no fertilizer meets the ratio, you can use single element fertilizer such as Urea, Triplesuper Phosphate and Muriate of Potash to achieve the requirements. Consult the enclosed instruction sheet on lime and fertilizer application.
- · Apply the amount of lime recommended in first page to raise pH
- For more in depth explanation, go to our website www.al-labs-eastern.com and select the "Lawn and Garden" tab at the top of home page. At the bottom of the "Lawn and Garden" page, you find information explaining a soil test report and fertilizer recommendations. http://al-labs-eastern.com/forms/LawnGardenSoilTestExplained.pdf



7621 Whitepine Road Richmond, Virginia 23237 (804) 743-9401 Fax (804) 271-6446

Submitted By: K BULLOCK

SOIL ANALYSIS

11-159-0530 Client : Report No: T.L.B. ASSOCIATES INC Cust No: T.L.B. ASSOCIATES INC 02323 Date Printed: THOMAS BROWN 06/10/2011 7280 BALTIMORE ANNAPOLIS BLVD Date Received: 06/08/2011 GLEN BURNIE MD 21061 PO: Page: 16 of 20

Lab Number: 19384 Field Id: Sample Id: IT-2

				SOI	L TEST RATII	NGS		Calculated Cation	
Test	Method	Results	Very Low	Low	Medium	Optimum	Very High	Exchange Capacit	.y
Soil pH	1:1	5.4						5.6	
Buffer pH		6.77						meq/100g	
Phosphorus (P)	М3	32 ppm						Calculated Cation	n
Potassium (K)	М3	36 ppm						Saturation	
Calcium (Ca)	М3	654 ppm						%K 1.6	
Magnesium (Mg)	М3	82 ppm						%Ca 58.4	
Sulfur (S)								%Mg 12.2	
Boron (B)	М3	0.1 ppm						%H 28.5	
Copper (Cu)								Hmeq 1.6	
Iron (Fe)								-	
Manganese (Mn)									
Zinc (Zn)								K : Mg Ratio	
Sodium (Na)								0.14	
Soluble Salts		0.12 mmhos/cm						0.14	
Organic Matter	WB	1.6 % ENR 74							
Nitrate Nitrogen									
Soil Classification		0							

^{*} Additional results to follow

SOIL FERTILITY GUIDELINES

Crop: Bedding Plants Rec Units: LB/1000 SF

(lbs)	LIME (tons)	N	P ₂ O ₅	K ₂ O	Mg	S	В	Cu	Mn	Zn	Fe
50		2.5	2.0	6.0	0		0.50				
Crop:								Rec U	nits:		

Comments:

Bedding Plants

- · Apply dolomitic lime to raise pH and improve the magnesium level.
- Broadcast boron using Borax and mix into the soil to raise boron level. Note boron should not be applied in the band near the plant.
- · All recommended fertilizers are on actual elemental basis. To convert to product basis, divide the recommended quantity in the first page by the percentage of the active ingredient then multiply by 100.
- For best result, if there are no existing plants, broadcast all lime then till and mix 6 inches into the soil. Limit the lime application to 50 pounds per 1000 sq. ft. for existing plants, apply every 4-6 months until the recommended amount is fulfilled.
- Phosphate is more efficient if applied near the plant, apply all phosphate beside the row. Broadcast N and/or K2O then mix into the soil. If there is no fertilizer meets the ratio, you can use single element fertilizer such as Urea, Triplesuper Phosphate and Muriate of Potash to achieve the requirements. Consult the enclosed instruction sheet on lime and fertilizer application.
 - · Apply the amount of lime recommended in first page to raise pH
- For more in depth explanation, go to our website www.al-labs-eastern.com and select the "Lawn and Garden" tab at the top of home page. At the bottom of the "Lawn and Garden" page, you find information explaining a soil test report and fertilizer recommendations. http://al-labs-eastern.com/forms/LawnGardenSoilTestExplained.pdf



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11-159-0530 Client: Report No: T.L.B. ASSOCIATES INC Cust No: 02323 T.L.B. ASSOCIATES INC Date Printed: 06/10/2011 THOMAS BROWN 7280 BALTIMORE ANNAPOLIS BLVD Date Received: 06/08/2011 GLEN BURNIE MD 21061 PO: Page: 17 of 20

Lab Number: 19385 Field ld: Sample ld: IT-3

				SOI	L TEST RATIN	IGS		Calculated	Cation
Test	Method	Results	Very Low	Low	Medium	Optimum	Very High	Exchange C	apacity
Soil pH	1:1	6.2						6.9	
Buffer pH		6.85						meq/10	
Phosphorus (P)	M3	22 ppm						Calculated	
Potassium (K)	M3	48 ppm						Saturat	ion
Calcium (Ca)	M3	1013 ppm						%K	1.8
Magnesium (Mg)	M3	100 ppm						%Ca	73.4
Sulfur (S)								%Mg	12.1
Boron (B)	M3	0.3 ppm						%H	12.0
Copper (Cu)								Hmeq	8.0
Iron (Fe)									
Manganese (Mn)									
Zinc (Zn)								K : Mg R	atio
Sodium (Na)								0.15	
Soluble Salts		0.1 mmhos/cm						0.13	
Organic Matter	WB	2.1 % ENR 82							
Nitrate Nitrogen									
Soil Classification		0							

^{*} Additional results to follow

SOIL FERTILITY GUIDELINES

Crop: Bedding Plants Rec Units: LB/1000 SF

(lbs) LI	ME (tons)	N	P ₂ O ₅	K ₂O	Mg	S	В	Cu	Mn	Zn	Fe
0		2.5	2.5	6.0	0		0.50				
Crop:	Crop : Rec Units:										

Comments:

Bedding Plants

- · Broadcast boron using Borax and mix into the soil to raise boron level. Note boron should not be applied in the band near the plant.
- All recommended fertilizers are on actual elemental basis. To convert to product basis, divide the recommended quantity in the first page by the percentage of the active ingredient then multiply by 100.
- Phosphate is more efficient if applied near the plant, apply all phosphate beside the row. Broadcast N and/or K2O then mix into the soil. If there is no fertilizer meets the ratio, you can use single element fertilizer such as Urea, Triplesuper Phosphate and Muriate of Potash to achieve the requirements. Consult the enclosed instruction sheet on lime and fertilizer application.
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Lab Number: 19386 Field ld: Sample ld: IT-5

	l			SOI	L TEST RATII	NGS		Calculate	ed Cation
Test	Method	Results	Very Low	Low	Medium	Optimum	Very High	Exchange	e Capacity
Soil pH	1:1	5.9						5	.5
Buffer pH		6.84							/100g
Phosphorus (P)	M3	31 ppm							ed Cation
Potassium (K)	M3	45 ppm						Satu	ration
Calcium (Ca)	M3	751 ppm		•				%K	2.1
Magnesium (Mg)	M3	80 ppm						%Ca	68.3
Sulfur (S)								%Mg	12.1
Boron (B)	М3	0.2 ppm						%H	17.1
Copper (Cu)								Hmeq	0.9
Iron (Fe)									
Manganese (Mn)									
Zinc (Zn)								K : Mo	Ratio
Sodium (Na)								0.1	
Soluble Salts		0.08 mmhos/cm						0.	'
Organic Matter	WB	1.9 % ENR 80							
Nitrate Nitrogen									
Soil Classification		0							

^{*} Additional results to follow

SOIL FERTILITY GUIDELINES

Crop: Bedding Plants Rec Units: LB/1000 SF

(lbs)	LIME (tons)	N	P ₂ O ₅	K ₂O	Mg	S	В	Cu	Mn	Zn	Fe
20		2.5	2.0	6.0	0		0.50				
Crop:								Rec U	nits:		

Comments:

Bedding Plants

- · Apply dolomitic lime to raise pH and improve the magnesium level.
- Broadcast boron using Borax and mix into the soil to raise boron level. Note boron should not be applied in the band near the plant.
- · All recommended fertilizers are on actual elemental basis. To convert to product basis, divide the recommended quantity in the first page by the percentage of the active ingredient then multiply by 100.
- For best result, if there are no existing plants, broadcast all lime then till and mix 6 inches into the soil. Limit the lime application to 50 pounds per 1000 sq. ft. for existing plants, apply every 4-6 months until the recommended amount is fulfilled.
- Phosphate is more efficient if applied near the plant, apply all phosphate beside the row. Broadcast N and/or K2O then mix into the soil. If there is no fertilizer meets the ratio, you can use single element fertilizer such as Urea, Triplesuper Phosphate and Muriate of Potash to achieve the requirements. Consult the enclosed instruction sheet on lime and fertilizer application.
- · Apply the amount of lime recommended in first page to raise pH
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Submitted By: K BULLOCK

SOIL ANALYSIS

11-159-0530 Client : Report No: T.L.B. ASSOCIATES INC Cust No: 02323 T.L.B. ASSOCIATES INC Date Printed: THOMAS BROWN 06/10/2011 7280 BALTIMORE ANNAPOLIS BLVD Date Received: 06/08/2011 GLEN BURNIE MD 21061 PO: Page: 19 of 20

Lab Number: 19387 Field Id: Sample Id: IT-6

		5 "		SOI	L TEST RATII	NGS			ed Cation
Test	Method	Results	Very Low	Low	Medium	Optimum	Very High	Exchange	Capacity
Soil pH	1:1	5.6						5.	.1
Buffer pH		6.81							100g
Phosphorus (P)	M3	27 ppm						Calculate	
Potassium (K)	M3	40 ppm						Satur	ation
Calcium (Ca)	M3	636 ppm						%K	2.0
Magnesium (Mg)	M3	73 ppm						%Ca	62.4
Sulfur (S)								%Mg	11.9
Boron (B)	M3	0.1 ppm						%H	23.2
Copper (Cu)								Hmeq	1.2
Iron (Fe)									
Manganese (Mn)									
Zinc (Zn)								K : Mg	Ratio
Sodium (Na)								0.1	
Soluble Salts		0.08 mmhos/cm						0.1	'
Organic Matter	WB	2.1 % ENR 85							
Nitrate Nitrogen									
Soil Classification		0							

^{*} Additional results to follow

SOIL FERTILITY GUIDELINES

Crop: Bedding Plants Rec Units: LB/1000 SF

(lbs)	LIME (tons)	N	P ₂ O ₅	K ₂O	Mg	S	В	Cu	Mn	Zn	Fe
35		2.5	2.0	6.0	0.16		0.50				
Crop:								Rec U	nits:		

Comments:

Bedding Plants

- · Apply dolomitic lime to raise pH and improve the magnesium level.
- If dolomitic lime is not used, apply required magnesium with magnesium oxide. Epsom Salts, K-Mag or Sul-PO-Mag.
- · Broadcast boron using Borax and mix into the soil to raise boron level. Note boron should not be applied in the band near the plant.
- All recommended fertilizers are on actual elemental basis. To convert to product basis, divide the recommended quantity in the first page by the percentage of the active ingredient then multiply by 100.
- For best result, if there are no existing plants, broadcast all lime then till and mix 6 inches into the soil. Limit the lime application to 50 pounds per 1000 sq. ft. for existing plants, apply every 4-6 months until the recommended amount is fulfilled.
- Phosphate is more efficient if applied near the plant, apply all phosphate beside the row. Broadcast N and/or K2O then mix into the soil. If there is no fertilizer meets the ratio, you can use single element fertilizer such as Urea, Triplesuper Phosphate and Muriate of Potash to achieve the requirements. Consult the enclosed instruction sheet on lime and fertilizer application.
- · Apply the amount of lime recommended in first page to raise pH
- For more in depth explanation, go to our website www.al-labs-eastern.com and select the "Lawn and Garden" tab at the top of home page. At the bottom of the "Lawn and Garden" page, you find information explaining a soil test report and fertilizer recommendations. http://al-labs-eastern.com/forms/LawnGardenSoilTestExplained.pdf



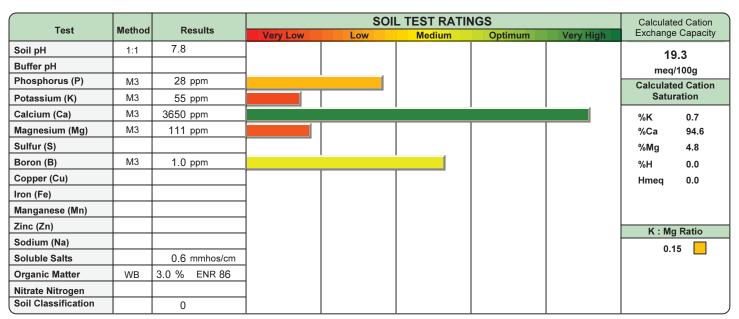
7621 Whitepine Road Richmond, Virginia 23237 (804) 743-9401 Fax (804) 271-6446

Submitted By: K BULLOCK

SOIL ANALYSIS

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Lab Number: 19388 Field Id: Sample Id: IT-10



^{*} Additional results to follow

SOIL FERTILITY GUIDELINES

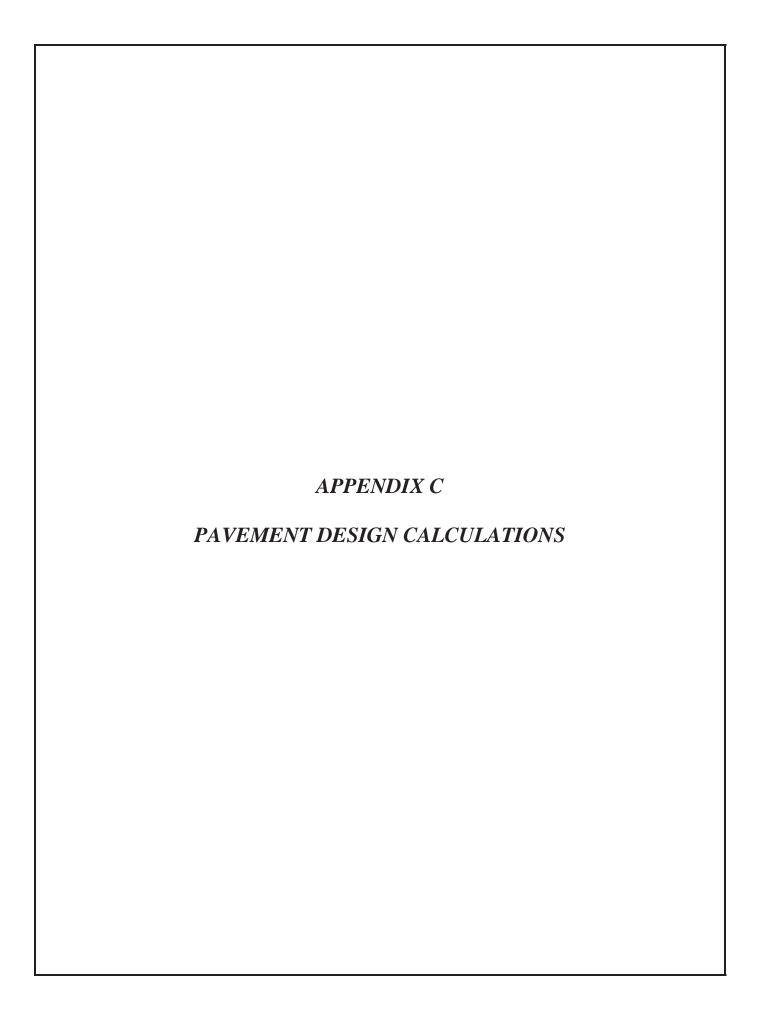
Crop: Bedding Plants Rec Units: LB/1000 SF

(lbs) L	IME (tons)	N	P ₂ O ₅	K ₂O	Mg	S	В	Cu	Mn	Zn	Fe
0		2.5	2.0	5.5	0		0				
Crop:								Rec U	nits:		

Comments:

Bedding Plants

- · Cation Exchange Capacity may be over-estimated due to high pH and free lime in the soil.
- All recommended fertilizers are on actual elemental basis. To convert to product basis, divide the recommended quantity in the first page by the percentage of the active ingredient then multiply by 100.
- Phosphate is more efficient if applied near the plant, apply all phosphate beside the row. Broadcast N and/or K2O then mix into the soil. If there is no fertilizer meets the ratio, you can use single element fertilizer such as Urea, Triplesuper Phosphate and Muriate of Potash to achieve the requirements. Consult the enclosed instruction sheet on lime and fertilizer application.
- Soil pH will come down naturally, it is not necessary to reduce pH if it is below 7.2. To reduce pH, apply 2.5 pounds of elemental sulfur per 1000 square feet for every 0.1 of pH unit above 7.2. For example, pH 7.4 requires 5 pounds. Sulfur should be thoroughly mix 6 inches into the soil
- · Use ammonium sulfate as all or portion of the N requirement to reduce pH.
- · For more in depth explanation, go to our website www.al-labs-eastern.com and select the "Lawn and Garden" tab at the top of home page. At the bottom of the "Lawn and Garden" page, you find information explaining a soil test report and fertilizer recommendations. http://al-labs-eastern.com/forms/LawnGardenSoilTestExplained.pdf



LITTLE BENNETT REGIONAL PARK - DAY USE AREA

Street: PROPOSED ROADWAY

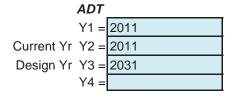
Pavement Design Data

_	_
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	Lab	DCP (Top 12.0")	CBR
Min	0.4	0.0	Laboratory CBR = 0.4
Max	0.6	0.0	
Ave	0.5	not	DCP CBR = not performed
Sdev	0.1	performed	<u>─</u>
Design	0.4		Use CBR = 0.4

Seasonal Roadbed Resillient Modulus (M_R)

CBR (design)	Correlations	M _R (correlated)	Months	Seasons	Factors	Seasonal M _R
0.4	(A) 2555 x CBR ^{0.64} =	1421	Dec - Feb	Winter	1.05	630
	(B) 1500 x CBR =	600	Mar - Apr	Spring	0.9	540
			May - Sept	Summer	1	600
	Min =	600	Oct - Nov	Fall	0.9	540



$$ADT (Y1) = 58$$
 (Given)
 $ADT (Y4) = (Given)$
 $ADT (2011) = ADT (Y2) = 1330$

$$\begin{aligned} r &= 0.010 &(\text{estm.}) \\ \mathbf{r} &= \textbf{1.0\%} \end{aligned}$$

$$ADT (Y2) = ADT (Y1) \times [1 + r]^{0.5(Y2 - Y1)}$$

 $ADT (2011) = 58$

ADT (Y3) = ADT (Y2) x
$$[1 + r]^{0.5(Y3 - Y2)}$$

ADT (2031) = 64

Description	PROPOSED ROADWAY
Functional Classification	Urban Local
ADT (2011)	58
ADT (2031)	64
Posted Speed	25 mph
Truck Traffic	1%
Directional Distribution	50/50

1993 AASHTO Pavement Design

DARWin Pavement Design and Analysis System

A Proprietary AASHTOWare Computer Software Product

Flexible Structural Design Module

Un-Stabilized Subgrade Little Bennett Regional Park Day Use Area Proposed Roadway

Flexible Structural Design

18-kip ESALs Over Initial Performance Period	50,000
Initial Serviceability	4.2
Terminal Serviceability	2.4
Reliability Level	75 %
Overall Standard Deviation	0.45
Roadbed Soil Resilient Modulus	573 psi
Stage Construction	1
Calculated Design Structural Number	4.91 in

Effective Roadbed Soil Resilient Modulus

		Roadbed
		Resilient
<u>Period</u>	<u>Description</u>	Modulus (psi)
1	Dec - Feb	630
2	Mar - Apr	540
3	May - Sept	600
4	Oct - Nov	540

Calculated Effective Modulus 573 psi

Simple ESAL Calculation

Performance Period (years)	-
Two-Way Traffic (ADT)	-
Number of Lanes in Design Direction	-
Percent of All Trucks in Design Lane	- %
Percent Trucks in Design Direction	- %
Percent Heavy Trucks (of ADT) FHWA Class 5 or Greater	- %
Average Initial Truck Factor (ESALs/truck)	-
Annual Truck Factor Growth Rate	- %
Annual Truck Volume Growth Rate	- %
Growth	Simple

Total Calculated Cumulative ESALs

*Note: This value is not represented by the inputs or an error occurred in calculation.

Rigorous ESAL Calculation

Performance Period (years) Two-Way Traffic (ADT) Number of Lanes in Design Direction Percent of All Trucks in Design Lane - %
Percent Trucks in Design Direction - %

			Average Initial	Annual %	Accumulated
	Percent	Annual	Truck Factor	Growth in	18-kip ESALs
Vehicle	of	%	(ESALs/	Truck	over Performance
<u>Class</u>	<u>ADT</u>	<u>Growth</u>	Truck)	<u>Factor</u>	<u>Period</u>
Total	0	-	-	-	0

Growth Simple

Total Calculated Cumulative ESALs 0 *

Specified Layer Design

		Struct	Drain			
		Coef.	Coef.	Thickness	Width	Calculated
<u>Layer</u>	Material Description	<u>(Ai)</u>	(Mi)	(Di)(in)	<u>(ft)</u>	<u>SN (in)</u>
1	Surface PG	0.44	1	2	-	0.88
2	Base PG	0.42	1	3	-	1.26
3	Graded Aggregate Base	0.11	1	26	-	2.86
Total	-	-	-	31.00	-	5.00

Layered Thickness Design

Thickness precision Actual

		Struct	Drain	Spec	Min	Elastic		Calculated	
		Coef.	Coef.	Thickness	Thickness	Modulus	Width	Thickness	Calculated
Layer	Material Description	(Ai)	(Mi)	(Di)(in)	(Di)(in)	(psi)	<u>(ft)</u>	<u>(in)</u>	SN (in)
Total	_	_	_	_	_	_	_	_	_

^{*}Note: This value is not represented by the inputs or an error occurred in calculation.

Optimized Layer Design

		Struct	Drain		Min	Max		Optimum		Calculated
		Coef.	Coef.	Cost	Thick	Thick	Width	Thick	Calculated	Cost
Layer	Material Description	<u>(Ai)</u>	(Mi)	(sq yd/in)	(Di)(in)	<u>(in)</u>	<u>(ft)</u>	<u>(in)</u>	SN (in)	<u>(sq yd)</u>
Total	_	_	_	_	_	_	_	_	_	

^{*}Note: This value is not represented by the inputs or an error occurred in calculation.

^{*}Note: This value is not represented by the inputs or an error occurred in calculation.

1993 AASHTO Pavement Design

DARWin Pavement Design and Analysis System

A Proprietary AASHTOWare Computer Software Product

Flexible Structural Design Module

Stabilized Subgrade Little Bennett Regional Park Day Use Area Proposed Roadway

Flexible Structural Design

18-kip ESALs Over Initial Performance Period	50,000
Initial Serviceability	4.2
Terminal Serviceability	2.4
Reliability Level	75 %
Overall Standard Deviation	0.45
Roadbed Soil Resilient Modulus	573 psi
Stage Construction	1
Calculated Design Structural Number	4.91 in

Effective Roadbed Soil Resilient Modulus

		Roadbed
		Resilient
<u>Period</u>	<u>Description</u>	Modulus (psi)
1	Dec - Feb	630
2	Mar - Apr	540
3	May - Sept	600
4	Oct - Nov	540

Calculated Effective Modulus 573 psi

Simple ESAL Calculation

Performance Period (years)	-
Two-Way Traffic (ADT)	-
Number of Lanes in Design Direction	-
Percent of All Trucks in Design Lane	- %
Percent Trucks in Design Direction	- %
Percent Heavy Trucks (of ADT) FHWA Class 5 or Greater	- %
Average Initial Truck Factor (ESALs/truck)	-
Annual Truck Factor Growth Rate	- %
Annual Truck Volume Growth Rate	- %
Growth	Simple

Total Calculated Cumulative ESALs

*Note: This value is not represented by the inputs or an error occurred in calculation.

Rigorous ESAL Calculation

Performance Period (years) Two-Way Traffic (ADT) Number of Lanes in Design Direction Percent of All Trucks in Design Lane - %
Percent Trucks in Design Direction - %

			Average Initial	Annual %	Accumulated
	Percent	Annual	Truck Factor	Growth in	18-kip ESALs
Vehicle	of	%	(ESALs/	Truck	over Performance
Class	<u>ADT</u>	<u>Growth</u>	Truck)	<u>Factor</u>	<u>Period</u>
Total	0	=	-	-	0

Growth Simple

Total Calculated Cumulative ESALs 0 *

Specified Layer Design

		Struct	Drain			
		Coef.	Coef.	Thickness	Width	Calculated
<u>Layer</u>	Material Description	<u>(Ai)</u>	<u>(Mi)</u>	(Di)(in)	<u>(ft)</u>	<u>SN (in)</u>
1	Surface PG	0.44	1	1.5	-	0.66
2	Base PG	0.42	1	3	-	1.26
3	Stabilized Subgrade mixed with Gr	0.2	1	15	-	3.00
Total	-	-	-	19.50	-	4.92

Layered Thickness Design

Thickness precision Actual

		Struct	Drain	Spec	Min	Elastic		Calculated	
		Coef.	Coef.	Thickness	Thickness	Modulus	Width	Thickness	Calculated
Layer	Material Description	(Ai)	(Mi)	(Di)(in)	(Di)(in)	(psi)	<u>(ft)</u>	<u>(in)</u>	SN (in)
Total	_	_	_	_	_	_	_	_	_

^{*}Note: This value is not represented by the inputs or an error occurred in calculation.

Optimized Layer Design

		Struct	Drain		Min	Max		Optimum		Calculated
		Coef.	Coef.	Cost	Thick	Thick	Width	Thick	Calculated	Cost
Layer	Material Description	<u>(Ai)</u>	(Mi)	(sq yd/in)	(Di)(in)	<u>(in)</u>	<u>(ft)</u>	<u>(in)</u>	SN (in)	(sq yd)
Total	_	_	_	_	_	_	_	_	_	

^{*}Note: This value is not represented by the inputs or an error occurred in calculation.

^{*}Note: This value is not represented by the inputs or an error occurred in calculation.

Design Information

			Design Assistant
Design Life (years)	20		
Initial Serviceability	4.2		ESALs
Terminal Serviceability	2.5		Subgrade Classification
Reliability	75%		Subgrade Conditions
Z_{R}	-0.674		
Standard Deviation	0.45		Base Type
	Layer Coefficient	Thickness	
Pavers and Bedding	0.44	130.175	Recommended Design
Unbound Dense Graded Base	0.11		
Asphalt Treated Base	0.28		Pavers and Bedding
Cement Treated Base	0.20		Unbound Dense Graded Bas
Base Asphalt Concrete Base	0.42		Unbound Dense Graded Sub
Unbound Dense Graded Subbase	0.09		

50,000	CI, CH	Poor	Unbound Dense Graded Base		
ESALs	Subgrade Classification	Subgrade Conditions	Base Type	Recommended Design	

Pavers and Bedding	130.175
Unbound Dense Graded Base	100
Unbound Dense Graded Subbase	475

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- Indicate i resolution	Darrent Commercial	# Shace	TOTAL PARTY	

	mmercial # Lanes % 2 % 2 % 2 % 2 % 2 % 2 % 2 % 2 % 2 % 2 % 2 % 2 % 2 % 2 % 2 % 2 % 2 % 5	Functional Category Percent Commercial # Lanes Local 0.5% 2 Local 1% 2 Local 1% 2 Minor Arterial/Collector 3% 2 Vajor Arterial 5% 2 Principal Arterial 5% 2 Principal Arterial 5% 2	Estimated ESALs Total ESALs	719 14,387	2825 56,498	13516 270,313	79521 1,590,414	196501 3,930,025	259825 5,196,505	509163 10 183 257
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		_								
	litions (MPa)	Poor	70	50	35	30	25	18	15	7
	Resilient Modulus (MR) for Typical Subgrade Conditions (MPa)	Fair	80	70	20	35	30	25	20	10
	Resilient Moc	Poop	06	08	70	50	40	30	27	30
	Susceptibility to Frost	Action	None	Negligible	Negligible to slight	Slight to moderate	Negligible to slight	Severe	Slight to severe	Nocinciple to exercise
		Drainage Characteristics	Excellent	Excellent	Excellent to fair	Fair to semi-impervious	Practically impervious	Typically poor	Practically impervious	and in the second secon
port		Classification	Boulders/coubbles	GW, SW	GP, SP	GM, SM	GC, SC	ML, MI	CL, MH	17.17
subgrade support	Category	No.	1	2	3	4	5	9	7	0

STORMWATER MANAGEMENT CONCEPT PLAN

Little Bennett Day Use Area – Facility Plan

Stormwater Concept #: 239813

September 6, 2011

Submitted To:

Montgomery County
Department of Permitting Services
255 Rockville Pike, 2nd Floor
Rockville, Maryland 20850

Prepared For:

The Maryland National Capital Park and Planning Commission Park Development Division 9500 Brunnet Avenue Silver Spring, Maryland 20901

Prepared By:

A. Morton Thomas and Associates, Inc. 12750 Twinbrook Parkway Rockville, Maryland 20852-1700

Tel: 301-881-2545 Fax: 301-881-0814

AMT: 108-157.008

"Professional Certification: I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the state of Maryland, License No. 32151 Expiration Date: 07/18/2013."

Matthew Ernest, P.E.

September 6, 2011

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VICINITY MAP / LOCATION MAP

Figure-1 Little Bennett Regional Park Gateway Area

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- 4.2. Documentation of Applications to State and Federal Agencies for Wetlands Permits
- 4.3. Documentation of Impervious Area
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- 4.13. Groundwater Recharge

5. Stormwater Management Concept

- 5.1. Stormwater Management Concept Narrative
- 5.2. Environmental Site Design Narrative
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- 5.4. Water Quantity Controls
- 5.5. Leadership in Energy and Environmental Design
- 5.6. Anticipated Performance Special Protection Area Design Standards
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 - 5.6.2. Minimize Storm Flow Run-off Increase
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 - 5.6.4. Minimize Sediment Loading
 - 5.6.5. Minimize Nutrient Loading

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APPENDICES

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APPENDIX B: STORMWATER MANAGEMENT CONCEPT

B1: ENVIRONMENTAL SITE DESIGN (ESD) EXHIBITS
B2: ENVIRONMENTAL SITE DESIGN (ESD) CALCULATIONS

B3: 2007 LITTLE BENNETT MASTER PLAN (FOR COMPARISON PURPOSES)

APPENDIX C: CONCEPT EROSION CONTROL PLANS

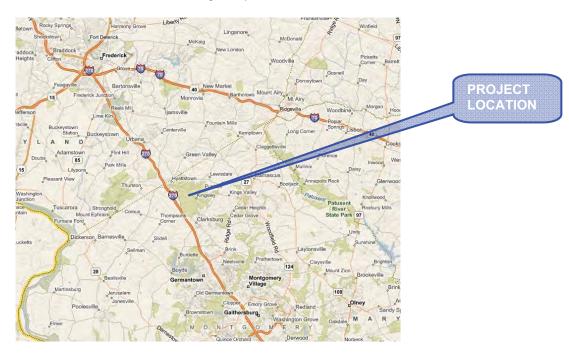
APPENDIX D: APPROVED NRI/FSD

APPENDIX E: FLOODPLAIN STUDY

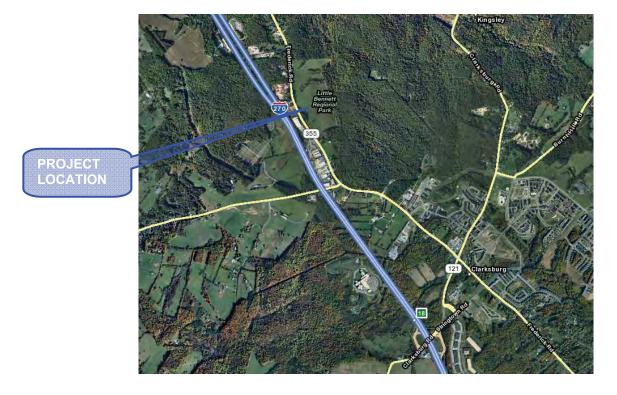
APPENDIX F: GEOTECHNICAL REPORT

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Vicinity Map



Location Map



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STORMWATER MANAGEMENT CONCEPT PLAN

Little Bennett Day Use - Facility Plan

1.0 INTRODUCTION

The Maryland- National Capital Park and Planning Commission (M-NCPPC) plans to develop a distinct entrance and Day Use Area for the Little Bennett Park. The improvements are to be focused on nature based recreation and retreat. The entrance and Day Use area will be located on a 137.62 acre parcel, located along the Park's frontage off of Frederick Avenue (MD Route 355). The project area is bordered by Park land and existing residential lots. Figure 1 below shows an aerial image of the project site.



Figure 1 - Little Bennett Regional Park Gateway Area

The proposed project has been developed to comply with the Environmental Site Design requirements as required by the Maryland Department of the Environment and Montgomery County DPS. The proposed stormwater management facilities will treat stormwater runoff from the new project's impervious areas, as well as portions of Existing Frederick Avenue (MD Route 355) along the project's frontage.

2.0 PURPOSE

This report evaluates and quantifies the Environmental Site Design (ESD) requirements, to the Maximum Extent Practical, in accordance with the Stormwater Management Act of 2007.

3.0 CRITERIA

All work was performed in accordance with the <u>2010 Stormwater Guidelines for State and Federal Projects</u> and the <u>Maryland Department of the Environment (MDE) Stormwater Management Design Manual Volumes I and II,</u> and the <u>Montgomery County Government Storm Drain Design Criteria, Department of Transportation</u>, dated August 1998.

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Stormwater Management requirements were determined using the procedures specified within Chapter 5, Revised 2009, of the <u>MDE Stormwater Management Design Manual</u>. Runoff curve numbers (RCN) and time of concentration (Tc) were determined using the Soil Conservation Service (SCS) TR-55 method for <u>Urban Hydrology for Small Watersheds</u>, dated June 1986. Run-off rates were calculated using TR-20 <u>Project Formulation-Hydrology</u> and Soil Conservation Service (SCS) TR-55 method for <u>Urban Hydrology for Small Watersheds</u>, dated June 1986.

4.0 Environmental Site Design Summary

4.1 Description of the Proposed Development

The M-NCPPC is acting on the recommendation of the 2007 Master Plan for Little Bennett Regional Park to develop a distinct entrance to the Park, and a Day Use Area focused on nature-based recreation and retreat. The creation of this gateway area will use design elements to welcome visitors to the largest regional park in the area, and offer recreation and interpretive opportunities that connect visitors to the beauty of the regional landscape, and the specific attributes of the site. To that end, M-NCPPC's designers are proposing nature-based learning opportunities, an adventure playground, trails, an amphitheater, picnic areas, an overlook area, and creating new meadow habitats. These elements are being designed with the intention of highlighting the unique characteristics of the site.

4.2 Documentation of Applications to State and Federal Agencies for Wetlands Permits

The Little Bennett Day Use project is currently in the Concept Planning stage. As such, applications to State and Federal agencies for Wetland, Waterway and Floodplain permits are not being filed at this time. In the subsequent stages of design, the selected engineering team will apply for all required permits including the appropriate Wetland, Waterway and Floodplain permits.

4.3 Documentation of Impervious Area

The Little Bennett Day Use project is situated on a parcel of land that is majority meadow and woods, and is considered New Development. The total disturbed area (LOD) covered by this report is 25.44 acres. An Impervious Area summary, and Alternative Surface Area summary are shown in Table-1 and Table-2 below:

Table-1: Impervious Area Sumn	nary
Existing Route 355 Pavement	0.76 acres
Proposed Pavement, Gravel, and Building Roof Areas	4.66 acres
Trails (Compacted, Treated as Impervious)	0.66 acres
Total Impervious Area	6.08 acres

Table-2: Alternative Surface Area Summary				
Permeable Pavement and Permeable Playground surface 1.40				
Reinforced Turf	0.10			
Building Green Roof	.02			
Total Alternative Surface Area	1.52 acres			

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4.4 Slopes

The Approved NRI/FSD, prepared by A. Morton Thomas and included in Appendix D, indicates areas with slopes between 15% and 25%, and areas with slopes greater than 25%. Disturbance to existing steep slopes will be minimized to the maximum extent practical.

4.5 Soils

According to the *Natural Resource Conservation Service Soil Survey of Montgomery County, Maryland* (USDA 1973), the study area is underlain by soils of Glenville Silt Loam (5B, 3-8% slopes), Linganore-Hyattstown Silt Loam (9B, 3-8% slope), Linganore-Hyattstown Silt Loam (9C, 8-15% slope), Brinklow-Blocktown Silt Loam (16B; 3-8% slope), Brinklow-Blocktown Silt Loam (16C; 8-15% slope), and Hyattstown Silt Loam (109D, 15-25%) soil series. Additional Soils information is provided in Appendix A. A Geotechnical Report is included in Appendix F.

4.6 Forests

The study area for the Natural Resource Inventory/Forest Stand Delineation, included in Appendix D, consists mainly of meadow with fescue as the dominant species. The majority of forest areas on-site are located within the Stream Buffer surrounding Soper Branch. There are no Montgomery County of State Champion Trees on the parcel, and no trees on the parcel have a DBH greater than or equal to 75% of a County or State Champion tree.

4.7 Rare, Threatened and Endangered Species

There are no know records nor readily observable rare, threatened or endangered (RTE) species nor critical habitats located on the subject parcel.

4.8 <u>Streams and Stream Valley Buffers</u>

As indicated on the approved NRI/FSD, included in Appendix D, a stream and associated stream buffer originates at the southern limit of the subject parcel and travels to the north. Three draws or branches join the stream from the West, flowing east and joining with the main stream, Soper Branch. M-NCPPC – Park Development has coordinated minor encroachments into the Stream Buffer with M-NCPPC Environmental Planning. It was determined that these buffer encroachments are acceptable as long as they fall outside of the existing tree line.

4.9 Floodplain

The Federal Emergency Management Administration (FEMA) *Community Panel Number 2400490050B, Montgomery County Maryland (Unincorporated Areas),* effective date July 2, 1979, indicates no Floodplain on site. Montgomery County DPS requires a Floodplain Study for all areas receiving over 30 acres of drainage. A Concept Floodplain Study for Soper Branch is included in Appendix E, as well as a study for a perennial stream on-site which drains 31.66 acres.

4.10 Description of other Mitigation Techniques

The site layout has been refined to limit the proposed impervious area as much as possible. 20' drive aisles are proposed along Meadow Drive, the North/South Park Road, and within the proposed parking areas. Permeable pavements are being provided in the parking areas in lieu of asphalt, where possible,. Along the

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West Side of Meadow Drive, gravel has been used for the shoulder, in lieu of asphalt. The shoulder on the East Side will double as a bike path, so gravel was not used in this location.

4.11 Stabilization Requirements

An engineered sediment control plan will be developed for the project during final design. For now, Concept Erosion Control plans are included in Appendix C. At the direction of M-NCPPC, the project is being designed as if it falls within a Special Protection Area, so redundant sediment control practices will be provided. Super silt fence will be used as a minimum control. Any off-site clean water will be diverted around the project area. Phased grading will be implored to minimize the amount of area disturbed at one time All storm structures conveying off-site water will be built during Phase I to minimize up-stream clean water entering the Limits of Disturbance. The erosion control plans will reflect that vegetative stabilization will be required within 3-days of establishment.

4.12 Phase Grading

Grading will be phased to minimize the amount of disturbed area denuded at one time. The North-South road, Meadow Drive, will be constructed first, including all associated storm drain. Once the roadway is completed and stabilized, construction of the additional park improvements will commence.

4.13 Groundwater Recharge

Groundwater Recharge will be provided through stone reservoirs underneath the proposed ESD facilities.

5.0 STORMWATER MANAGEMENT CONCEPT

5.1 Stormwater Management Concept Narrative

The Little Bennett project has been developed to comply with the Environmental Site Design requirements as required by the Montgomery County Department of Permitting Services (MC DPS) and the Maryland Department of the Environment (MDE). The Stormwater Management design seeks to replicate the natural hydrology of the site by utilizing small-scale Stormwater Management practices to minimize the impact of land development on downstream water resources. The Stormwater Management design calculations for this project are based upon the Environmental Site Design (ESD) criteria established by the Stormwater Management Act of 2007. Therefore, Typical Pre-Developed and Post-Developed runoff conditions (TR-20, TR-55) are not shown herein, except for the Floodplain Studies provided in Appendix C.

5.2 Environmental Site Design - Narrative

One of the goals of Environmental Site design is "Minimizing Site Imperviousness". The original Master Plan for Little Bennett Park, drafted in 2007 and included in Appendix B3, showed meandering roadways throughout the Park. In total, the Master Plan proposed 8,450 LF of roadway within the park. One of the goals of the current design was to decrease the amount of proposed impervious. The current design proposed 3,216 LF of roadway, a decrease of 62%. In addition, 1.40 acres of Permeable Pavement, and 0.02 acres of Green Roof are proposed.

The first step in Environmental Site Design is to determine which of the 15 methods would fit with M-NCPPC's vision of the Park. A table is included below outlining why certain methods are proposed and other methods were not implemented.

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Table 3 – Environmental Site Design Summary

S	O Selection Summary	Proposed (Y/N)	Max Drainage Area	Slope Restrict ions	Other Restrictions	Comments
Alter surfa	native ices:					
A-1	Green Roofs	Yes				A portion of the proposed building is designed as a Green Roof.
A-2	Permeable Pavements	Yes	For applications that exceed 10,000 SF, underlying soils shall have an infiltration rate (f) of 0.52 in/hr.	5% max	Permeable Pavements can not be used in fill, and can not be used in D soils.	Permeable Pavements are proposed for parking areas. Exact Locations are shown on the Impervious Area Map, in Appendix B.
A-3	Reinforced Turf	Yes		1% min; 5% max	Reinforced Turf can not be used in areas of Compacted fill, and are ineffective (although allowable) in D soils.	The Access Paths to both the Underlook Building and the Playscape area are lined with reinforced turf as requested by the Montgomery County Fire Marshal.
Nons	tructural					
N-1	Disconnect of Rooftop Runoff	No	500 sf	5% max	75' /150' max length	Not used due to Topographical Constraints. The majority of site slopes greater than 5%
N-2	Disconnect of Non- Rooftop Runoff	No	1,000 sf	5% max		Not used due to Topographical Constraints. The majority of site slopes greater than 5%
N-3	Sheet flow to Conservation Areas	No		5% max	50' min width/20,000 sf min conservation area	Not used due to Topographical Constraints. The majority of site slopes at greater than 5%

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Micro	o-scale					
pract	ices:					
M-1	Rainwater Harvesting	Limited				At final design, harvesting rainwater to supply the on-site 30,000 gallon Fire Suppression Tank will be explored and considered.
M-2	Submerged Gravel Wetlands	No			Soils infiltrate too well. Drainage Area is insufficient to maintain wet pool	Ground water required
M-3	Landscape Infiltration	No	10,000 sf			Site Area and drainage area sizes create need for increased number of facilities
M-4	Infiltration Berms	Yes			Limited to volume stored behind the berm due to topography	A berm would "break up" the natural slope of the meadow. M-NCPPC wants to preserve the Meadow as much as possible.
M-5	Dry Wells	No	1,000 sf			Not practical for this project type
M-6	Micro- Bioretention	Yes	20,000 sf			2 micro- bioretentions (biofilters) are proposed with this application.
M-7	Rain Gardens	No.	2,000 sf / 10,000 sf			Site Area and drainage area sizes create need for increased number of facilities
M-8	Swales	Yes	1 acre	4% max	ESDv velocity < 1.0 cfs 2'-8' bottom width	2,358 LF of bio- swales are proposed adjacent to Meadow Drive.
M-9	Enhanced Filters	Yes			Used in bioretention facilities	Will be used for recharge to maximum extent practical

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5.3 Environmental Site Design - Implementation

Environmental Site Design Controls will be provided through the use of micro-bioretention facilities (biofilters), bio swales, Green Roofs and Permeable Pavements; strategically located throughout the site. The proposed facilities provide the necessary volume to provide Full ESD for the site. The required groundwater recharge for the project will be provided beneath the underdrains of the micro-bioretention facilities and bio swales. A total of 2,215 linear feet of bio-swale, two (2) micro-bioretentions, 1.40 acres of Permeable Pavement, and 0.02 acres of Green Roof, are proposed with this Concept.

Under proposed conditions, the project site area will consist of 6.08 acres of impervious area. Gravel areas and compacted trails are treated as impervious for the purposes of this analysis. An additional 1.52 acres of Alternative Surface are provided with this design.

For the purpose of calculating ESD requirements, the site was broken into two Study Points, POS-1 and POS-2. A Study Point Map Exhibit is included in Appendix B1.

For POS-1, utilizing a site impervious to POS of 8%, and areas of B, C, and D soils, the resulting weighted Target PE is 1.00" of rainfall (per MDE Table 5.3). The Target PE is then applied to the Limits of disturbance within the drainage area to POS-1; 24.00 acres. Under these conditions, the site has a respective QE of 0.31" of run-off over the site area. An ESD Volume of 26,768 cubic feet shall be treated on-site within POS-1.

For POS-2, utilizing a site impervious to POS of 15%, and areas of B, and D soils, the resulting weighted Target PE is 1.00" of rainfall (per MDE Table 5.3). The Target PE is then applied to the Limits of disturbance within the drainage area to POS-2; 1.44 acres. Under these conditions, the site has a respective QE of 0.51" of run-off over the site area. An ESD Volume of 2,679 cubic feet shall be treated on-site within POS-2.

Tables 4 and 5 below provides a Summary of the PE and ESD Volumes shown on the Stormwater Management Concept Plan.

Table 4 – Environmental Site Design Volumetric Summary – POS-1

Target PE	PE Provided	Target ESD Volume	ESD Volume Provided
(inches)	(inches)	(Cubic feet)	(Cubic Feet)
1.00	1.03	26,768	

Table 5 – Environmental Site Design Volumetric Summary – POS-2

Target F (inches)	_	E Provided (inches)	Target ESD Volume (Cubic feet)	ESD Volume Provided (Cubic Feet)
1.00		1.10	2,679	2,937

Complete ESD design calculations are provided in Appendix B.2.

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An infiltration rate of 26.01 in/hr was observed at the location of the northern parking area. The Geotechnical Engineer views this result as anomalous. Additional infiltration testing will take place during Final Design. To address any DPS concerns regarding the high infiltration rates observed, a second option which does not utilize any permeable pavement was provided for POS-2. The table below provides a Summary of the PE and ESD Volumes for POS-2; Option #2. Full computations are included in Appendix B.2.

Table 6 – Environmental Site Design Volumetric Summary – POS-2; Option #2.

Target PE	PE Provided	Target ESD Volume	ESD Volume Provided
(inches)	(inches)	(Cubic feet)	(Cubic Feet)
1.00	1.01	2,679	2,707

5.4 Water Quantity Controls

With ESD facilities used to the Maximum Extent Practical, a PE of in excess of 1.00" was achieved with a total ESD Volume provided of 30,425 cubic feet. This exceeds the ESD Volume required. Therefore, all Water Quality Channel Protection Volume requirements have been met, and no structural practices are required to provide additional storage volume. Ten year control is not proposed with this application, and will only be provided at the request of Montgomery County DPS.

The stormwater management design strategy for this project is to seek to replicate the natural hydrology of the site by utilizing small-scale stormwater management practices to minimize the impact of land development on downstream water resources. The project was divided into 26 small drainage areas. Each of these drainage areas are tributary to an ESD measure that has been designed to fully meet the ESD criteria to the maximum extent practicable, with a Target PE of 1".

5.5 Leadership in Energy and Environmental Design

The project will utilize several small BMPs to provide ESD treatment to the maximum extent practical. Grass swales, providing pre-treatment, will be used to direct run-off into the bioretention facilities. Grass filter strips or grass swales will be used to pre-treat run-off prior to discharging to infiltration facilities. Redundant treatment systems will be used where practical. Swales are being used to the maximum extent practical in order to minimize storm drain piping systems.

The project is also being designed to meet LEED[©] requirements as governed by the U.S. Green Building Council. The project will attempt to meet the requirements of LEED credits SS 6.1, Stormwater Design: Quantity Control and SS6.2, Stormwater Design: Quality Control.

To achieve LEED Credit SS6.1, the project will implement a stormwater management plan that prevents the post-development peak discharge rate and quantity from exceeding the pre-development peak discharge rate and quantity for the one- and two-year 24-hour design storms, or implement a stormwater management plan that protects receiving stream channels from excessive erosion by implementing a stream channel protection strategy and quantity control strategies.

To achieve LEED Credit SS6.2, a stormwater management plan that reduces impervious cover, promotes infiltration, and captures and treats the stormwater runoff from 90% of the average annual rainfall using acceptable best management practices (BMPs) will be implemented. BMPs used to treat runoff must be capable of removing 80% of the average annual post development total suspended solids (TSS) load based

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on existing monitoring reports. BMPs are considered to meet these criteria if (1) they are designed in accordance with standards and specifications from a state or local program that has adopted these performance standards, or (2) there exists in-field performance monitoring data demonstrating compliance with the criteria. Data must conform to accepted protocol (e.g., Technology Acceptance Reciprocity Partnership [TARP], Washington State Department of Ecology) for BMP monitoring.

The project site is designed to meet LEED requirements through the use of Environmental Site Design criteria. The project site meets all Environmental Site design standards.

5.6 Anticipated Performance – Special Protections Area Design Standards

Even though the subject project is not located within a Special Protection Area, at the direction of M-NCPPPC, AMT is designing this development to meet the below water quality performance goals established in Executive Regulation 29-95. The performance goals applicable to this development are as follows:

- Maintain stream base flow
- Minimize storm flow runoff increases
- o Minimize increases to ambient water temperatures
- o Minimize sediment loading
- Minimize nutrient loadings

Each Performance Goal will be discussed separately.

5.6.1 Maintain Stream Base Flow

Intent: Maximum base flow preservation is essential to supporting the critical habitat needs of the aquatic biological community.

Discussion: Good infiltration rates are anticipated at the site. Enhanced filters will be used at all bioretention and filtering practices to achieve groundwater infiltration and recharge to the maximum extent practical.

5.6.2 Minimize Storm Flow Run-off Increase

Intent: The frequency of runoff events that create erosion in stream channels needs to be managed because cumulative increases in sub watershed runoff affect downstream channels as well as on-site channels.

Discussion: Environmental Site Design (ESD) is being used to the Maximum Extent Practicable (MEP) in order to minimize storm flow run-off increases. The site will be designed such that it's post-development run-off characteristics will mimic an area of Woods in "Good" condition.

5.6.3 Minimize Increases to Ambient Water Temperatures

Intent: Peak stream temperatures and durations are primary determinants of the biological community that can be maintained. During low flow periods of the year, the peaks and durations should not increase beyond the limits of those documented during baseline monitoring or the upper temperature limits specified in State Water Use Standards.

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Discussion: To reduce thermal impacts associated with ESD practices, infiltration at each practice will be maximized. Good infiltration rates are anticipated. Increasing and maximizing infiltration will reduce the amount of surface run-off and will lower the thermal energy flowing into coldwater stream areas.

All filtering practices such as micro-bioretention facilities, bioretention facilities and Bio-swales will be designed such that underdrains will be at least 4 feet below the surface. Soil temperatures at this depth are cooler and fluctuate little in response to surface weather conditions. As run-off flows through the filtering practice, the thermal energy will dissipate and discharge temperatures will be reduced. If underdrain depth is limited, the MDE enhanced filter option 2 will be used. This enhanced filter option utilizes a perforated underdrain located at the bottom of a stone reservoir and below the outfall pipe. As the water surface elevation rises above the invert of the outlet pipe, the cooler water is siphoned from the bottom.

Shade producing plants will be used in landscaped areas. Shade trees and shrubs will be used to screen impervious areas from the sun to the maximum extent practical.

5.6.4 Minimize Sediment loading

Intent: Excessive loadings of fine sediment can deplete the habitat of an entire biological community. Abrasive sediment loadings accelerate stream bed and bank erosion contributing to stream bank instability.

Discussion: During the construction phase, the project will use SSF as a minimum for perimeter erosion control. Phased Grading, as explained in Section 4.13 of this report will be implored to minimize sediment loading, and additional disturbance associated with the construction of larger controls. All sediment controls will be stabilized within 3-days of establishment.

5.6.5 Minimize Nutrient Loading

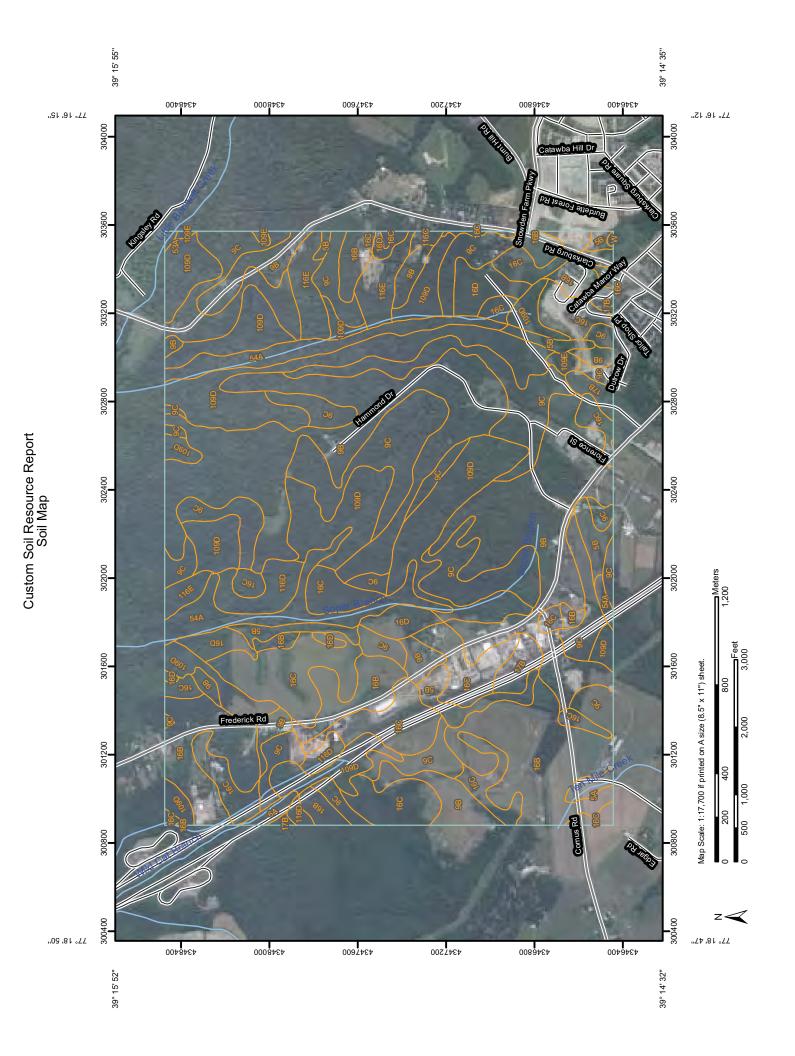
Intent: Excessive nutrient loadings can cause algae blooms and alter the community composition of the stream fish and macro invertebrates. The State has established tributary protection strategies to reduce the impact of nitrogen and phosphorus loadings in the Chesapeake Bay.

Discussion: Minimizing nutrient loading will begin with limiting nutrient input. The use of fertilizers will be minimized. If required, low or no phosphorus fertilizer can be used. A meadow type ground cover will be used to promote filtering and infiltration, thereby, reducing nutrient loading.

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APPENDIX A:

SOILS MAP AND INFORMATION



MAP LEGEND

Area of Interest (AOI)

Very Stony Spot

Wet Spot

Other

Area of Interest (AOI) Soils

Soil Map Units

Special Line Features

Gully

Special Point Features

Borrow Pit Clay Spot Blowout

Short Steep Slope

Other

{

Closed Depression

Gravel Pit

Cities

Political Features

Gravelly Spot

Landfill

Streams and Canals

Oceans

Water Features

Marsh or swamp Lava Flow

Mine or Quarry

Interstate Highways

Rails

ŧ

Fransportation

Major Roads

US Routes

Local Roads

Miscellaneous Water

Perennial Water Rock Outcrop

Saline Spot

Sandy Spot

Sinkhole

Severely Eroded Spot

Slide or Slip

Sodic Spot

Spoil Area

Stony Spot

MAP INFORMATION

Map Scale: 1:17,700 if printed on A size (8.5" × 11") sheet.

The soil surveys that comprise your AOI were mapped at 1:15,840.

Please rely on the bar scale on each map sheet for accurate map measurements.

Web Soil Survey URL: http://websoilsurvey.nrcs.usda.gov Source of Map: Natural Resources Conservation Service Coordinate System: UTM Zone 18N NAD83 This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Montgomery County, Maryland Survey Area Data: Version 7, Feb 2, 2007

Date(s) aerial images were photographed: 6/21/2005; 6/8/2005

imagery displayed on these maps. As a result, some minor shifting The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background of map unit boundaries may be evident.

Map Unit Legend

Montgomery County, Maryland (MD031)				
Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI	
5A	Glenville silt loam, 0 to 3 percent slopes	4.3	0.3%	
5B	Glenville silt loam, 3 to 8 percent slopes	33.7	2.5%	
6A	Baile silt loam, 0 to 3 percent slopes	3.6	0.3%	
9B	Linganore-Hyattstown channery silt loams, 3 to 8 percent slopes	301.9	22.4%	
9C	Linganore-Hyattstown channery silt loams, 8 to 15 percent slopes	251.3	18.6%	
16B	Brinklow-Blocktown channery silt loams, 3 to 8 percent slopes	162.0	12.0%	
16C	Brinklow-Blocktown channery silt loams, 8 to 15 percent slopes	187.5	13.9%	
16D	Brinklow-Blocktown channery silt loams, 15 to 25 percent slopes	34.0	2.5%	
17B	Occoquan loam, 3 to 8 percent slopes	15.2	1.1%	
53A	Codorus silt loam, 0 to 3 percent slopes, occasionally flooded	0.8	0.1%	
54A	Hatboro silt loam, 0 to 3 percent slopes, frequently flooded	56.3	4.2%	
109D	Hyattstown channery silt loam, 15 to 25 percent slopes, very rocky	237.5	17.6%	
109E	Hyattstown channery silt loam, 25 to 45 percent slopes, very rocky	4.1	0.3%	
116C	Blocktown channery silt loam, 8 to 15 percent slopes, very rocky	2.2	0.2%	
116D	Blocktown channery silt loam, 15 to 25 percent slopes, very rocky	22.1	1.6%	
116E	Blocktown channery silt loam, 25 to 45 percent slopes, very rocky	33.4	2.5%	
W	Census water	0.3	0.0%	
Totals for Area of Inte	rest	1,350.3	100.0%	

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape,

September 6, 2011

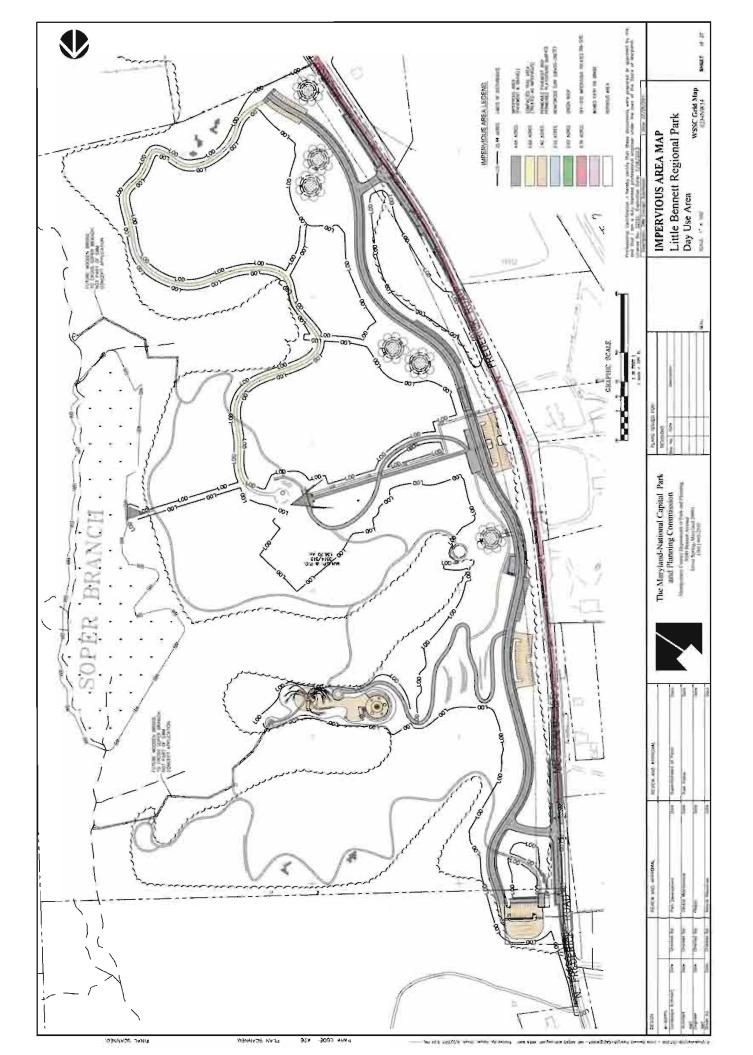
APPENDIX B:

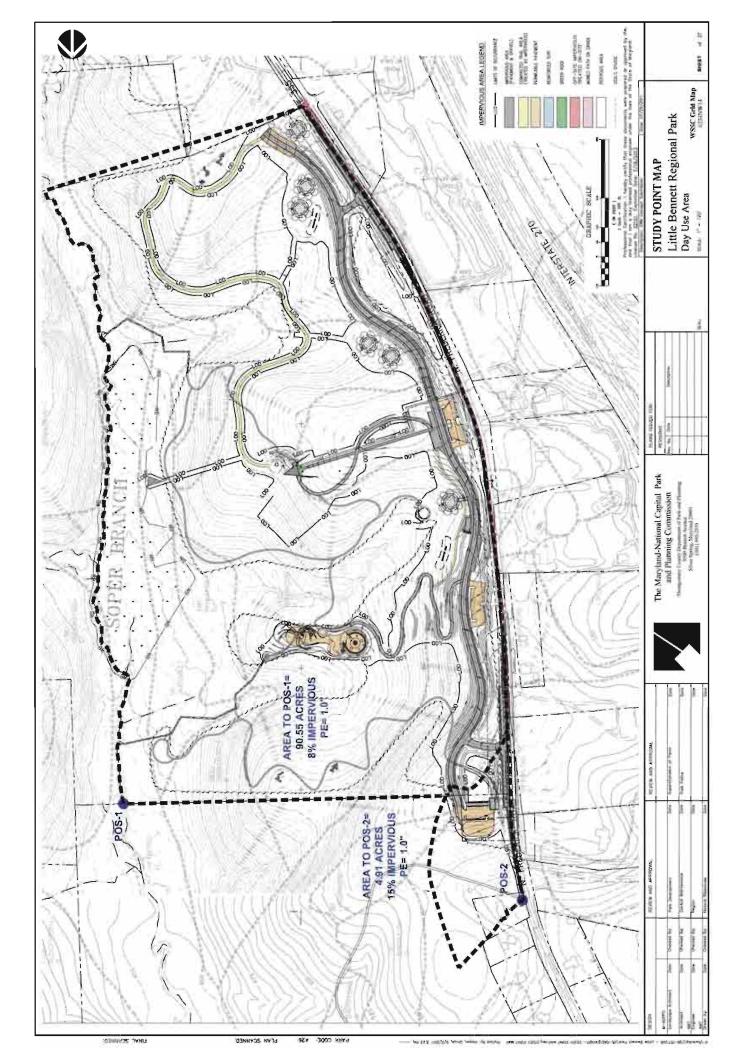
STORMWATER MANAGEMENT CONCEPT

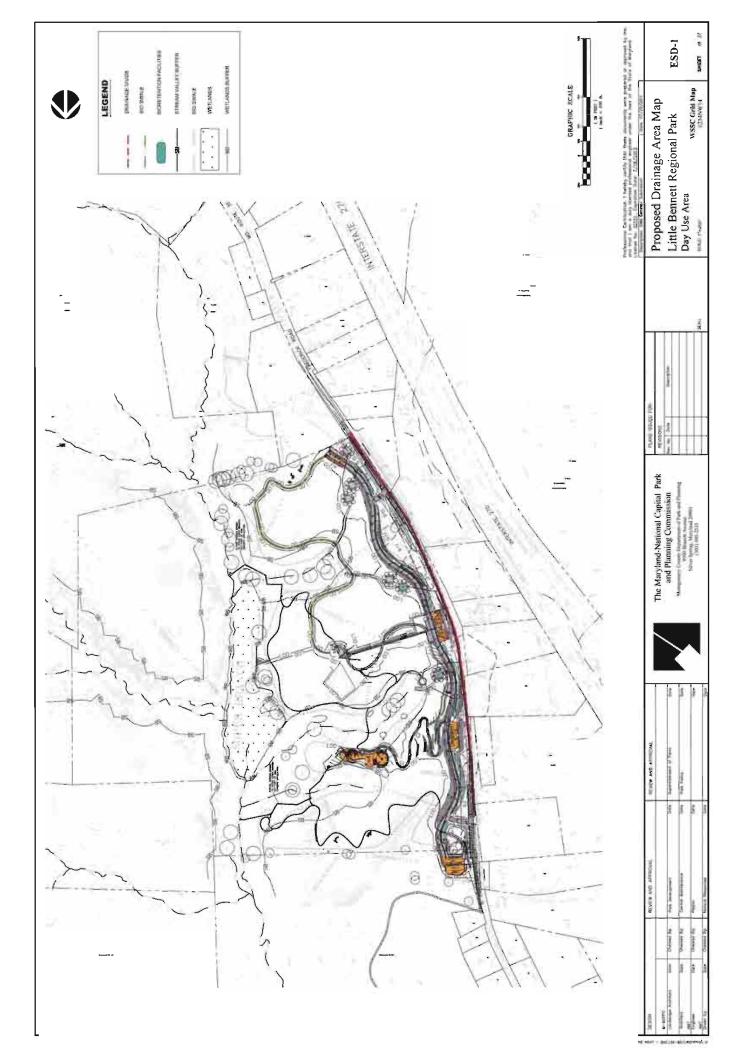
September 6, 2011

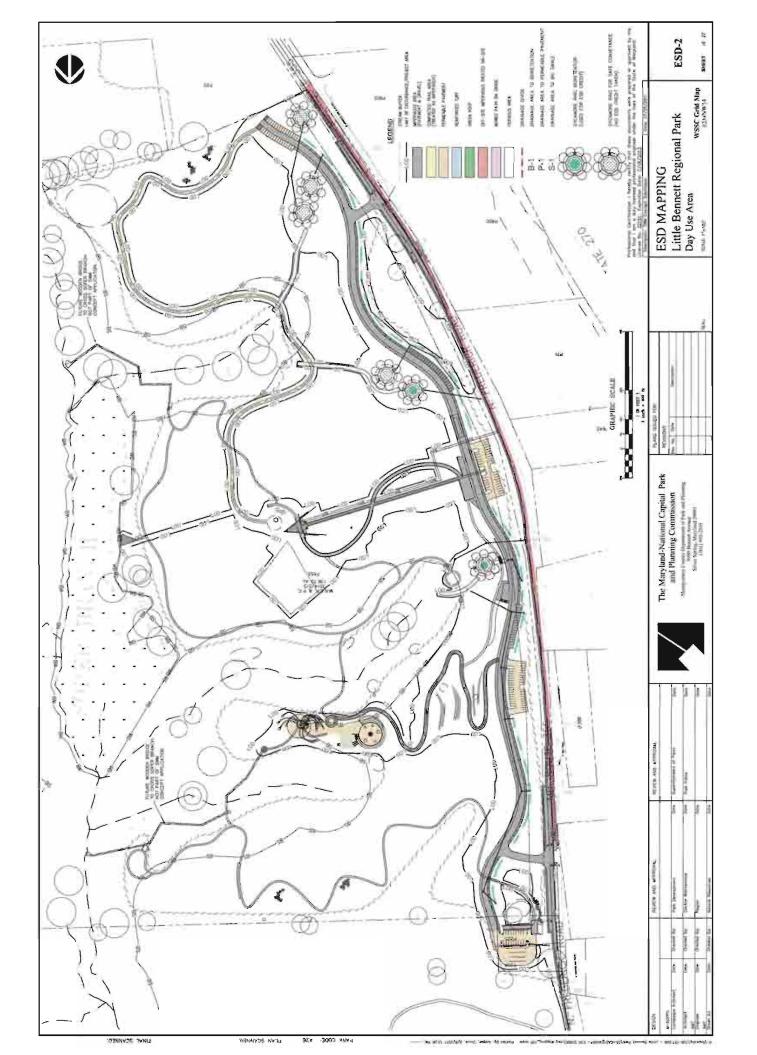
APPENDIX B1:

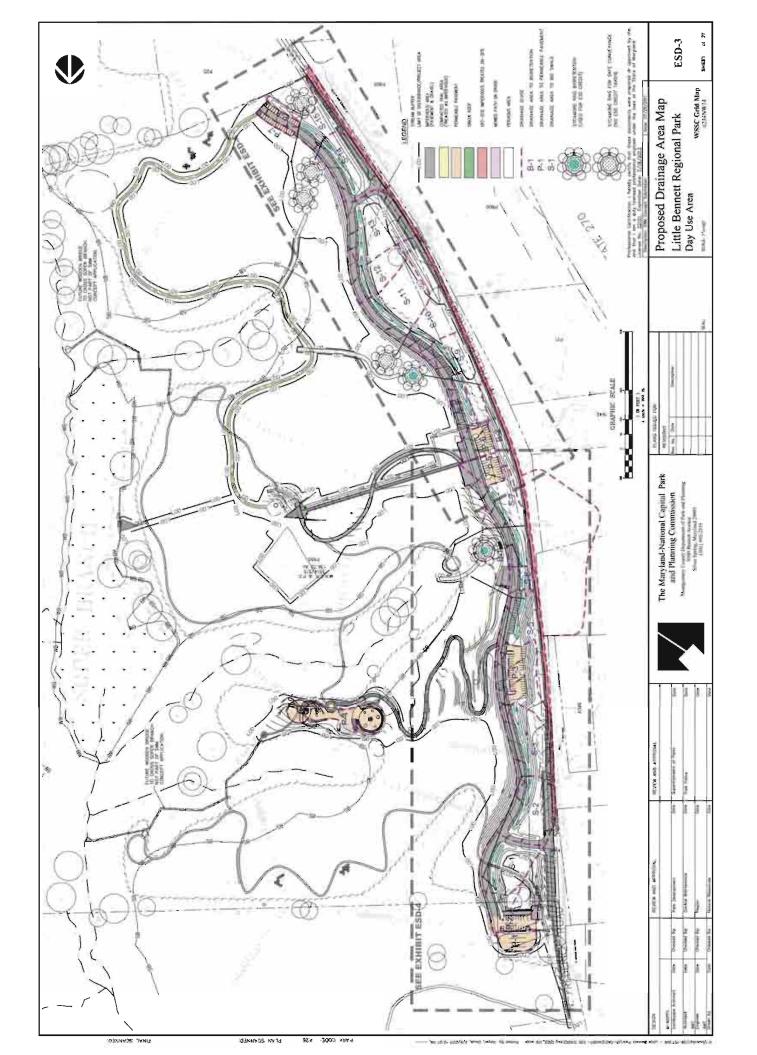
ENVIRONMENTAL SITE DESIGN (ESD) EXHIBITS

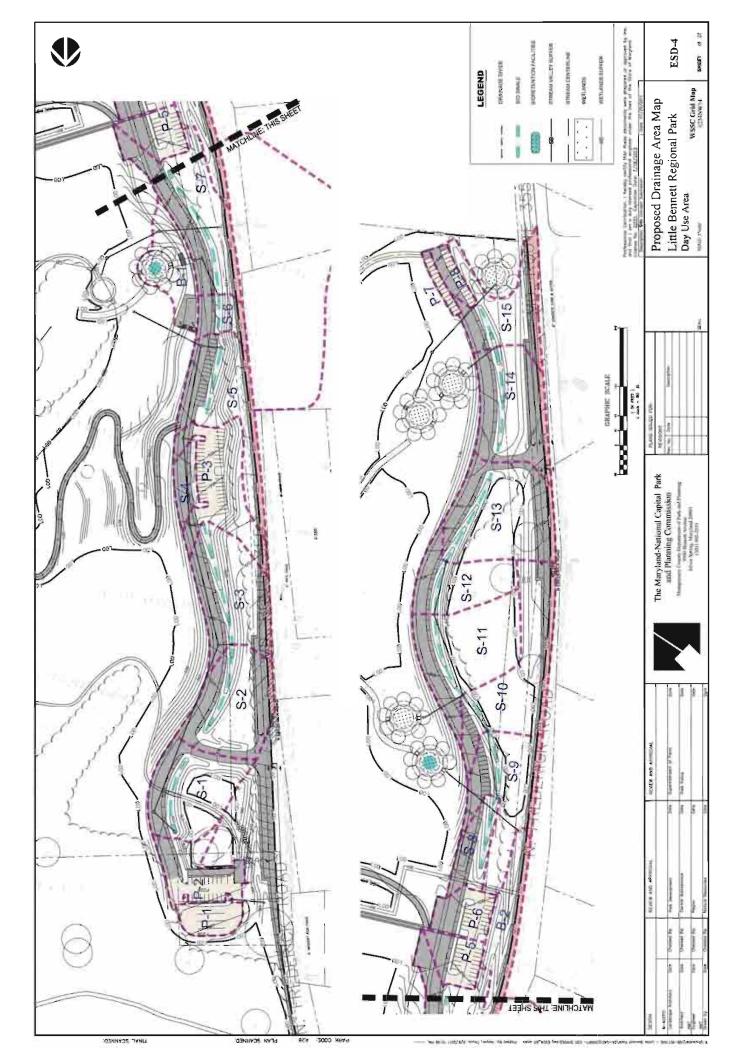


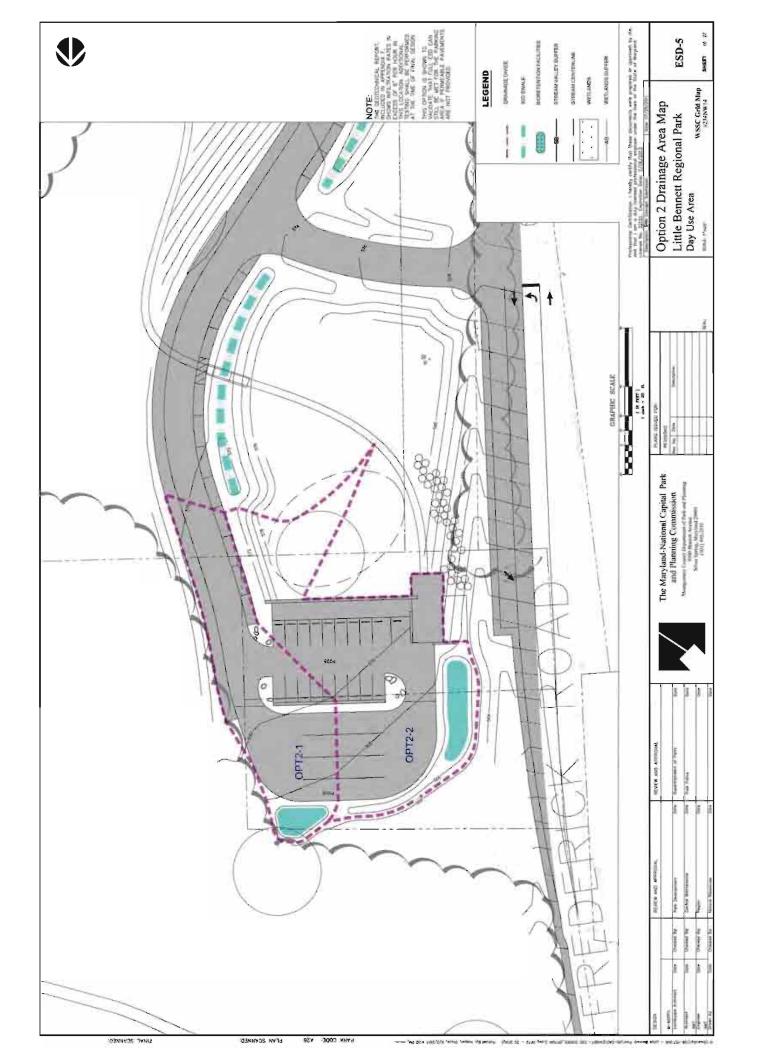


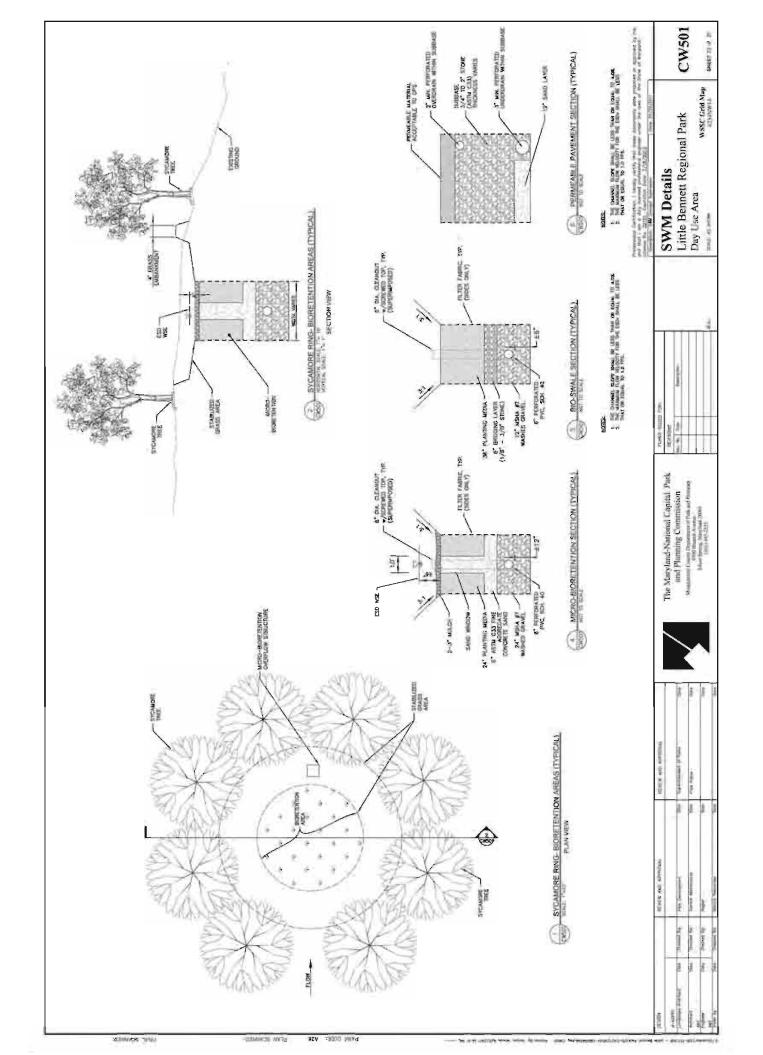












September 6, 2011

APPENDIX B2:

ENVIRONMENTAL SITE DESIGN (ESD) CALCULATIONS



CAH USER: DATE: 9/6/2011 CHECKED BY: MAE PROJECT NO.: 108-157.008

Site Data:

Location: Little Bennett - POS 1

Montgomery County:

1-YR 24 Hour Rainfall:

Area to POS: 90.55

acres Proposed Impervious Area (Total): 6.86 acres*

Proposed % Impervious (I): 8%

*Proposed Impervious includes all Existing Impervious To Remain

Step 1: Determine ESD Implementation Criteria

1A: Determine Pre-Developed Conditions

1A1: Determine Soil Conditions and RCN's for "Woods in Good Condition"

Soil Conditions

HSG	RCN ⁽¹⁾	Area	Percent
A ⁽²⁾	38	0.00	0%
В	55	69.74	77%
С	70	6.91	8%
D	77	13.90	15%
TC	TAL	90.55	100%

(1) RCN for ""Woods in Good Condition" (Table 2-2, TR-55)

(2) Actual RCN is less than 30, Use RCN=38

RCN_{Woods}= (RCN_AxArea_A) + (RCN_BxArea_B) + (RCN_CxArea_C) + (RCN_DxArea_D)

(Area_A + Area_B + Area_C + Area_D)

RCN_{Woods}= 60 (Target RCN)

1B: Determine Target P_E Using Table 5.3

P_E = Rainfall used to size ESD practices

Is project using using Alternative Surfaces (Green Roof, Permeable Pavements)? *MC DPS treats Alternative Surfaces as Impervious for the Purpose of Calculating PE YES

Enter Yes or No

% Impervious:	8			
HSG	% Impervious	Area	PE	(From Table 5.3)
Α		0.00		
В	8	69.74	1.0	
С	8	6.91	1.0	
D	8	13.90	1.0	

90.55

Weighted PE 1.00 inches

1C: Compute Q_{E;}

1D: Identify Projects Limits, Calculate ESD Volume required

Project Limits

(LOD within 24.00 acres

POS -1):

 $Q_E = P_E x R_v$, Where

P_E = 1.00 inches of rainfall

 $R_V = 0.05 + (0.009)(I)$

QE = Runoff Depth Used to Size ESD Practices

1 = 29 (Based on LOD) $R_V =$ 0.31 (Based on LOD)

QE = 0.31 Inches of Runoff

ESD Targets for Project:

 $\begin{array}{c|cccc} \text{es in Floyest.} \\ P_{\text{E}} = & 1.00 & \text{inches of rainfall} \\ Q_{\text{E}} = & 0.31 & \text{inches of runoff} \\ \text{SDv=} & 0.615 & \text{acre-feet} \\ & & 26768 & \text{cubic feet} \\ \end{array}$ ESDv=

1E: Compute Total Site Recharge Volume

Recharge (Percent Volume) = S*Rv*A/12

_		
HSG	S Value	Area
A ⁽²⁾	0.38	0.00
В	0.26	69.74
С	0.13	6.91
D	0.07	0.00
	Total	76.65
	Weighted "S"	0.25

Recharge (Percent Volume) =

Rev 0.576 acre-feet 25074.3 cubic feet

Recharge (Percent Area) = S*Aimp

Rev 1.703 acres 74191.6 square feet



CHECKED BY: MAE PROJECT NO.: 108-157.008

USER: CAH4 DATE: 9/6/2011

27488 27488 2080 1976 2300 11003 676 676 686 686 686 506 686 11020 11020 11113 2288 11030 2253 64 0 ESDv Provided Volume Check: Bioretention Wet Swales 1020 1352 1040 1633 1113 2101 2288 1030 2080 1976 2300 -676 1425 686 2881 B io Swales Grass Swales Raingardens -488 793 Micro-Bioretention Dry Wells Infiltration Berms Infiltration raudscape Wetlands Submerged Gravel Harvesting Rainwater Conservation Area Sheetflow to Rooftop Disconnect of Non-Rooftop Disconnect of Pavements 265 253 Permeable Green Roofs 64 Subarea

ESD Achieved

1.00 26768

Target PE: TOTAL ESDv REQUIRED:

TOTAL ESDV PROVIDED: PE PROVIDED:



USER: CAH
DATE: 9/6/2011
CHECKED BY: MAE
PROJECT NO.: 108-157.008

Site Data:

Location: Little Bennett - POS 2

County: Montgomery

1-YR 24 Hour Rainfall: 2.6

Area to POS: 4.91 acres

Proposed Impervious Area (Total): 0.74 acres*

Proposed % Impervious (I): 15%

*Proposed Impervious includes all Existing Impervious To Remain

Step 1: Determine ESD Implementation Criteria

1A: Determine Pre-Developed Conditions

1A1: Determine Soil Conditions and RCN's for "Woods in Good Condition"

Soil Conditions

HSG	RCN (1)	Area	Percent
A ⁽²⁾	38	0.00	0%
В	55	4.73	96%
С	70	0.00	0%
D	77	0.18	4%
TC	TAL	4.91	100%

(1) RCN for ""Woods in Good Condition" (Table 2-2, TR-55)

(2) Actual RCN is less than 30, Use RCN=38

 $RCN_{Woods} = \frac{(RCN_{A}xArea_{A}) + (RCN_{B}xArea_{B}) + (RCN_{C}xArea_{C}) + (RCN_{D}xArea_{D})}{(RCN_{A}xArea_{A}) + (RCN_{B}xArea_{B}) + (RCN_{C}xArea_{C}) + (RCN_{D}xArea_{D})}$

(Area_A + Area_B + Area_C + Area_D)

RCN_{Woods}= 56 (Target RCN)

1B: Determine Target P_E Using Table 5.3

P_E = Rainfall used to size ESD practices

Is project using using Alternative Surfaces (Green Roof, Permeable Pavements)?
*MC DPS treats Alternative Surfaces as Impervious for the Purpose of Calculating PE

YES Enter Yes or No

% Impervious: 15 PΕ HSG (From Table 5.3) % Impervious Area Α 0.00 В 15 4.73 1.0 С 0.00 D 15 0.18 1.0 4.91

Weighted PE 1.00 inches

1C: Compute Q_{E;}

1D: Identify Projects Limits, Calculate ESD Volume required

Project Limits
(LOD within 1.44 acres POS):

 $Q_E = P_E x R_v$, Where

 $\label{eq:PE} \begin{array}{ll} P_E = & 1.00 & \text{inches of rainfall} \\ R_V = 0.05 + (0.009)(I) & \end{array}$

QE = Runoff Depth Used to Size ESD Practices

I = 51 (Based on LOD) $R_V = 0.51$ (Based on LOD)

QE = 0.51 Inches of Runoff

ESD Targets for Project:

 $\begin{array}{c} P_{\text{E}} = & 1.00 & \text{inches of rainfall} \\ Q_{\text{E}} = & 0.51 & \text{inches of runoff} \\ \text{ESDv} = & 0.062 & \text{acre-feet} \\ & & 2679 & \text{cubic feet} \\ \end{array}$

1E: Compute Total Site Recharge Volume

Recharge (Percent Volume) = S*Rv*A/12

HSG	S Value	Area
A ⁽²⁾	0.38	0.00
В	0.26	4.73
С	0.13	0.00
D	0.07	0.00
	4.73	
	Weighted "S"	0.26

Recharge (Percent Volume) =

Rev 0.055 acre-feet 2375.0 cubic feet

Recharge (Percent Area) = S*Aimp

Rev 0.192 acres

Rev 0.192 acres 8380.9 square feet



CHECKED BY: MAE PROJECT NO.: 108-157.008

USER: CAH4 DATE: 9/6/2011

1.00 1446 1491 0 0 0 2937 1.10 ESDv Provided Target PE: TOTAL ESDv REQUIRED: TOTAL ESDV PROVIDED: PE PROVIDED: 0 0 Volume Check: Bioretention Wet Swales B io Swales Grass Swales Raingardens Micro-Bioretention Dry Wells Infiltration Berms Infiltration гвидесере Wetlands Submerged Gravel Harvesting Rainwater Conservation Area Sheetflow to Rooftop Disconnect of Non-Rooftop Disconnect of Pavements 1446 Permeable Green Roofs Volume

Subarea

ESD Achieved



USER: CAH
DATE: 9/6/2011
CHECKED BY: MAE
PROJECT NO.: 108-157.008

Site Data:

Location: Little Bennett - POS 2

County: Montgomery

1-YR 24 Hour Rainfall: 2.6

Area to POS: 4.91 acres

Proposed Impervious Area (Total): 0.74 acres*

Proposed % Impervious (I): 15%

*Proposed Impervious includes all Existing Impervious To Remain

Step 1: Determine ESD Implementation Criteria

1A: Determine Pre-Developed Conditions

1A1: Determine Soil Conditions and RCN's for "Woods in Good Condition"

Soil Conditions

HSG	RCN (1)	Area	Percent
A ⁽²⁾	38	0.00	0%
В	55	4.73	96%
С	70	0.00	0%
D	77	0.18	4%
TC	TAL	4.91	100%

(1) RCN for ""Woods in Good Condition" (Table 2-2, TR-55)

(2) Actual RCN is less than 30, Use RCN=38

 $RCN_{Woods} = \frac{(RCN_{A}xArea_{A}) + (RCN_{B}xArea_{B}) + (RCN_{C}xArea_{C}) + (RCN_{D}xArea_{D})}{(RCN_{A}xArea_{A}) + (RCN_{B}xArea_{B}) + (RCN_{C}xArea_{C}) + (RCN_{D}xArea_{D})}$

(Area_A + Area_B + Area_C + Area_D)

RCN_{Woods}= 56 (Target RCN)

1B: Determine Target P_E Using Table 5.3

P_E = Rainfall used to size ESD practices

Is project using using Alternative Surfaces (Green Roof, Permeable Pavements)?
*MC DPS treats Alternative Surfaces as Impervious for the Purpose of Calculating PE

YES Enter Yes or No

% Impervious: 15 PΕ HSG (From Table 5.3) % Impervious Area Α 0.00 В 15 4.73 1.0 С 0.00 D 15 0.18 1.0 4.91

Weighted PE 1.00 inches

1C: Compute Q_{E;}

1D: Identify Projects Limits, Calculate ESD Volume required

Project Limits
(LOD within 1.44 acres POS):

 $Q_E = P_E x R_v$, Where

 $\label{eq:PE} \begin{array}{ll} P_E = & 1.00 & \text{inches of rainfall} \\ R_V = 0.05 + (0.009)(I) & \end{array}$

QE = Runoff Depth Used to Size ESD Practices

I = 51 (Based on LOD) $R_V = 0.51$ (Based on LOD)

QE = 0.51 Inches of Runoff

ESD Targets for Project:

 $\begin{array}{c} P_{\text{E}} = & 1.00 & \text{inches of rainfall} \\ Q_{\text{E}} = & 0.51 & \text{inches of runoff} \\ \text{ESDv} = & 0.062 & \text{acre-feet} \\ & & 2679 & \text{cubic feet} \\ \end{array}$

1E: Compute Total Site Recharge Volume

Recharge (Percent Volume) = S*Rv*A/12

HSG	S Value	Area
A ⁽²⁾	0.38	0.00
В	0.26	4.73
С	0.13	0.00
D	0.07	0.00
	4.73	
	0.26	

Recharge (Percent Volume) =

Rev 0.055 acre-feet 2375.0 cubic feet

Recharge (Percent Area) = S*Aimp

Rev 0.192 acres

Rev 0.192 acres 8380.9 square feet



CHECKED BY: MAE PROJECT NO.: 108-157.008

USER: CAH4 DATE: 9/6/2011

1.00 951 1757 0 0 0 2707 1.01 ESD_v Provided TOTAL ESDV PROVIDED: PE PROVIDED: Target PE: TOTAL ESDv REQUIRED: Volume Check: Bioretention Wet Swales B io Swales Grass Swales Raingardens 951 1757 Micro-Bioretention Dry Wells Infiltration Berms Infiltration гвидесере Wetlands Submerged Gravel Harvesting Rainwater Conservation Area Sheetflow to Rooftop Disconnect of Non-Rooftop Disconnect of Pavements Permeable Green Roofs OPT2-1 OPT2-2 Volume Subarea

ESD Achieved

September 6, 2011

APPENDIX B3:

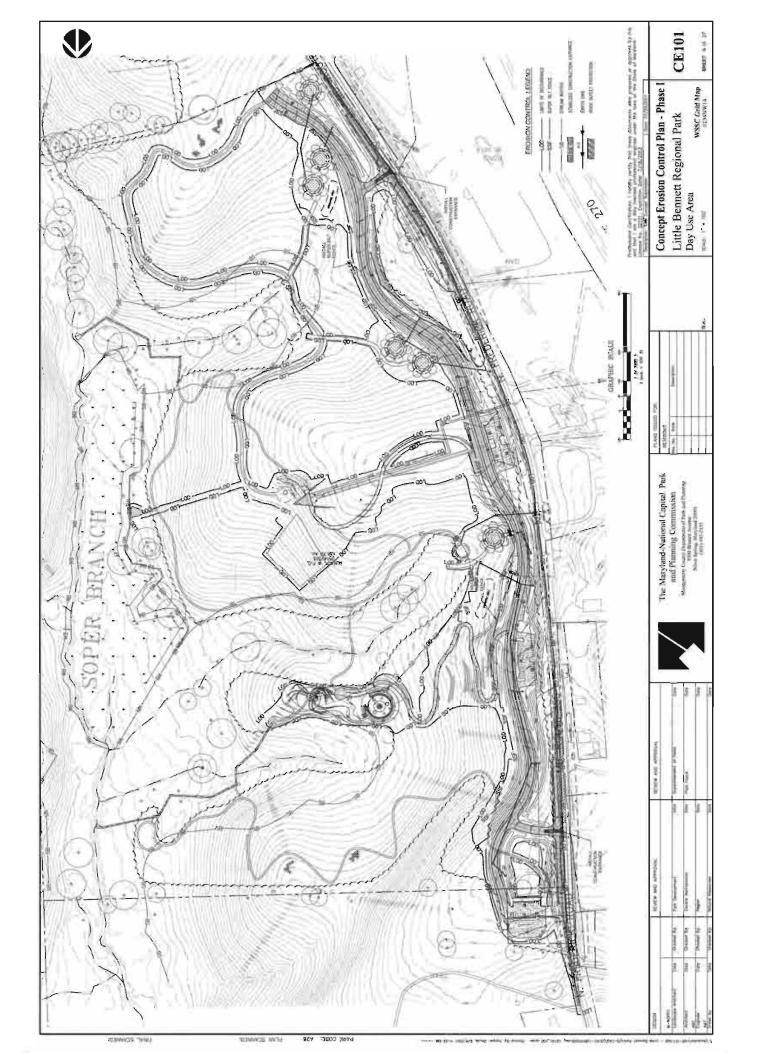
2007 LITTLE BENNETT MASTER PLAN (FOR COMPARISON PURPOSES)

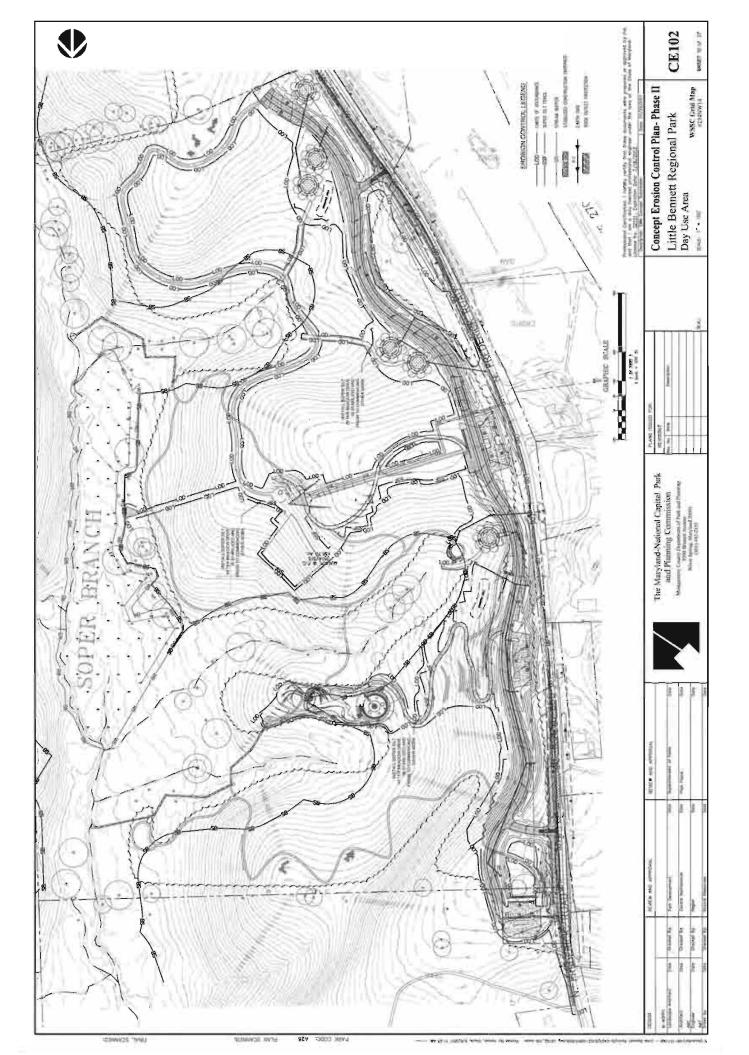
LITTLE BENNETT REGIONAL PARK **GATEWAY AREA** CONCEPT PLAN Meadow Interpretive Area ~ birding ~ butterfly gardens Native American interpretation Secondary Entrance good access/egress visibility at opposite end of day use area Large-Group Picnic Area ~ fire ring ~ Boy scout bridge open space Enhanced buffer Trail Connections along 355 (typ) Small-Group Picnic Area - overlaps w/meadow habitat - amphitheater area - 'shelved' parking (typical) Adventure Playground ~ nature and/or Native American Meadow Garden Overlook Area - native collections sculpture garden seating terrace + demo gardens ~ medicinal plants Focal Point Water Feature Visitor Welcome & Nature Center (tbd) pond, wetlands, deck, pier, island, SWM?, well recharge? on top of meadowed knoll overlook deek parking toward back w/transit stop next to meadow Future acquisition/expansion Primary Entrance - good access/egress visibility - instant view to water feature close/view to Visitor Center

September 6, 2011

APPENDIX C:

CONCEPT EROSION CONTROL PLANS

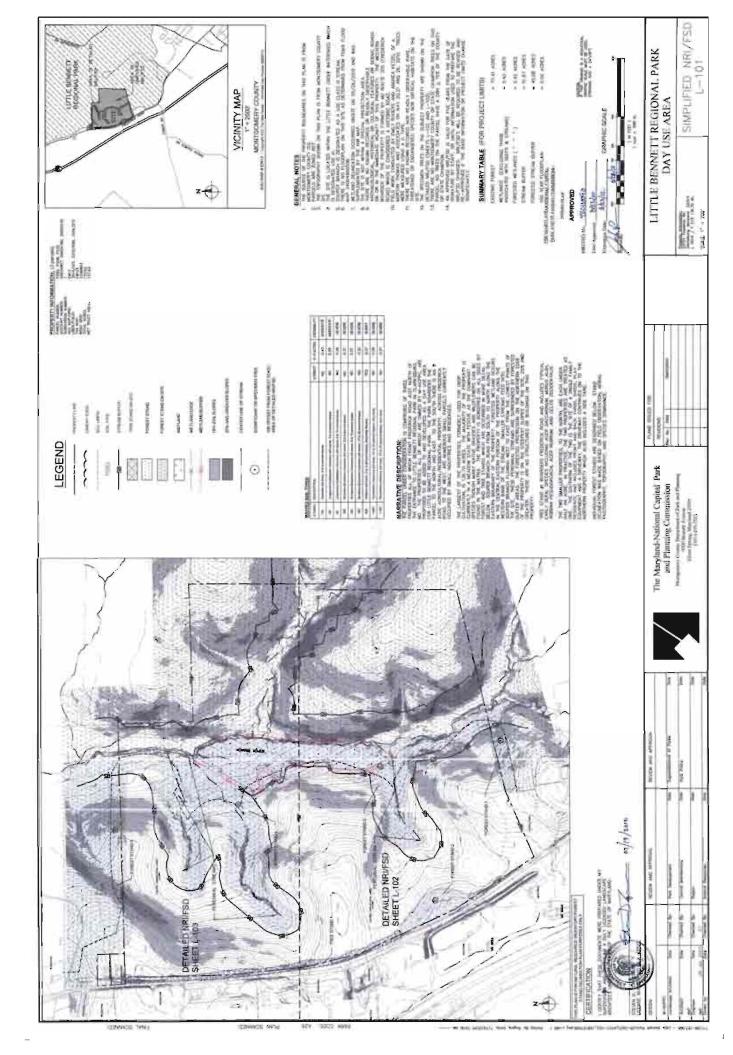


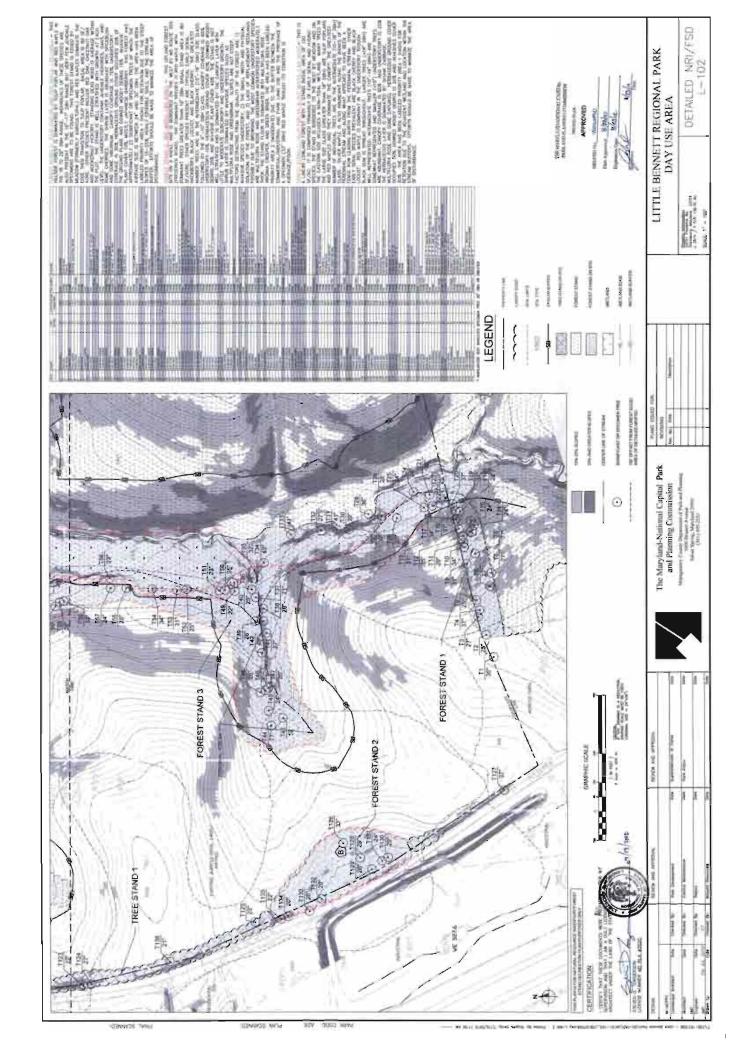


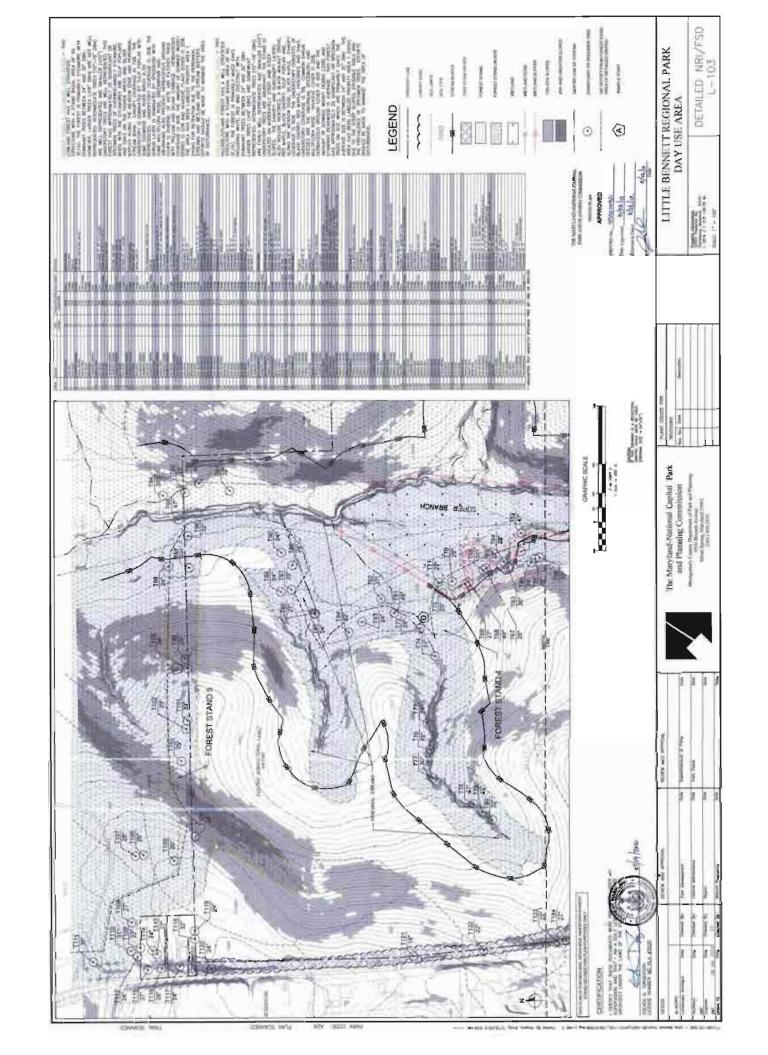
September 6, 2011

APPENDIX D:

APPROVED NRI/FSD





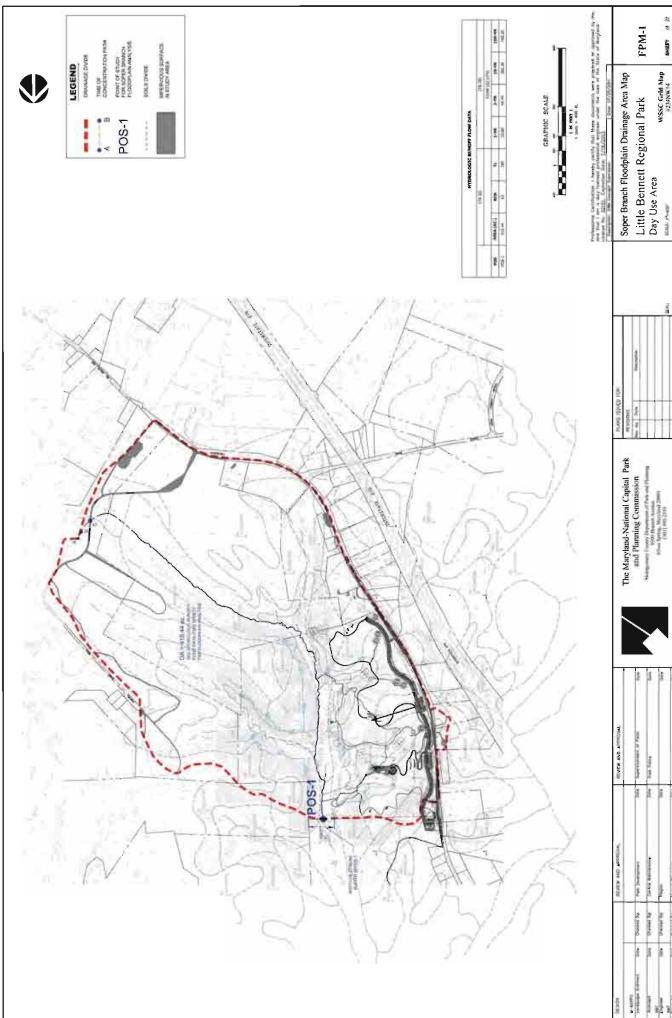


Stormwater Management Concept Plan #239813

September 6, 2011

APPENDIX E:

FLOODPLAIN STUDY



T MAT - BOOLES BOOKERS

Worksheet for Soper Branch 100 Year

Project Description

Friction Method Manning Formula
Solve For Normal Depth

Input Data

Channel Slope 0.01100 ft/ft Discharge 760.05 ft 3 /s

Section Definitions

Station (ft)		Elevation (ft)
	0+00	457.75
	1+30	456.00
	1+36	454.00
	1+44	452.00
	1+47	451.00
	1+51	451.00
	1+54	452.00
	1+60	454.00
	1+72	455.00
:	2+30	454.00
:	2+32	453.00
:	2+34	453.00
:	2+36	454.00
:	2+50	456.00
:	2+63	458.00
:	3+76	477.00

Roughness Segment Definitions

Start Station	Ending	Station	Roughness Coefficient	
(0+	00, 457.75)	(3+76, 477.00)		0.050

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Current Roughness Weighted Pavlovskii's Method
Open Channel Weighting Method Pavlovskii's Method

Worksheet for Soper Branch 100 Year

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Closed Channel Weighting Method Pavlovskii's Method

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\Box	C	0	u	ıι	3

Normal Depth		4.60	ft
Elevation Range	451.00 to 477.00 ft		
Flow Area		182.24	ft²
Wetted Perimeter		117.74	ft
Hydraulic Radius		1.55	ft
Top Width		115.95	ft
Normal Depth		4.60	ft
Critical Depth		4.10	ft
Critical Slope		0.03564	ft/ft
Velocity		4.17	ft/s
Velocity Head		0.27	ft

Flow Type Subcritical

GVF Input Data

Specific Energy

Froude Number

Downstream Depth	0.00	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth

Profile Description		
Profile Headloss	0.00	ft
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	4.60	ft
Critical Depth	4.10	ft
Channel Slope	0.01100	ft/ft
Critical Slope	0.03564	ft/ft

4.87 ft

0.00 ft

0.59

100-Year Cross Section for Soper Branch

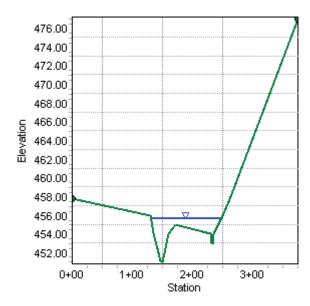
Project Description

Friction Method Manning Formula
Solve For Normal Depth

Input Data

 $\begin{array}{ccc} \text{Channel Slope} & 0.01100 & \text{ft/ft} \\ \text{Normal Depth} & 4.60 & \text{ft} \\ \text{Discharge} & 760.05 & \text{ft}^{3}\text{/s} \\ \end{array}$

Cross Section Image



CAH

Little Bennett Regional Park POS-1 Floodplain Analysis Montgomery County, Maryland

Sub-Area Time of Concentration Details

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimete (ft)	r Velocity (ft/sec)	Travel Time (hr)
POS-1 SHEET SHALLOW CHANNEL	100 200 6731	0.0300 0.0700	0.400 0.050			4.000	0.304 0.013 0.467
				Ti	me of Cor	centration	.784

CAH

Little Bennett Regional Park POS-1 Floodplain Analysis Montgomery County, Maryland

Sub-Area Land Use and Curve Number Details

Sub-Area			Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number
POS-1	Open space; grass cover > 75%	(good) В	83.92	61
	Open space; grass cover > 75%	(good) D	4.38	80
	Paved parking lots, roofs, drivewa	ys	В	17.06	98
	Woods	(good) B	212.97	55
	Woods	(good) C	6.91	70
	Woods	(good) D	90.2	77
	Total Area / Weighted Curve Number			415.44	63
				=====	==

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:	***	**:	****	**80-80	LIST	OF	INPUT	DATA	FOR	TR-20	HY	DRO	LOGY	/**	***	***	*****
JOI	3 TR-20			ECON	1					SUI	MMA	RY	NO	OPL	OTS		
TI	TLE 001	L	TTLE	BENNETT	REG	IONA	AL PARI	K									
TI	ΓLE	S	OPER :	BRANCH F	LOODI	PLAI	IN STUI	PΥ									
6	RUNOFF	1	001	1		0.	.6491		6	3		0.78	341	1	1	1	SOPERDA
	ENDATA																7
7	INCREM	6			0.10												
	ENDCMP	1															
7	COMPUT	7	001	001			2	.1		1.0			2	2	01	01	1Year
	ENDCMP	1															
7	COMPUT	7	001	001			3	. 2		1.0			2	2	01	02	2Year
	ENDCMP	1															
7	COMPUT	7	001	001			5	. 2		1.0			2	2	01	10	10Year
	ENDCMP	1															
7	COMPUT	7	001	001			7	. 2		1.0			2	2	01	99	100Year
	ENDCMP	1															
	END/TOB	2															

TR20 XEQ 8/31/** LITTLE BENNETT REGIONAL PARK
REV 09/01/83 SOPER BRANCH FLOODPLAIN STUDY JOB 1 SUMMARY PAGE 7

SUMMARY TABLE 3 - DISCHARGE (CFS) AT XSECTIONS AND STRUCTURES FOR ALL STORMS AND ALTERNATES

XSECTION/ STRUCTURE

DRAINAGE AREA (SQ MI)

ID

XSECTION 1 .65

ALTERNATE 1 10.89 94.44 385.26 760.05

ECON2/URB1 "FLOW-FREQ" PEAKS FROM SUMMARY TABLE #3 REORDERED IN DECENDING ORDER

Acoustic Report



May 17, 2011

Ms. Ching-Fang Chen Park Development Division The Maryland-National Capital Park and Planning Commission 9500 Brunett Avenue Silver Spring, MD 20901

> Re: Little Bennett Regional Park Day Use Area Acoustical Analysis

Ms. Chen:

This report summarizes the highway noise analysis for the Little Bennett Regional Park Day Use Area project in Montgomery County, MD.

1. Executive summary

A site survey was performed and sound levels were measured in the locations shown in Figure 2 for seven days. Traffic volumes were counted briefly at the end of the survey. The Traffic Noise Model was used to model existing conditions. The output sound levels compared moderately well to the measured sound levels. A traffic forecast was developed based on data provided by the state highway administration. The Traffic Noise Model was used to predict future noise levels in outdoor use areas and at the façade of the one proposed building.

The design goals are to ensure that the Day-Night Average Sound Level (DNL) not exceed 55 dB in most outdoor use areas and 50 dB at the base/stage of the amphitheater, and that the hourly average sound level not exceed 35 dB in the visitor facility multipurpose room.

The year 2030 DNL will be as presented in section 5. The year 2030 DNL may slightly exceed the goal of 50 dB in parts of the base/stage of the amphitheater, may slightly exceed the goal of 55 dB at the picnic shelter, sycamore ring at the retaining wall, and in portions of the amphitheater (at the sloping trail), and will significantly exceed the goal of 55 dB at the park house picnic area. If it were desired to reduce noise levels it would be appropriate to reduce ground elevations in the affected locations, and construct an earthen berm between the park house picnic area and the road. Indoor noise levels will meet the goal in the visitor center multipurpose room.

2. Introduction

Hush Acoustics LLC was contracted by the Maryland-National Capital Park and Planning Commission to perform sound level measurements on the site, to model future noise levels, to design noise barriers, and to design modifications to the proposed building to limit indoor noise levels, as necessary. This analysis was based on the Overall Grading Plans (at various scales) prepared by A. Morton Thomas & Associates, Inc. These drawings show locations of proposed recreation areas, the proposed building, existing ground elevations, selected proposed spot elevations, pavement of Route 355, proposed lanes of



Route 355, and lanes of I-270. The site is located along the east side of Route 355 north of Comus Road and south of Old Hundred Road (Route 109). An aerial photo of the site is included as Figure 1.

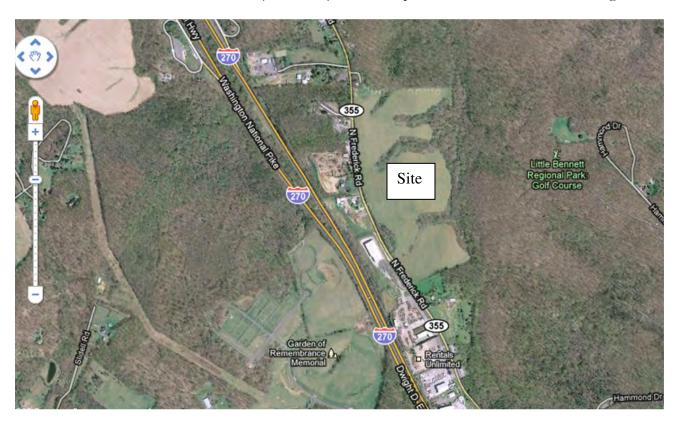


Figure 1. Vicinity Map

It is not clear exactly what the goal should be for noise in a park. The Code of Maryland Environmental Noise Standards are a DNL of 55 dB for a residential zone and 64 dB for a commercial zone; there is no mention of parks. The Montgomery County Staff Guidelines for the Consideration of Transportation Noise Impacts in Land Use Planning and Development dated June 1983 provides outdoor DNL criteria as a function of both site location and community type. Per Table 2-1 of the guidelines, the DNL goal should be 55 dB in permanent rural areas, 60 dB where suburban densities predominate, and 65 dB in the urban ring and along major highway corridors. Again, there is no mention of public parks; the focus is residences. The Montgomery County Staff Guidelines also state that the interior noise guideline is a DNL of 45 dB. Most county and state environmental policies and regulations can historically be traced to the U.S. Environmental Protection Agency (EPA). The EPA guideline is a DNL of 55 dB for outdoor activity interference and annoyance for all "outdoor areas where people spend widely varying amounts of time and other places in which quiet is a basis for use." Given this information, we recommend a design goal of a DNL of 55 dB for most outdoor use areas. One exception is the base/stage of the amphitheater which might benefit from an even stricter goal, since people would need to hear voices and music at greater distances than in picnic areas. In that area a better goal might be a DNL of 50 dB.



3. Site survey

The purposes of the site survey are as follows:

- 1. to measure <u>noise levels</u> on the site. Noise level data are useful for the following reasons:
 - a. to determine how the hourly average sound levels compare to the Day-Night Average Sound Levels (DNL). The DNL is the noise metric used by many organizations. However, the Traffic Noise Model (TNM) used the hourly average sound level. For locations mostly impacted by traffic noise, the relationship between the DNL and loudest hour average sound level is relatively constant. The measured sound levels are useful for determining this relationship.
 - b. to identify any significant non-traffic noise sources.
- 2. to observe <u>traffic conditions</u> such as prevailing speeds, classifications (i.e., percentages of automobiles, trucks, buses, and motorcycles), and directional distributions. Many of these parameters are not well documented in traffic studies. The prevailing speed often differs from the posted speed limit.
- 3. to observe <u>road conditions</u> such as locations and timing of traffic flow control devices (e.g., traffic signals, stop signs, and toll booths), and the pavement type.
- 4. to observe <u>site conditions</u> not represented on the site plan such as the presence and height of existing noise barriers along the road right-of-way.

3.1 Sound level measurement procedure

Three Larson Davis model 831 and LxT sound level meters were installed in the locations indicated in Figure 2 from 12 p.m. on Thursday February 3, 2011, through 10 a.m. on Friday February 10, 2011. The sound level meters were programmed to report average, maximum, and minimum A-weighted sound levels during each one-minute interval. For an explanation of A-weighted sound levels see the appendix. The meters were chained to trees and the microphones were attached to branches 6 to 7 feet above the ground.

3.2 Site observations

The site currently has open fields with small forested areas. The main noise source on the site is traffic on Route 355 and I-270. There is also some aircraft noise and activity on industrial/commercial parcels between Route 355 and I-270.

There are no stop signs or traffic signals on Route 355 or I-270 near the site. The pavement of both roads is asphaltic concrete. There are no median barriers or noise barriers along either road. Route 355 has one lane each direction with partial (roughly 1- to 3-foot wide) paved shoulders. I-270 has two lanes each direction with full shoulders.

The posted speed limit is 50 mph on Route 355 at the site although it drops to 30 mph at Route 121 to the south of the site and at Route 109 to the north of the site. The posted speed limit is 65 mph on I-270 at the site although it drops to 55 mph approximately 1/2 mile south of the site.



3.3 Measured sound levels

Average sound levels during five-minute intervals were calculated based on the measured one-minute average sound levels. Figure 3 presents the resulting five-minute average sound levels. Hourly average sound levels were calculated based on the five-minute average sound levels. Figure 4 presents the hourly average sound levels. The Day-Night Average Sound Levels (DNL) were calculated for each full calendar day. For an explanation of DNL see the appendix. Table 1 presents the DNL and loudest-hour average sound level, and the difference between the two, for each calendar day.

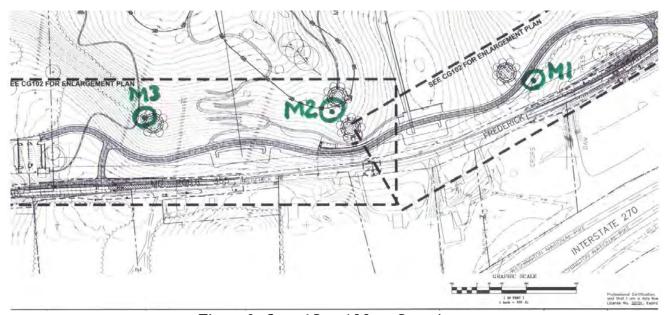


Figure 2. Sound Level Meter Locations

Table 1. Measured DNL and Loudest-Hour Average Sound Levels, dB

Day, Date		DNL		Loudest-Hour Average Sound Level				Minus Loudest- ur Average			
Thu., Feb. 03, 2011	-	-	-	59.4	54.7	50.7	-	-	-		
Fri., Feb. 04, 2011	60.8	58.2	55.9	60.8	58.8	55.4	-0.1	-0.6	0.5		
Sat., Feb. 05, 2011	57.5	55.5	54.5	56	55.8	54.6	1.5	-0.3	-0.1		
Sun., Feb. 06, 2011	57.6	55.7	53.9	56.1	53.6	52.4	1.4	2.1	1.5		
Mon., Feb. 07, 2011	60.9	60.1	58.8	60.2	58.7	57.1	0.7	1.4	1.7		
Tue., Feb. 08, 2011	62.6	59.2	55.5	61.9	59.0	55.8	0.7	0.1	-0.3		
Wed., Feb. 09, 2011	60.0	55.7	51.8	58.7	56.4	53.6	1.3	-0.7	-1.8		
Thu., Feb. 10, 2011	·			60.1	55.5	50.2	-	-	-		



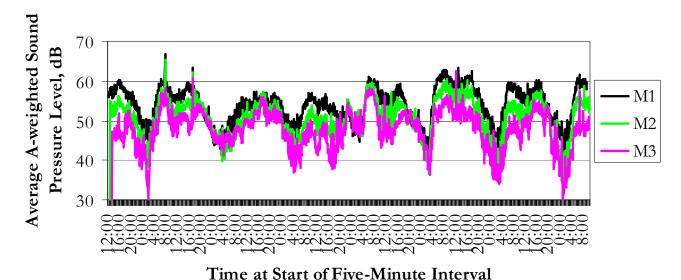


Figure 3. Five-Minute Average Sound Levels

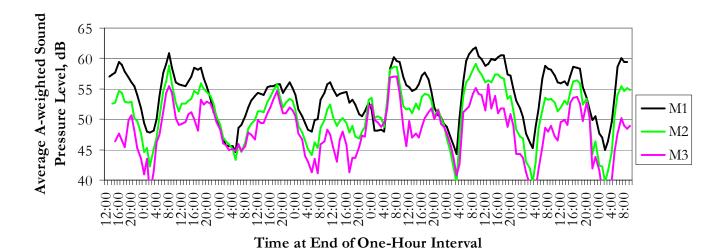


Figure 4. Hourly Average Sound Levels

3.4 Traffic counts

Traffic volumes were counted during one 15-minute interval for each direction of traffic on Route 355 and on one 5-minute interval for each direction of traffic on I-270 at the end of the survey. From these volumes the hourly average traffic volumes were extrapolated. Table 2 presents the extrapolated hourly traffic volumes. Automobiles include pickup trucks, passenger cars hauling trailers, and vans. Medium trucks are six-wheeled cargo vehicles with two axles. Heavy trucks are cargo vehicles with three or more axles. Speeds were determined using a hand-held radar gun. The median speeds for dozens of vehicles are listed in Table 2.



Table 2.	Extrapolated Hourly	Traffic Volumes	and Prevailing Speeds,	Thursday Feb. 10, 2011
	1		9 1 /	<i>y</i>

Time	Lanes	Speed	Autos	Medium	Heavy	Buses	Motor-
				Trucks	Trucks		cycles
841-856 am	Rt. 355 NB	47.5	52	4	4	0	0
	Rt. 355 SB	49.0	420	16	4	4	0
859-904 am	I-270 SB	-	3,996	0	60	24	0
905-910 am	I-270 NB	-	1,464	12	48	0	0

4. Outdoor noise modeling

4.1 TNM overview

In the United States, highway noise levels are typically analyzed using the Federal Highway Administration's (FHWA) Traffic Noise Model (TNM). The current version is 2.5. The output from TNM is the hourly average sound level at the receivers. The program allows input of the following information:

- Coordinates of selected points along the road centerlines
- Pavement width and type
- Road locations which are elevated (structure roadways)
- Hourly volumes and speeds of autos, medium trucks, heavy trucks, buses, and motorcycles for each road segment
- Locations of traffic flow control devices such as stop signs, traffic signals, and toll booths at the start of roads
- Coordinates and heights of evaluation points (receivers)
- Coordinates of ground elevations in selected locations (terrain lines)
- The default ground type, and coordinates and ground material in selected locations (ground zones)
- Coordinates and height of areas covered with thick evergreen forest (tree zones)
- Coordinates of existing and proposed objects that shield the site such as noise walls and buildings (barriers)
- Coordinates, height and spacing between buildings of rows of buildings which partially shield the site (building rows)

4.2 TNM validation

The traffic volumes and speeds presented in Table 2 were input into TNM. This TNM run is called the validation run. Each direction of travel of each road was modeled as an individual road in TNM. The locations and elevations of selected points along Route 355 and I-270, and the widths of each road, were taken from the site plan. Since the observed existing pavement is asphaltic concrete, the pavement was modeled as Dense-Graded Asphaltic Concrete (DGAC). This is the louder, and more common, of the two types of asphaltic concrete available in TNM. Two terrain lines were added on the site east of



Route 355, and two terrain lines were added between Route 355 and I-270 to model the changes in elevation. The default ground type was lawn.

The output sound levels were then compared to the sound levels measured during the traffic counts. Table 3 presents this comparison.

It can be seen from Table 3 that TNM produced sound levels between 2.8 dB lower and 1.7 dB greater than were measured. This level of agreement between the modeled and measured sound levels is reasonable.

Table 3. Comparison of TNM Validation Run Output and Measured Sound Levels, dB

	M1	M2	M3
Measured 8:41-8:56 am	59.4	54.8	48.0
TNM output	58.7	52.0	49.7
TNM Minus Measured	-0.7	-2.8	1.7

4.3 Future traffic conditions

The Maryland State Highway Administration (MDSHA) provided Average Daily Traffic (ADT) volumes for 2030 of 115,750 for I-270 and 11,200 for Route 355. They also provided peak-hour volumes of 6% for I-270 (morning and afternoon), and 11% morning and 14% afternoon for Route 355. They had no class counts for Route 355, but they were able to provide class counts for one full day on I-270; from those counts we calculated an average of 4.1% medium trucks, 5.0% heavy trucks, 0.7% buses, and 0.4% motorcycles. Based on our brief traffic counts for Route 355, it was assumed that traffic would include 4.0% medium trucks, 1.6% heavy trucks, 0.8% buses, and 0.0% motorcycles.

For 3 of the 4 full weekdays of sound level data presented above, the loudest hour occurred during the morning rush-hour period. This is surprising since (1) there is clearly more traffic southbound in the morning and the southbound lanes are all farther from the site, and (2) based on the MDSHA data there should be more traffic on Route 355 in the afternoon than in the morning. In modeling noise levels, the sound levels output from the model would be higher with more traffic heading northbound than southbound, since the northbound lanes are closer to the site and the edge of the road partially shields southbound traffic. For our analysis of future noise levels we evaluated the afternoon rush-hour period. Based on an extrapolation from our traffic counts it was assumed that the future directional factor in the afternoon will be 88% northbound on Route 355 and 73% northbound on I-270. For simplicity it was assumed that the peak traffic hour will generate the highest noise levels (i.e., the loudest-hour).

The resulting forecast traffic volumes are presented in Table 4. It can be seen from Tables 2 and 4 that the forecast total traffic volumes are much higher than those observed during the site visit on Route 355 and somewhat higher on I-270. To be conservative the speed was assumed to be 50 mph on Route 355 which is a bit greater than the speeds we noted while on site, but is equal to the posted speed limit. For I-270 the speed was assumed to be equal to the posted speed limit of 65 mph.



Lanes	Autos	Medium	Heavy	Buses	Motor-	Prevailing
		Trucks	Trucks		cycles	Speed (mph)
Rt. 355 NB	1,292	55	22	11	0	50
Rt. 355 SB	176	8	3	2	0	50
I-270 SB	1,684	78	93	13	7	65
I-270 NB	4,554	210	253	35	18	65

Table 4. Year 2030 Loudest-Hour Traffic Volumes

4.4 Future highway noise modeling

TNM was run using the traffic volumes and speeds presented in Table 4. Unless noted in this paragraph all modeling parameters were the same as for the validation run. The site plan shows widening Route 355 at the two entrances to the park. These road changes were incorporated into the analysis. The proposed interior road was included in the analysis solely as an indication of ground elevation, as well as to account for sound reflections off the pavement. Receivers were located in selected outdoor activity areas, at the proposed building, as well as in various other locations on the site. Locations and ground elevations of receivers were taken from the site plan. The receiver heights were 5 feet above the ground elevations in all cases other than 7 feet above the ground elevation at the building.

4.5 Future outdoor highway noise levels

It can be seen from Table 2 that the DNL was between 1.8 dB less than and 1.7 dB greater than the loudest-hour average sound level. The future loudest-hour average sound levels were output from TNM. To be conservative, and to account for the inaccuracy of the model discussed in section 4.2, it was assumed that in the year 2030 the DNL would be approximately 2 dB greater than the loudest-hour average sound level. The resulting year 2030 DNL are presented in Figure 5.

5. Outdoor highway noise mitigation

There are ten areas of interest shown on the latest site plan. The year 2030 DNL can be summarized as follows (referencing the key numbers 1-10 on the site plan):

Areas where we are proposing a goal of a DNL of 55 dB:

		- PP
1.	64-71 dB	Park house (building to be demolished) and picnic area
2.	<45 dB	Bottom of sledding hill
3.	51-57 dB	Amphitheater (trail traverses across slope with places for people to sit – no bldg)
4.	48-49 dB	Playscape
5.	56 dB	Picnic shelter (shielded by retaining wall)
6.	56 dB	Sycamore ring at the retaining wall
8.	55 dB	Visitor facility roof (on top of building)
10.	<45 dB	Outdoor classroom (overlook deck)

Areas where we are proposing a goal of a DNL of 50 dB:

9. 50-52 dB Base/stage of the amphitheater



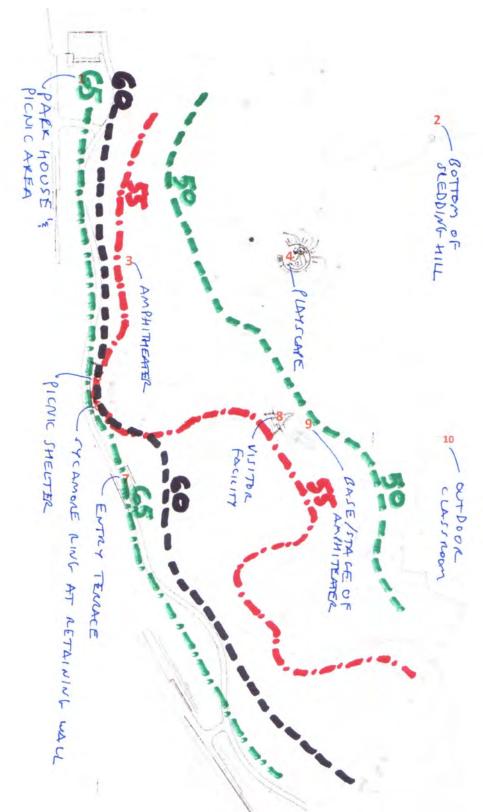


Figure 5. Year 2030 DNL Contours



Other areas: 7. 64-65 dB

Entry terrace

Note that this analysis is very sensitive to the assumed future ground elevations. Lower elevations will produce lower sound levels.

At the base/stage of the amphitheater the predicted DNL may slightly exceed (i.e., by 1-2 dB) the goal of 50 dB in some areas. The DNL in this location could be reduced simply by reducing the elevation of the amphitheater to benefit from natural shielding of the surrounding terrain, or by moving it slightly farther to the east to accomplish the same lower elevation.

The DNL will meet the goal of 55 dB at the bottom of the sledding hill, the playscape, the visitor facility roof (on top of the building), and the outdoor classroom (overlook deck). The DNL may slightly exceed (i.e., by 1 to 2 dB) the goal of 55 dB at the picnic shelter, sycamore ring at the retaining wall, and in portions of the amphitheater (at the sloping trail).

The DNL will significantly exceed the goal of 55 dB at the park house and picnic area. Reducing sound levels in this area would require constructing a noise barrier such as a noise wall or earthen berm. The goal is extremely hard to meet here due to the proximity to the road. It would take a 25-foot tall barrier to approach the goal. With a 20-foot tall barrier the DNL would be approximately 56-57 dB. With a 15-foot tall barrier the DNL would be approximately 57-58 dB. With a 10-foot tall barrier the DNL would be approximately 58-60 dB.

6. Indoor highway noise levels

The visitor facility will have several spaces including a multipurpose room. A reasonable goal for a multipurpose room which is sometimes used as a classroom is an hourly average sound level no higher than 35 dB. This is the criterion used by American National Standard ANSI S12.60 for classrooms. We predict an outdoor hourly average sound level of 53 dB at the roof of the building, 49 dB at the side facades, and approximately 40-45 dB within the open corridor of the building. These sound levels correspond to DNL of approximately 55 dB on the roof, 51 dB at the side facades, and 42-47 dB in the open corridor.

Since the side facades are the weakest link acoustically, this means that the building envelope must reduce traffic noise levels by 14 dB (i.e., 49-35=14 dB). This reduction is called the NLR.

We performed calculations for the proposed multipurpose room. With standard windows and doors and a non-carpeted floor the predicted NLR is approximately 20 dB. This easily meets the goal of 14 dB. Therefore, no upgrades are required beyond standard construction to reduce traffic noise indoors.

The following appendices provide additional information about acoustical terminology and criteria, and the precision of this analysis.



If you have any questions, please contact me at 703/534-2790 or via e-mail at Gary@HushAcoustics.com.

Sincerely,

Gary Ehrlich, P.E.

Fany Ehrlis.

Principal



Appendix A – Noise Metrics

There are many different ways to express sound levels, but all ways must have some means of incorporating the three most important aspects of the sound: loudness (level), pitch (frequency), and duration (time pattern). The chosen way to express the sound level is known as the noise metric.

Level. The sound level is almost always expressed in decibels, abbreviated dB. The decibel is a unitless quantity; it is technically based a ratio between the sound pressure and a standard reference pressure. Sound level meters can show the sound level varying with a moving needle or changing electronic display. How quickly this display changes, and therefore how quickly the meter responds to changes in sound level, is called the time weighting network or simply the meter "response." The four most commonly used responses are peak, impulsive, fast, and slow; peak response is the fastest response while slow is the slowest. The peak response is only normally used to evaluate the potential for hearing damage and damage to structures, and is never used to express the annoyance of noise. The impulsive response is only typically used to evaluate loud periodic noises such as pile driving and gun fire. The fast and slow responses are the most commonly used. Fast response is used when the sound level changes relatively rapidly over time as would be the case at a night club or a construction site. Slow response is used when the sound level is relatively steady as would be the case for environmental noise such as near highways, railroads, and airports.

Following are how high A-weighted sound levels are for some familiar sounds (taken from U.S. Environmental Protection Agency documents):

Noises:

Chain saw operator	103-115 dBA
Heavy truck at 50 feet	85-95 dBA
Motorcycle driver	80-115 dBA
Power lawn mower operator	80-95 dBA
Subway rider	80-90 dBA
Train passenger	72-90 dBA
City bus at 50 feet	70-85 dBA
Waste food disposer	67-93 dBA
Automobile at 50 feet	64-88 dBA
Vacuum cleaner	60-85 dBA
Washing machine	47-73 dBA
Refrigerator	45-68 dBA
 	- 4

Average conversational speech at 1 meter:

Inside suburban house	55 dBA
Outdoors in suburban area	55 dBA
Inside urban house	57 dBA
Outdoors in urban area	65 dBA
On a train	66 dBA
On an aircraft	68 dBA

Frequency. The frequency of sound is always expressed in Hertz, abbreviated Hz. The audible frequency range (20 Hz to approximately 15,000 or 20,000 Hz) is typically divided into bands covering one octave,



or one-third of an octave. Each doubling of frequency is defined as one octave. A sound level can then be stated either as a single-value covering the entire audible frequency range, or for a given octave or one-third octave band. When sound levels are stated for the entire audible frequency range, the sound could be filtered to roughly simulate the hearing sensitivity of the average person. There are two commonly-used filter types: A- and C-weighting. An A-weighted sound level is by far the most-commonly used, and was designed to approximately represent the hearing sensitivity of a person exposed to sounds of moderate loudness. A C-weighted sound level is occasionally used to assess noise from blasting and other loud short-duration sounds and was developed to approximately represent the hearing sensitivity of a person exposed to loud sounds. For environmental noise studies, or for most other purposes as well, it is assumed that the sound level is A-weighted if there is no specific designation otherwise.

Time Pattern. The variation of a sound level over time is perhaps the most complex of the three parameters, and there are a myriad of ways to express this variation. The various ways can be divided into single-event sound levels and long-term sound levels. Examples of "single events" are a train passby, an aircraft overflight, or a gun firing. Single-event sound levels can be based on the maximum sound level reached during the event (abbreviated L_{max}), the total sound energy produced during the event (known as the sound exposure level, or SEL), or the number of times the sound level exceeds a threshold value (known as the number of events above, or NA). Long-term sound levels must be based on sound levels over a given time interval. Common time intervals are one hour and 24 hours. During this time interval the stated quantity could be the average sound level (known as the equivalentcontinuous sound level, or L_{eo}), the amount of time the sound level exceeds a threshold value (known as time above, or TA), or the sound level exceeded any set percentage of the time (known as the statistical sound level; e.g., the sound level exceeded ten percent of the time is written L₁₀, while the sound level exceeded 90 percent of the time is written the L_{90}). One-hour average sound levels, or occasionally onehour statistical sound levels, are used by the Federal Highway Administration and state departments of transportation to express highway noise levels. The sound level exceeded 90 percent of the time, L₉₀, is often considered the background sound level, since it is not significantly affected by loud periodic noise events. 24-hour average sound levels, and occasionally 24-hour statistical sound levels, are typically used to express all forms of transportation noise including highway, aircraft, and railroad noise. The 24-hour average noise level can include some adjustments to account for peoples' increased sensitivity to noise in the evening and at night. The two most common ways to account for this sensitivity is with the Day-Night Average Sound Level (DNL) and the Community Noise Equivalent Level (CNEL). The DNL is just a 24-hour average sound level for a calendar day with 10 dB added to all noise which occurs between 12 a.m. and 7 a.m. and between 10 p.m. and midnight. The CNEL is the same as DNL but with 5 dB added to all noise which occurs between 7 p.m. and 10 p.m.

Appendix B – Noise Criteria

Noise is unwanted since it causes: (1) hearing damage, (2) annoyance, (3) speech interference, and (4) sleep disturbance. There are various types of noise criteria that revolve around different unwanted causes. The Occupational Safety and Health Act (OSHA) established maximum allowable sound levels in the workplace in an effort to prevent hearing damage. The OSHA limits often become significant in industrial and military settings, as well as for construction workers. In most work and home environments the sound levels are well below the OSHA limits. Most noise criteria relate to the other



three unwanted effects of noise. There are noise criteria at the federal, state, and local levels, and there are also non-regulatory criteria developed by many private and governmental organizations.

Federal Noise Criteria. There are many government agencies that have established noise criteria. The U.S. Environmental Protection Agency (EPA) developed many of the criteria used by other federal agencies. The U.S. Department of Housing and Urban Development (HUD) established an outdoor noise standard that residential use assisted or supported by HUD is "acceptable" where the DNL does not exceed 65 dB, "normally unacceptable" where the DNL is over 65 dB but does not exceed 75 dB, and "unacceptable" where the DNL exceeds 75 dB. The HUD indoor noise goal is that the DNL not exceed 45 dB inside proposed residences. These limits are typically only evaluated by HUD when the project receives funding from the Federal Housing Administration (FHA). The Federal Aviation Administration (FAA) has established a threshold of a DNL of 65 dB, above which residential development is not compatible; the FAA indoor threshold is a DNL of 45 dB. These limits are typically only evaluated when environmental noise studies (such as environmental assessments or environmental impact statements) are performed in support of a major project, or when existing residences, schools, or churches are sound insulated in FAA-sponsored programs. The Department of the Navy uses similar criteria which are typically only evaluated when environmental noise studies (such as Air Installation Compatible Use Zone, or AICUZ, studies) are completed in support of a major realignment of assets. The Federal Highway Administration (FHWA) established noise abatement criteria (NAC) for various land uses; the NAC for residential use is an hourly average sound level of 67 dB outdoors and 52 dB indoors. When the sound level approaches or exceeds the NAC a noise impact occurs; the state departments of transportation may define the word "approach" although it is typically considered to be when the sound level reaches within one dB of the NAC.

State Noise Criteria. Many states have established different noise criteria for four purposes: (1) to control noise produced by citizens, (2) to evaluate the compatibility of a proposed land use with respect to environmental noise, (3) to determine if construction of a state-funded noise barrier is warranted along a highway, and (4) to verify that new construction provides adequate acoustical separation between dwelling units of multi-family housing. The first purpose is incorporated into a noise ordinance and is enforceable against the person generating the noise. The Code of Maryland includes such as noise ordinance, while in the state of Virginia the noise ordinances are developed at the local level. Noise ordinances typically limit the maximum A-weighted noise level, and many also limit the maximum noise level in each octave band. The second purpose is incorporated into the environmental noise policy and is enforceable by the state and local (if adopted at the local level) planning and zoning departments. The Code of Maryland also includes such an environmental noise policy, while in most other states such as Virginia it is solely up to the municipalities to develop such a policy. The state of California has a building code requirement that where the outdoor DNL or CNEL exceeds 60 dB, an acoustical analysis shall be performed demonstrating that the indoor DNL or CNEL not exceed 45 dB. Environmental noise policies are almost always expressed in terms of the DNL, with the exception of the state of California which also uses CNEL. The third purpose is incorporated in the noise barrier policy and is used by the state department of transportation. Maryland and Virginia, as well as other states, have such a noise barrier policy. The noise barrier policies are almost always expressed in terms of the hourly average sound level referencing the noise abatement criteria used by the FHWA, although some are expressed in terms of the sound level exceeded during 10 percent of the hour (the L_{10}). The fourth



purpose is incorporated into the state and local building code in the form of a minimum acceptable Sound Transmission Class (STC) or Impact Insulation Class (IIC) rating.

Local Noise Criteria. Many municipalities have established both a noise ordinance and an environmental noise policy. The environmental noise policy is sometimes summarized in a policy plan, comprehensive plan, or similar document, while in other jurisdictions it is not documented at all, outside of in-house planning department memos. The environmental noise policy is sometimes enforceable by ordinance in the case of an overlay zone. Overlay zones are often adopted around airports or military air bases, as is the case for High Point, North Carolina. In some municipalities the state department of transportation noise barrier policy is used to assist determining if a developer applying for a re-zoning must build a highway noise barrier.

Private Noise Criteria. In many cases, there are no applicable regulatory criteria. For example, there rarely is any regulatory limit on noise levels due to plumbing systems, noise levels in classrooms, or noise levels transmitted from one office to another. In these cases it is useful to consider non-binding criteria developed by private and governmental organizations. The American Society of Heating Refrigerating and Air-conditioning Engineers (ASHRAE) provides recommendations regarding noise from mechanical systems. The ASHRAE recommendations are typically expressed in terms of the Room Criterion (RC) rating, and used to be expressed in terms of the Noise Criterion (NC) rating. The American National Standards Institute (ANSI) developed a standard regarding noise levels in schools, and this standard has been adopted into law in some jurisdictions. The World Health Organization (WHO) has developed many noise standards for various purposes. In some cases it is useful to assess what percentage of syllables, words, or sentences would be intelligible in a given noise environment; two noise metrics used for this purpose are called speech transmission index (STI) and articulation index (AI). Various textbooks provide guidance on appropriate STI and AI values. There has also been some research into the percentage of people that would be "highly annoyed" or awakened by given noise levels. This research could be cited in the development on a noise criterion.

Appendix C – Precision of Predictions

It is not generally feasible to calculate the precision of a noise level or noise level reduction predictions. Unlike fields such as structural engineering, it is not typical practice to incorporate a specific margin of error in acoustical studies. Where possible, somewhat conservative assumptions were used in the outdoor noise level analysis. However, STC ratings quoted by manufacturers of products such as windows and doors are inherently anti-conservative, since the manufacturer has the option to test products many times and only publish the best rating the product ever achieved. Also, there are a variety of field installation issues which could make the STC ratings of walls be lower than anticipated. These two factors (slightly conservative assumptions used to predict outdoor noise levels, and possibly anti-conservative data used to predict indoor noise levels) may roughly balance each other out. The end result is that our predictions should roughly match future measured sound levels on average, with a statistical variation above and below.

If a general margin of error were desired, it would be advisable to exceed the recommended acoustical performance (often expressed by the STC rating) of walls, windows, and doors by a couple of points. For highway noise analyses, a margin of error could be also incorporated by extending any



recommended highway noise barriers farther (i.e., shielding a greater angle of view) and a couple of feet higher. If you would like to incorporate a specific margin of error, please let us know and we could revise our analysis.

Hush Acoustics LLC does not provide any warranty or guarantee as to the precision of the noise level or noise level reduction predictions or measurements.

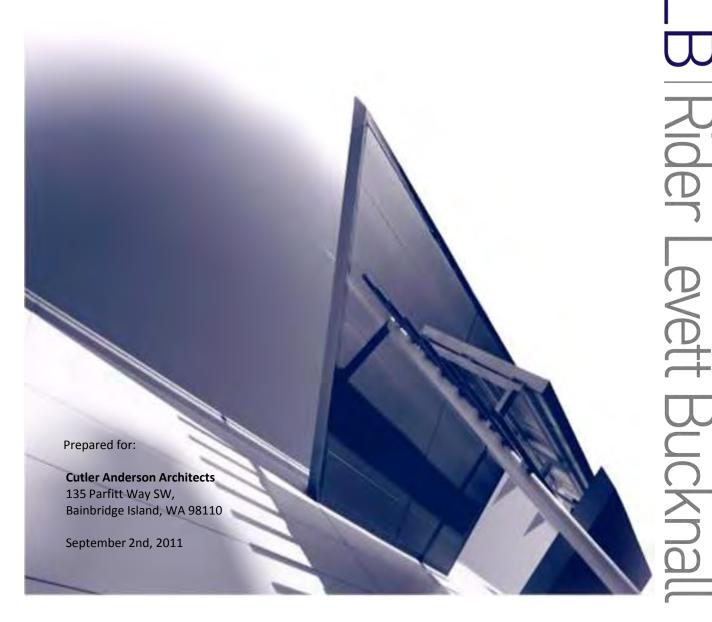
Detailed Cost Estimate

REPORT

30% Construction Documents Cost Estimate

Little Bennett Regional Park - Day Use Area The Underlook and Amphitheater

Clarksburg, MD



Little Bennett Regional Park - 30% Construction Documents Estimate

Project Summary

BASIS OF ESTIMATE

This estimate has been prepared at the request of Cutler Anderson Architects and is to provide a 30% Construction Documents Cost Estimate for the proposed Construction of a Day Use Area at the Little Bennett Regional Park, located at Frederick Road, Clarksburg, MD.

The estimate is based upon measured quantities and built-up rates prepared from the Design Development Drawings and Outline Specifications, dated July 6th 2011.

Where information was insufficient, assumptions and allowances were made, based wherever possible on discussions with the architect and engineers.

Pricing is based on September 2011 costs. An allowance of 7.5% for Design/Estimating Contingency has been included. No escalation has been included in this estimate.

It is assumed that the project will be competitively bid and that the contractor will be required to pay prevailing wage rates.

ITEMS SPECIFICALLY EXCLUDED

- Excavation in rock and/or contaminated fill
- Hazardous materials abatement
- Work outside building footprint unless noted otherwise
- Site pathways, and emergency/maintenance access roads
- Parking lots and picnic areas
- Landscaping unless noted otherwise
- Site utilities up to within 5'0" from building footprint
- Furniture, Fittings and Equipment (FF&E)
- Raised access flooring
- Murals and works of art
- Mockups
- Staging / Phasing costs
- Construction Management Fees
- Owner's Contingency
- Land and legal costs
- Architectural, Engineering and other professional fees
- Items marked as "Excl." in the estimate

Little Bennett Regional Park - 30% Construction Documents Estimate

Project Summary

QUALIFICATIONS AND ASSUMPTIONS

Site Construction

- Due to the lack of geotechnical information, we have assumed that the max depth cut of 6'-0" will be suitable to be over excavated and battered, in lieu of a shoring/earth retention system.
- Site aerial photographs indicate that it is a "greenfield" site and that no major site clearing wil be required. We have not allowed for site clearing beyond topsoil strip, and associated earthworks.
- Site filling and grading has been allowed for. No allowance for grass or landscaping these areas has been included.
- All landscaping to site and green roof has been excluded unless noted otherwise
- Imported planting mix to the green roof system has been included
- Pedestrian paving to Breezeway entrances has been included to the extent inferred by the floor plans, approx. 20'-0" from building footprint (total area allowed 470 sq. ft.)
- Site walls assumed as colored concrete
- No backup waterproofing or drainage mat has been allowed to site walls
- External stairs assumed as colored concrete on grade
- Site lighting has been included only to the stepped pedestrian passageway

Structural

- Where detailed sections have not been provided, continuous strip footings have been allowed at 1'-4" x 0'-8" deep (as per Detail A)
- Isolated spread footings have been allowed at column locations (assumed 1'-4" sq. x 0'-8" deep)
- Suspended concrete slab to green roof system has been allowed at 8" thick
- All suspended slabs priced using traditional formwork method (in lieu of metal deck construction)
- Reinforcing bar to continuous strip footings has been allowed at 60 lbs / cy
- Reinforcing bar to isolated spread footings has been allowed at 100 lbs / cy
- Reinforcing bar to foundation, site, and exterior concrete walls has been allowed at 150 lbs / cy
- Structural steel columns have been assumed as HSS 6 x 6 x 1/2"

Building Enclosure

- Exterior cladding assumed as clapboard wood siding type system

Finishes

- Wood ceiling finish, with clear sealer, has been allowed to the Breezeway
- No ceiling finish has been allowed to the Storage/Fountain areas or Mech/Janitor (unfinished concrete)
- Wood base has been allowed to the Office and Multi-Function Room where walls lined with GWB.
- Resilient base has been allowed to all other walls where lined with GWB

Fittings and Miscellaneous

- Overhead coiling roller door has been allowed to the Storage/Fountain room
- Glass paved skylight allowed as "mid-range" quality/finish
- We have allowed for 2'-0" adjustable plastic laminate shelving to the Storage Room
- We have allowed for 4'-0" mirrors at full length to each of the Lavatory countertops



Little Bennett Regional Park - Day Use Area

30% Construction Documents Cost Estimate - Revision 1 Location Summary

Rates Current At August 2011 GFAR: Gross Floor Area

Code	Description		GFAR SF	Cost/SF	Total Cost
Α	Amphitheater				\$50,400.00
В	Building Works - The U	nderlook	2,126	\$316.64	\$673,186.00
		Estimated Net Cost	2,126	\$340.35	\$723,586.00
Margins	& Adjustments				
General C	Conditions	8.0%			\$57,887.00
Bonds an	d Insurance	1.8%			\$13,676.00
Overhead	d and Profit	4.0%			\$31,806.00
Design/Es	stimating Contingency	6.6%			\$54,269.00
Escalatio	n				Excl.

Estimated Total Cost 2,126 \$414.50 \$881,224.00

Divisions/Elements Summary

Rates Current At August 2011 Gross Floor Area: 2,126.00 SF

				Alea. 2, 120.00 01
		Percentage	Cost/SF	Total Cost
02	Site Construction			
A1010	Standard Foundations	0.2%	\$0.54	\$1,142.00
A1030	Slab on Grade	0.2%	\$0.76	\$1,618.00
C2010	Stair Construction	0.4%	\$1.48	\$3,150.00
G1010	Site Clearing			
G1030	Site Earthwork	2.4%	\$8.20	\$17,441.00
G2020	Parking Lots			
G2030	Pedestrian Paving	1.7%	\$5.81	\$12,357.00
G2040	Site Development	14.6%	\$49.66	\$105,581.00
G2050	Landscaping	0.1%	\$0.50	\$1,071.00
G3010	Water Supply			
G3020	Sanitary Water			
G3030	Storm Sewer			
G4010	Electrical Distribution			
G4020	Site Lighting	0.4%	\$1.48	\$3,150.00
G4030	Site Communications & Security			
	Site Construction Total	20.1%	\$68.44	\$145,510.00
03	Concrete			
A1010	Standard Foundations	2.9%	\$9.80	\$20,843.00
A1030	Slab on Grade	1.3%	\$4.37	\$9,291.00
B1010	Floor Construction	0.5%	\$1.58	\$3,353.00
B1020	Roof Construction	2.2%	\$7.38	\$15,698.00
B2010	Exterior Walls	7.3%	\$24.76	\$52,646.00
	Concrete Total	14.1%	\$47.90	\$101,831.00
05	Metals			
B1020	Roof Construction	5.7%	\$19.26	\$40,949.00
C1030	Fittings	0.7%	\$2.33	\$4,950.00
	Metals Total	6.3%	\$21.59	\$45,899.00
06	Wood, Plastics, and Composites			
B1010	Floor Construction	0.9%	\$3.12	\$6,627.00
B1020	Roof Construction	1.4%	\$4.60	\$9,776.00
B2010	Exterior Walls	1.9%	\$6.35	\$13,494.00
C1010	Partitions	0.3%	\$1.18	\$2,504.00
C1030	Fittings	1.5%	\$5.08	\$10,801.00
	Wood, Plastics, and	6.0%	\$20.32	\$43,202.00
	Composites Total			
07	Thermal and Moisture Protection			
A1030	Slab on Grade	0.6%	\$1.94	\$4,129.00
B1010	Floor Construction	0.2%	\$0.76	\$1,620.00
B1020	Roof Construction	3.5%	\$11.89	\$25,271.00
B2010	Exterior Walls	2.3%	\$7.87	\$16,728.00

BOS11033-201

Divisions/Elements Summary

Rates Current At August 2011 Gross Floor Area: 2,126.00 SF

B3010				
B3010		Percentage	Cost/SF	Total Cost
	Roof Coverings	4.0%	\$13.63	\$28,969.00
	Thermal and Moisture Protection Total	10.6%	\$36.09	\$76,717.00
08	Openings			
B2010	Exterior Walls	13.4%	\$45.68	\$97,119.00
B2030	Exterior Doors	2.5%	\$8.68	\$18,450.00
B3020	Roof Openings	1.2%	\$3.98	\$8,460.00
C1029	Interior Doors	0.4%	\$1.52	\$3,240.00
C1030	Fittings	7.0%	\$23.93	\$50,873.00
	Openings Total	24.6%	\$83.79	\$178,142.00
09	Finishings			
C3010	Wall Finishes	0.5%	\$1.83	\$3,893.00
C3020	Floor Finishes	1.7%	\$5.83	\$12,393.00
C3030	Ceiling Finishes	4.3%	\$14.72	\$31,305.00
	Finishings Total	6.6%	\$22.39	\$47,591.00
10	Specialities			
C1030	Fittings	1.2%	\$4.00	\$8,505.00
	Specialities Total	1.2%	\$4.00	\$8,505.00
11	Equipment			
E2020	Movable Furnishings			
	Equipment Total			
15	Mechanical			
D2010	Plumbing Fixtures	2.7%	\$9.31	\$19,800.00
D3040	Distribution Systems	2.1%	\$7.20	\$15,307.00
	Mechanical Total	4.9%	\$16.51	\$35,107.00
	Electrical			
16	EL 1: 10 : 0 D: 1:1 ::	4.00/		
16 D5010	Electrical Service & Distribution	1.3%	\$4.50	\$9,567.00
	Lighting and Branch Wiring	1.3% 3.2%	\$4.50 \$10.80	\$9,567.00 \$22,961.00
D5010				
D5010 D5020	Lighting and Branch Wiring	3.2%	\$10.80	\$22,961.00

Divisions/Elements/Item

Rates Current At August 2011 Gross Floor Area: 2,126.00 SF

		Unit	Qty	Rate	Total Cost
02	Site Construction				
A1010	Standard Foundations				
5	Excavate for foundations	CY	21	\$11.00	\$227.00
25	Excavate for foundation walls	CY	9	\$11.00	\$97.00
27	Excavate for external walls partially below grade	CY	12	\$11.00	\$130.00
6	Backfill to foundations with excavated material	CY	7	\$11.00	\$76.00
7	Remove and dispose excavated spoil from site	CY	34	\$18.00	\$612.00
	Standard Foundations Total			\$0.54/SF	\$1,142.00
A1030	Slab on Grade				
16	Excavate for slab on grade	CY	39	\$11.00	\$421.00
17	Imported washed gravel as sub-base for slab on grade	CY	22	\$23.00	\$495.00
18	Remove and dispose excavated spoil from site	CY	39	\$18.00	\$702.00
	Slab on Grade Total			\$0.76/SF	\$1,618.00
C2010	Stair Construction				
111	4'-0" Wide reinforced colored concrete stair on grade, complete	FT/R	7	\$450.00	\$3,150.00
	Stair Construction Total			\$1.48/SF	\$3,150.00
G1010	Site Clearing				
143	General site clearing - Nil (none required, presumed greenfield site)	Item			Excl.
	Site Clearing Total				
G1030	Site Earthwork				
142	Strip topsoil and stockpile on site	CY	26	\$5.00	\$117.00
1	Bulk excavation to reduced levels	CY	397	\$9.00	\$3,573.00
2	Imported granular fill to rear of walls below grade	CY	51	\$22.00	\$1,147.00
149	Site grading to East of green roof system using excavated material	CY	297	\$11.00	\$3,208.00
3	Backfill to bulk excavation with excavated material	CY	45	\$11.00	\$486.00
4	Remove and dispose excavated spoil from site	CY	55	\$18.00	\$990.00
141	Shoring - Excluded; max cut 6'-0" (allowance made to over excavate and batter)	Item			Excl.
144	Final grading and trim to building platform	Item			\$900.00
29	4" Sub-soil foundation drain	LF	290	\$18.00	\$5,220.00
	Page Total				\$21,551

Divisions/Elements/Item

Rates Current At August 2011 Gross Floor Area: 2,126.00 SF

		Unit	Qty	Rate	Total Cost
)2	Site Construction (cont)				
145	Allowance for miscellaneous site grading around building footprint	Item			\$1,800.00
	Site Earthwork Total			\$8.20/SF	\$17,441.00
G2020	Parking Lots				
84	Parking lots and roadways - Excluded (not part of Scope of Works)	Item			Excl.
86	Emergency/maintenance access - Excluded (not part of Scope of Works)	Item			Excl.
	Parking Lots Total				
G2030	Pedestrian Paving				
83	Pedestrian paving, complete including 1 1/2" sand bedding on 6" imported structural fill over vapor barrier	SF	469	\$18.00	\$8,442.00
79	4'-0" Wide stepped passageway, complete including 4" decomposed granite on 6" structural fill, vapor barrier, and 6" x 18" concrete curb risers at 5'-0" O.C.	SF	290	\$14.00	\$3,915.00
	Pedestrian Paving Total			\$5.81/SF	\$12,357.00
G2040	Site Development				
74	8" Reinforced colored concrete site walls, complete	SF	1,309	\$32.00	\$41,234.00
75	Extra over for 1" x 6" board form finish	SF	1,964	\$3.00	\$5,303.00
76	1'-4" x 0'-8" Reinforced concrete continuous strip footing, complete	LF	204	\$27.00	\$5,508.00
77	Waterproofing to site walls	SF	328	\$3.00	\$886.00
115	2" Dia. Wood wall mounted handrail	LF	20	\$68.00	\$1,350.00
140	Allowance for amphitheater	SF	8,000	\$6.00	\$50,400.00
124	Allowance for external building signage	Item			\$900.00
87	Site walkways - Excluded (not part of Scope of Works)	Item			Excl.
85	Paved picnic area - Excluded (not part of Scope of Works)	Item			Excl.
	Site Development Total		;	\$49.66/SF	\$105,581.00
G2050	Landscaping				
61	Imported planting fill to green roof construction	CY	34	\$32.00	\$1,071.00
112	Landscaping to green roof system - Excluded (assume not part of Scope of Works)	SF	918		Excl.
	Page Total				\$119,009

Divisions/Elements/Item

Rates Current At August 2011 Gross Floor Area: 2,126.00 SF

		Unit	Qty	Rate	Total Cost
02	Site Construction (cont)				
88	Landscaping - Excluded (not part of Scope of Works)	Item			Excl.
	Landscaping Total			0.50/SF	\$1,071.00
G3010	Water Supply				
116	Site water supply utilities - Excluded (not part of Scope of Works)	Item			Excl.
	Water Supply Total				
G3020	Sanitary Water				
117	Site sanitary sewer utilities - Excluded (not part of Scope of Works)	Item			Excl.
	Sanitary Water Total				
G3030	Storm Sewer				
118	Site storm sewer utilities - Excluded (not part of Scope of Works)	Item			Excl.
	Storm Sewer Total				
G4010	Electrical Distribution				
119	Site electrical utilities - Excluded (not part of Scope of Works)	Item			Excl.
	Electrical Distribution Total				
G4020	Site Lighting				
121	Allowance for path lighting to stepped pedestrian passageway	Item			\$3,150.00
120	Miscellaneous site lighting - Excluded (not part of Scope of Works)	Item			Excl.
	Site Lighting Total		Ş	1.48/SF	\$3,150.00
G4030	Site Communications & Security				
122	Site communications and security services - Excluded (not part of Scope of Works)	Item			Excl.
	Site Communications & Securi	ty Total			
	Site Construction Total		\$6	88.44/SF	\$145,510.00
			•		, ,
	Page Total				\$3,150

Divisions/Elements/Item

Rates Current At August 2011 Gross Floor Area: 2,126.00 SF

		Unit	Qty	Rate	Total Cos
3	Concrete				
A1010	Standard Foundations				
8	1'-4" x 0'-8" Reinforced concrete continuous strip footing, complete	LF	361	\$23.00	\$8,123.0
9	1'-8" x 0'-8" Reinforced concrete continuous strip footing, complete	LF	110	\$27.00	\$2,970.0
10	1'-4" sq. x 0'-8" Deep reinforced concrete isolated spread footing, complete	EA	12	\$45.00	\$540.0
11	6" Reinforced concrete foundation wall, complete	SF	319	\$24.00	\$7,752.0
12	8" Reinforced concrete foundation wall, complete	SF	54	\$27.00	\$1,458.0
	Standard Foundations Total			\$9.80/SF	\$20,843.0
A1030	Slab on Grade				
13	5" Reinforced colored concrete slab on grade, complete including 6x6 W2.9xW2.9 WWF reinforcement	SF	1,147	\$8.00	\$9,291.0
	Slab on Grade Total			\$4.37/SF	\$9,291.0
B1010	Floor Construction				
14	5" Reinforced colored concrete suspended slab, complete	SF	207	\$9.00	\$1,863.0
15	Soffit formwork to suspended slab	SF	207	\$7.00	\$1,490.0
	Floor Construction Total			\$1.58/SF	\$3,353.0
B1020	Roof Construction				
55	8" Reinforced concrete suspended slab, complete	SF	918	\$9.00	\$8,262.0
56	Sloping soffit formwork to suspended slab	SF	918	\$8.00	\$7,436.0
	Roof Construction Total			\$7.38/SF	\$15,698.0
B2010	Exterior Walls				
21	8" Reinforced colored concrete wall, complete	SF	1,049	\$31.00	\$32,099.0
22	12" Reinforced colored concrete wall, complete	SF	114	\$33.00	\$3,796.0
23	12" "Thermomass" insulated precast colored concrete wall panel	SF	420	\$38.00	\$15,876.0
24	Extra over for 1" x 6" board form finish to visible faces	SF	324	\$3.00	\$875.0
	Exterior Walls Total		(24.76/SF	\$52,646.0
	Concrete Total		;	47.90/SF	\$101,831.0



Divisions/Elements/Item

Rates Current At August 2011 Gross Floor Area: 2,126.00 SF

		Unit	Qty	Rate	Total Cost
05	Metals				
B1020	Roof Construction				
43	Structural steel column framing	t	2.40	\$3,150.00	\$7,560.00
42	Structural steel beam roof framing	t	4.90	\$3,150.00	\$15,435.00
44	Structural steel fascia channel	t	2.26	\$3,150.00	\$7,119.00
45	Allowance for base plates and anchor bolts	EA	12	\$900.00	\$10,800.00
46	Allowance for miscellaneous steel, plates, and connections	t	0.01	\$3,500.00	\$35.00
	Roof Construction Total			\$19.26/SF	\$40,949.00
C1030	Fittings				
146	Allowance for stainless steel handrail to select operable glazed doors	LF	44	\$113.00	\$4,950.00
	Fittings Total			\$2.33/SF	\$4,950.00
	Metals Total			\$21.59/SF	\$45,899.00

Page Total \$45,899

Divisions/Elements/Item

Rates Current At August 2011 Gross Floor Area: 2,126.00 SF

		Unit	Qty	Rate	Total Cost
6	Wood, Plastics, and Composites				
B1010	Floor Construction				
35	2" x 6" Wood stud framing as vertical support to crawl space sub-floor	SF	123	\$6.00	\$775.00
31	11 7/8" TJI Wood joist framing	SF	591	\$5.00	\$2,660.00
32	3/4" T&G Plywood sub-floor	SF	591	\$4.00	\$2,128.00
33	Batt insulation to sub-floor framing	SF	591	\$2.00	\$1,064.00
	Floor Construction Total			\$3.12/SF	\$6,627.00
B1020	Roof Construction				
47	3 1/8" x 9" Glulam roof beam	LF	129	\$14.00	\$1,741.00
48	4" x 6" T&G Wood decking to underside of roof structure	SF	1,488	\$4.00	\$5,357.00
49	1/2" Plywood sheathing	SF	1,488	\$2.00	\$2,678.00
	Roof Construction Total			\$4.60/SF	\$9,776.00
B2010	Exterior Walls				
69	2" x 6" Wood stud exterior wall framing	SF	1,071	\$6.00	\$6,747.00
73	1/2" Plywood sheathing to exterior stud wall framing	SF	1,071	\$2.00	\$1,928.00
72	Exterior wall cladding fixed to stud framing	SF	1,071	\$5.00	\$4,819.00
	Exterior Walls Total			\$6.35/SF	\$13,494.00
C1010	Partitions				
66	2" x 4" Wood stud partition framing	SF	187	\$5.00	\$841.00
67	2" x 6" Wood stud partition framing	SF	264	\$6.00	\$1,663.00
	Partitions Total			\$1.18/SF	\$2,504.00
C1030	Fittings				
91	Casework - Plastic laminate lavatory countertop with 3" high red oak backsplash; 2'-0" Wide	LF	20	\$158.00	\$3,150.00
101	Casework - Plastic laminate adjustable shelving to 8'-0" high; 2'-0" wide (assumed)	LF	17	\$225.00	\$3,825.00
95	Allowance for rough carpentry	SF	2,126	\$1.00	\$1,913.00
96	Allowance for wood blocking	SF	2,126	\$1.00	\$1,913.00
	Fittings Total			\$5.08/SF	\$10,801.00

Divisions/Elements/Item

Rates Current At August 2011 Gross Floor Area: 2,126.00 SF

				31033 1 1001 7 1	ea. 2,126.00 SF
		Unit	Qty	Rate	Total Cost
07	Thermal and Moisture Protection				
A1030	Slab on Grade				
19	2" Rigid insulation; extruded polystyrene	SF	1,147	\$3.00	\$3,097.00
20	Vapor barrier	SF	1,147	\$1.00	\$1,032.00
	Slab on Grade Total			\$1.94/SF	\$4,129.00
B1010	Floor Construction				
34	Vapor barrier to crawl space	SF	1,800	\$1.00	\$1,620.00
	Floor Construction Total			\$0.76/SF	\$1,620.00
B1020	Roof Construction				
51	6" Rigid Insulation; polyisocyanurate	SF	1,488	\$5.00	\$8,035.00
52	3/4" Perlite insulation board	SF	1,488	\$2.00	\$2,678.00
60	4" Rigid insulation to green roof	SF	918	\$4.00	\$3,305.00
57	Acoustic insulation under suspended slab roof construction	SF	918	\$2.00	\$1,652.00
58	Monolithic rubberized asphalt waterproofing membrane to green roof	SF	918	\$5.00	\$4,957.00
59	Drainage mat to green roof	SF	918	\$4.00	\$3,305.00
50	Vapor barrier	SF	1,488	\$1.00	\$1,339.00
	Roof Construction Total		\$	511.89/SF	\$25,271.00
B2010	Exterior Walls				
30	2" Rigid insulation; extruded polystyrene	SF	691	\$3.00	\$1,866.00
70	Fiberglass batt insulation to exterior stud wall framing	SF	1,071	\$2.00	\$1,928.00
26	Monolithic rubberized asphalt waterproofing membrane	SF	1,330	\$5.00	\$7,182.00
28	Drainage mat to exterior wall	SF	1,330	\$4.00	\$4,788.00
71	Vapor barrier to exterior stud wall framing	SF	1,071	\$1.00	\$964.00
	Exterior Walls Total			\$7.87/SF	\$16,728.00
B3010	Roof Coverings				
53	TPO Roofing membrane	SF	1,488	\$14.00	\$20,088.00
54	1" x 6" Synthetic roof decking; removable pallet construction	SF	1,488	\$5.00	\$6,696.00
92	Sheet metal gutter and flashing; 24-ga	LF	47	\$18.00	\$846.00
94	Allowance for miscellaneous flashings and trims	SF	1,488	\$1.00	\$1,339.00
	Roof Coverings Total		\$	613.63/SF	\$28,969.00
	Thermal and Moisture Protection	on Total	\$	36.09/SF	\$76,717.00
	Page Total				\$76,717

Divisions/Elements/Item

Rates Current At August 2011 Gross Floor Area: 2,126.00 SF

fixed/operable of including screet	w-E aluminum framed glazing system, complete	Unit SF	Qty	Rate	Total Cost
Exterior Walls 1" Insulated Lor fixed/operable of including scree	w-E aluminum framed glazing system, complete	QE.			
1" Insulated Loginger fixed/operable gincluding scree	w-E aluminum framed glazing system, complete	QE.			
fixed/operable of including screet	glazing system, complete	CE.			
Louvered crawl	ns at operable panels	SF	858	\$113.00	\$96,525.00
	space access	SF	22	\$27.00	\$594.00
	Exterior Walls Total			\$45.68/SF	\$97,119.00
Exterior Doors	3				
glazed door, co	mplete including insulated	EA	2	\$5,850.00	\$11,700.00
3'-0" x 7'-0" Wood framed single leaf wood veneer door, complete including finish and hardware		EA	5	\$1,080.00	\$5,400.00
		EA	1	\$1,350.00	\$1,350.00
-	Exterior Doors Total			\$8.68/SF	\$18,450.00
Roof Opening	s				
		SF	47	\$180.00	\$8,460.00
-	Roof Openings Total			\$3.98/SF	\$8,460.00
Interior Doors					
3'-0" x 7'-0" Wo		EA	3	\$1,080.00	\$3,240.00
-	Interior Doors Total			\$1.52/SF	\$3,240.00
Fittings					
-	and stainless steel handrail to	LF	133	\$383.00	\$50,873.00
	Fittings Total			\$23.93/SF	\$50,873.00
-	Openings Total			\$83.79/SF	\$178,142.00
	2 @ 3'-0" x 7'-0 glazed door, co glass, rail, finish 3'-0" x 7'-0" Wo veneer door, co hardware 5'-5" x 8'-5" Hig complete include hardware Roof Opening: Prefabricated s Interior Doors 3'-0" x 7'-0" Wo veneer door, co hardware Fittings Glass guardrail rooftop	veneer door, complete including finish and hardware 5'-5" x 8'-5" High overhead coiling door, complete including frame, track, finish and hardware Exterior Doors Total Roof Openings Prefabricated solid glass paver skylight system Roof Openings Total Interior Doors 3'-0" x 7'-0" Wood framed single leaf wood veneer door, complete including finish and hardware Interior Doors Total Fittings Glass guardrail and stainless steel handrail to rooftop	2 @ 3'-0" x 7'-0" Aluminum framed double leaf glazed door, complete including insulated glass, rail, finish and hardware 3'-0" x 7'-0" Wood framed single leaf wood veneer door, complete including finish and hardware 5'-5" x 8'-5" High overhead coiling door, complete including frame, track, finish and hardware Exterior Doors Total Roof Openings Prefabricated solid glass paver skylight system Roof Openings Total Interior Doors 3'-0" x 7'-0" Wood framed single leaf wood veneer door, complete including finish and hardware Interior Doors Total Fittings Glass guardrail and stainless steel handrail to rooftop Fittings Total	2 @ 3'-0" x 7'-0" Aluminum framed double leaf glazed door, complete including insulated glass, rail, finish and hardware 3'-0" x 7'-0" Wood framed single leaf wood veneer door, complete including finish and hardware 5'-5" x 8'-5" High overhead coiling door, complete including frame, track, finish and hardware Exterior Doors Total Roof Openings Prefabricated solid glass paver skylight system SF 47 Roof Openings Total Interior Doors 3'-0" x 7'-0" Wood framed single leaf wood veneer door, complete including finish and hardware Interior Doors Total Fittings Glass guardrail and stainless steel handrail to LF 133 rooftop Fittings Total	2 @ 3'-0" x 7'-0" Aluminum framed double leaf glazed door, complete including insulated glass, rail, finish and hardware 3'-0" x 7'-0" Wood framed single leaf wood veneer door, complete including finish and hardware 5'-5" x 8'-5" High overhead coiling door, complete including frame, track, finish and hardware Exterior Doors Total Roof Openings Prefabricated solid glass paver skylight system Roof Openings Total Roof Openings Total Roof Openings Total S1-0" x 7'-0" Wood framed single leaf wood veneer door, complete including finish and hardware Interior Doors Total \$1.52/SF Fittings Glass guardrail and stainless steel handrail to rooftop Fittings Total \$23.93/SF

Divisions/Elements/Item

Rates Current At August 2011 Gross Floor Area: 2,126.00 SF

		Unit	Qty	Rate	Total Cost
09	Finishings				
C3010	Wall Finishes				
62	5/8" GWB wall lining fixed on stud framing; standard	SF	1,446	\$1.00	\$1,301.00
63	5/8" GWB wall lining fixed on stud framing; moisture resistant	SF	334	\$2.00	\$601.00
65	Paint finish to GWB wall lining	SF	1,779	\$1.00	\$1,601.00
64	Clear acrylic concrete sealer	SF	433	\$1.00	\$390.00
	Wall Finishes Total			\$1.83/SF	\$3,893.00
C3020	Floor Finishes				
39	3/4" x 2 1/4" T&G Wood flooring; Red Oak	SF	591	\$14.00	\$7,979.00
38	Hardener to concrete	SF	1,356	\$2.00	\$2,441.00
147	Clear polyurethane sealer to wood flooring	SF	591	\$1.00	\$532.00
40	Wood base	LF	83	\$13.00	\$1,120.00
41	3" Resilient base	LF	119	\$3.00	\$321.00
	Floor Finishes Total			\$5.83/SF	\$12,393.00
C3030	Ceiling Finishes				
36	Wood spaced board ceiling; CVG Douglas fir	SF	1,580	\$14.00	\$21,330.00
150	Wood spaced board external soffit	SF	284	\$14.00	\$3,834.00
37	Clear finish to wood ceiling and external soffit	SF	1,864	\$2.00	\$3,355.00
114	Allowance for lining to underside of floor framing at Multi-Function apex	SF	20	\$5.00	\$108.00
148	Allowance for painting to exposed steel framing	SF	1,488	\$2.00	\$2,678.00
	Ceiling Finishes Total		\$	14.72/SF	\$31,305.00
	Finishings Total		•	\$22.39/SF	\$47,591.00

Page Total \$47,591



Divisions/Elements/Item

Rates Current At August 2011 Gross Floor Area: 2,126.00 SF

		Unit	Qty	Rate	Total Cost
10	Specialities				
C1030	Fittings				
102	Toilet Accessories - Toilet partition cubicle; standard	EA	1	\$675.00	\$675.00
103	Toilet Accessories - Toilet partition cubicle; HC accessible	EA	2	\$900.00	\$1,800.00
106	Toilet Accessories - Grab rails	EA	2	\$135.00	\$270.00
104	Toilet Accessories - Toilet tissue dispenser	EA	3	\$45.00	\$135.00
108	Toilet Accessories - Toilet seat cover dispenser	EA	3	\$67.00	\$202.00
105	Toilet Accessories - Paper towel dispenser / receptacle	EA	2	\$180.00	\$360.00
107	Toilet Accessories - Soap dispenser	EA	2	\$45.00	\$90.00
90	Miscellaneous - Access hatch to crawl space	EA	1	\$900.00	\$900.00
109	Miscellaneous - Mirror to lavatory countertop	SF	80	\$27.00	\$2,160.00
123	Miscellaneous - Allowance for signage	SF	2,126	\$1.00	\$1,913.00
	Fittings Total			\$4.00/SF	\$8,505.00
	Specialities Total			\$4.00/SF	\$8,505.00

Page Total \$8,505

Divisions/Elements/Item

Rates Current At August 2011 Gross Floor Area: 2.126.00 SF

			G	ross Floor Ar	ea: 2,126.00 SF
		Unit	Qty	Rate	Total Cost
11	Equipment				
E2020	Movable Furnishings				
138	Furniture, Fittings, and Equipment - Excluded (assumed by Owner)	Item			Excl.
	Movable Furnishings Total				
	Equipment Total				
	Page Total				
	i ago iotai				



Divisions/Elements/Item

Rates Current At August 2011 Gross Floor Area: 2,126.00 SF

		Unit	Qty	Rate	Total Cost
15	Mechanical				
D2010	Plumbing Fixtures				
125	Water closet; standard	EA	1	\$1,800.00	\$1,800.00
126	Water closet; HC accessible	EA	2	\$1,800.00	\$3,600.00
127	Urinal; wall mounted	EA	1	\$1,800.00	\$1,800.00
128	Lavatory; counter basin	EA	3	\$1,800.00	\$5,400.00
129	Drinking fountain; dual height	EA	1	\$1,800.00	\$1,800.00
130	Janitors sink; floor mounted	EA	1	\$1,800.00	\$1,800.00
131	Hot water tank; 30-gal	EA	1	\$1,800.00	\$1,800.00
132	Pressure tank; 82-gal	EA	1	\$1,800.00	\$1,800.00
	Plumbing Fixtures Total			\$9.31/SF	\$19,800.00
D3040	Distribution Systems				
133	Allowance for electric forced heating / air-conditioning system	SF	2,126	\$7.00	\$15,307.00
	Distribution Systems Total			\$7.20/SF	\$15,307.00
	Mechanical Total			\$16.51/SF	\$35,107.00

Page Total \$35,107

Divisions/Elements/Item

Rates Current At August 2011 Gross Floor Area: 2,126.00 SF

		Unit	Qty	Rate	Total Cost
16	Electrical				
D5010	Electrical Service & Distribution				
134	Allowance for electrical service and distribution	s SF	2,126	\$5.00	\$9,567.00
	Electrical Service & Distribut	ion Total		\$4.50/SF	\$9,567.00
D5020	Lighting and Branch Wiring				
135	Allowance for lighting and branch wiring	SF	2,126	\$11.00	\$22,961.00
	Lighting and Branch Wiring	Γotal	\$	10.80/SF	\$22,961.00
D5030	Communications & Security				
136	Allowance for data and communications	Item			\$900.00
137	Allowance for security system	SF	2,126	\$2.00	\$3,827.00
139	Allowance for fire alarm system	SF	2,126	\$2.00	\$3,827.00
	Communications & Security	Total		\$4.02/SF	\$8,554.00
	Electrical Total		\$	19.32/SF	\$41,082.00

Total Cost \$723,586.00

Community Correspondence

Chen, Ching-Fang

From: krisnachuck becker [krisnachuck@gmail.com]
Sent: Wednesday, October 27, 2010 8:20 PM
To: Chen, Ching-Fang; Hanley, Wendy

Subject: comment on day use plan for Little Bennett

Hi,

I'm sorry I wasn't able to come to the public meeting about this. As a mother with young children, I expect to use the new facility often, but evening meetings are difficult. I really don't know much about the plan other than the design forwarded on Clarksburg Civic Association group, but I'd like to put in my vote for a natural playground in the "playscape" as they have elsewhere in the county (see Gazette article below.) http://www.gazette.net/stories/06162010/bethnew224147_32554.php

I love the different areas for the butterfly house, and the bugs house, etc. because it gets people to walk outside a little between each.

Is the sculpture something kids could climb on or otherwise interact with?

Meadowside Nature center is a popular destination for us when we need to meet with a few other moms on a cold winter day. Will there be something similar to their small Discovery Room with a carpet for kids to play on, and nature puzzles/toys, or a cave to climb through? Even though it is far from here, it is really one of the very few good places around for free (and educational) places to meet with a playgroup. If this request requires plans for a new structure though, don't worry about it.

In general, I hope the overall plan minimizes forest clearing/mown lawn areas. Seems unnecessary maintenance to me. Also, as the mother of young kids, I'd appreciate a pesticide-free park for them to play in, if they do play on manicured lawn area, or in the playground area.

Thanks for your consideration of these comments/suggestions. We love Little Bennett and use the trails to the stream often. We've camped there twice this year. We expect to use this part of the park too.

-Krisna Becker

Chen, Ching-Fang

From: Sousa, Lou [Lou.Sousa@ee.doe.gov]
Sent: Thursday, September 30, 2010 1:56 PM

To: Chen, Ching-Fang

Subject: Another Look at Little Bennett Park

Hello, Ching.

Thank you for your presentation last night and for asking for our input to your draft facility plan as well as for offering to send me your presentation. I would like to draw from it to put together a short piece about the plan and the whole review/CIP process on the home page of our public website at www.friendsoflittlebennett.org.

When you visit our website, please try to take a look at one of our archived homepage stories about another meadow in the park that shares many of the same serenely scenic characteristics as the one where the day use area will be located: www.friendsoflittlebennett.org./Archive20100611.htm Be sure also to take a look at the online album containing some of my photos of the meadow that accompanies the story at

www.friendsoflittlebennett.org./Images/Prescott.Meadow/index.html

I would feel privileged to show the meadow to you on your next visit to LBRP, if you have time. The absolute best time to see & experience the meadow is June but October would be OK if you can make it out sometime this month. Just let me know. I could take time off from work.

While visiting our website you might also want to take a long at some of our Trails pages (see for example

www.friendsoflittlebennett.org./WesternPiedmontTrail.htm).

For some views of Little Bennett during the last two weeks of October (when the Park is at its scenic peak), see

www.friendsoflittlebennett.org./Images/October.Color.2009/

Lou Sousa President Friends of Little Bennett Park Clarksburg, MD 202-586-9236 (wk)

MEETING REPORT

MEETING DATE: September 29th, 2010

STAFF ATTENDING: Mitra Pedoeem M-NCPPC Park Development Division (PDD) Chief

Patricia McManus M-NCPPC PDD Design Section Supervisor

Ching-Fang Chen M-NCPPC PDD Landscape Architect

Eileen Emmet M-NCPPC PDD Architect

Jim Humerick M-NCPPC Northern Region Operations Manager

Wendy Hanley M-NCPPC Park Manager Lanshing Hwang Symbiosis, Inc. Consultant

Steve Torgerson A. Morton Thomas & Associates, Inc. Consultant Matthew Ernest A. Morton Thomas & Associates, Inc. Consultant

PROJECT: Little Bennett Regional Park, Day Use Area-Facility Planning

SUBJECT: Community Meeting

AGENDA

- Introduction Patricia McManus
- Purpose of the meeting, facility planning process and schedule- Patricia McManus
- Background, site and approach Ching-Fang Chen/Lanshing Hwang
- Concept alternative- Ching-Fang Chen/Lanshing Hwang
- Question session Patricia McManus/Staff Team
- Summary Patricia McManus

PUBLIC COMMENTS AND DISCUSSION

The concept for the Day Use Area was presented to approximately 15 attendees, the majority of whom live in the surrounding area. Many of the attendees are familiar with the site and participated in the planning process for the 2007 Little Bennett Regional Park Master Plan.

The key concept of preserving the cultural landscape and capturing the spirit of the land was presented along with the approach of respecting the culture, ecology and land use of the site to achieve a balance for conservation, recreation, education and service. The design elements are intended to efficiently to serve multiple functions and to provide flexibility for the future, and included nature-based recreation and integrated educational and play experiences. Sustainable design strategies for circulation, vegetation, habitat protection, material recycling, energy harvesting and storm water management were discussed. Specific comments and suggestions are summarized below in general categories.

Vehicular Circulation and Parking

- Consider sight lines and locations of acceleration and deceleration lanes at park entrances.
- What kind of traffic will be generated by the park and how many parking spots are provided? Staff indicated that initial estimates are for 20-30 peak hour trips. Parking is provided for 450 cars, based on the program of requirements from the park master plan.
- Consider the potential for widening of Route 355 and how it might affect the location of the Meadow Drive and buffer landscape.
- The surface material for the Meadow Drive should be bike and pedestrian-friendly. Gravel may not be desirable.
- Provide some equestrian parking for the multi-use trail. Consider the locations and quantity of existing trailer parking available in other areas of the park to determine the need for the day use area.

Trails

- Where are the current natural surface trail connections? There is an existing multi-use natural surface trail that connects to the day use area at the northern portion of the site near the existing park house, and a hiker-only trail connection at the southeast portion of the site near Soper's Branch.
- Consider how the plan addresses the Clarksburg Greenway hard surface trail plan. The original plan located the trail immediately adjacent to Route 355 and not further inside the park as proposed.
 The trails should be separated from the driveway and vehicular traffic and not combined.
- Provide trail connectivity from Snowden Farm Parkway to the Clarksburg Greenway.
- Provide a trail connection from the Woodcrest community behind the maintenance yard to the day use area.
- Concern was expressed about allowing pedestrian access to any area of the meadow and whether
 this would damage the ecological health of the meadow. It was suggested that access should be
 restricted to minimize adverse impacts. Others indicated that visitors usually stay within mown
 paths, because they are not comfortable strolling in a tall meadow. Park staff intends to monitor
 the use and will protect the meadow as necessary.
- Provide accessibility to the playground for grandparents and children, and consider distances that people need to walk.

Program Elements

- Consider the safety of the playground and ensure that there is adequate visibility for parents watching their children.
- If considering restrooms or composting toilets, one citizen mentioned Liberty Town Park in Frederick, Maryland as a good example.
- Provide some picnic areas with covered shelters, so that there are different types of picnicking opportunities for different types of users.
- Consider relocating picnic areas further from MD 355 due to noise. A noise study should be conducted, and noise levels should be considered in the park design and location of picnic areas.
- Consider alternative means of identifying the park entrance than the white poles. The design of these elements is too modern and many people may have difficulty relating to them.
- Consider the purpose of the camp fire feature, if the park is closed at dusk. If the park will be used for special events or evening rentals, would additional lighting of the park be needed?

- Would the amphitheater be used for rock concerts or fireworks? This area is envisioned as a nature based retreat, so these uses would be incompatible. These uses would be more suitable for nearby recreational parks with athletic complexes, such as South Germantown Recreational Park.
- Consider providing interpretive information and kiosks at parking, gathering or trailhead areas along the Meadow Drive, to provide information for those who prefer not to walk long distances. The history of MD 355 should be interpreted in the park.
- Consider how the plan could accommodate winter activities, such as sledding or cross-country skiing.
- Use of the site at night for star gazing was suggested. The park manager indicated that there are other locations in the park that provide better opportunities and have less light pollution.

Site

- A neighbor of the park mentioned that there are two existing wells near the southern stream valley which may yield approximately 20 gallons per minute.
- Concerns were expressed regarding erosion problems near the southern property border and Soper's Creek.

Operational Considerations

- There was discussion about the challenges of operating and staffing this new facility. The design approach taken will result in reduced operating costs from the original master plan. The participants expressed a willingness to support the project.
- The project should seek opportunities to advertise, sell, persuade and inform the community
- Naming opportunities should be considered
- Atypical approaches to park operations and maintenance should be considered, including use of volunteer groups, partnerships, or use of correctional facility inmates.

Additional Comments and Discussion

There was discussion about why the concept changed from the Master Plan concept. Staff indicated that the development of the new concept was inspired by recent sustainable design approaches and an awareness of green strategies that have become more prevalent since the completion of the 2007 Master Plan. The proposed concept will be able to fulfill the program of requirements from the master plan, protect the unique culture and ecological landscape, increase the sustainability of the site, reduce costs, and reduce the developed footprint of the site. The attendees expressed general support for the design approach, the overall plan objectives and design elements, the maintenance and preservation of natural areas and open space, and indicated that the plan took a responsible approach that would address the needs of future generations.

This report was prepared by: Ching-Fang Chen, Landscape Architect/Project Manager, October 26, 2010

Agency Correspondence

MEETING REPORT

MEETING DATE: August 16, 2011

STAFF ATTENDING:

Gail Tait-Nouri DOT
Bob Simpson DOT
David Anspacher Planning

Ki Kim Planning Area 3

Charles Kines Parks Park Planning & Stewardship

Wendy Hanley Parks Northern Region
Patricia McManus Parks Park Development
Marian Elsasser Parks Park Development
Ching-Fang Chen Parks Park Development

PROJECT: Little Bennett Regional Park, Day Use Area-Facility Planning

SUBJECT: Trail and Sidewalk Concept

The M-NCPPC and Montgomery County Department of Transportation (MCDOT) staff met and discussed trail connection issues along MD Route 355 between M-NCPPC Maintenance Yard and Hyattstown. Staff revisited recommendations, memorandums and plans from 2005 Countywide Bikeways Functional Master Plan, 1994 Clarksburg Master Plan & Hyattstown Special Study Area (Greenway Network and Bikeway Plan), and 2007 Little Bennett Regional Park Master Plan (Clarksburg Greenway). Staff discussed the trail concept for the corridor along MD Route 355 to ensure consistency, efficiency and safety. Proposed trail for the facility plan of Little Bennett Regional Park-Day Use Area and future projects for countywide shared use path, bikeway and sidewalk connections have been coordinated and outlined. The comments are summarized as follows.

• MCDOT staff confirmed that SP-72 Countywide Shared Use Path per 2005 Countywide Bikeways Functional Master Plan and B-3 Frederick Road Bikeway per 1994 Clarksburg Master Plan & Hyattstown Special Study Area will be located on the west side of MD Route 355 within the road right-of-way and be completely independent from the park recreational trails. The path will support the future Corridor Cities Transitway and provide non-vehicular access for the community. The right-of-way on the west side of MD Route 355 is unconstrained by grade and trees and would best support the bikeway. DOT typically also builds a public sidewalk in the right-of-way on the opposite side of the road from a bikeway. In this case the sidewalk would be constructed on the east side of MD 355. DOT does not anticipate including the bikeway and sidewalk project in their CIP anytime in the near future.

- The Day Use Area is a major destination along MD Route 355 corridor. A trail connection from the existing trail on MD 355 near the M-NCPPC Maintenance Yard to the Day Use Area is desirable in the short term to provide a pedestrian connection from the Clarksburg Town Center. The 2007 Little Bennett Regional Park Master Plan recommends that the trail be set back from the road right-of-way a minimum of 50 feet to provide a better park experience. Staff discussed that the alignment may not be feasible due to environmental constraints such as steep slopes and encroachment to the wetland and stream buffer of the Sopers Branch. In addition, M-NCPPC does not own the parcel immediately south of the Day Use Area, and it may not be practical to bring public access in close proximity behind the private property. Staff recommends aligning an 8' hard-surface trail/sidewalk connection on the east side of MD Route 355 and stay fairly close to the road, so that this connection can serve dual purpose as a park trail connection and the future public sidewalk on MD 355. Park staff is recommending initiating a separate trail facility plan or design project to study this important connection from Stringtown Road to the Day Use Area and will coordinate the design with DOT.
- Once the sidewalk/trail on the east side of MD 355 reaches the Day Use Area, the sidewalk/trail will be located within the park on the east side of the access road, Meadow Drive, from the South Entrance to the Park House Picnic and Parking Area. This will be a separated, eight-foot wide hard surface path. Pavement design, landscape strips, signs and lighting will be studied in the detailed design phase to ensure safety and conformance to trail design guidelines.
- The path at the north end of the day use area will connect to a future natural surface trail connection within the park, as well as the future DOT sidewalk on the east side of MD 355 north of the Day Use Area to Hyattstown.
- Clarksburg Greenway through the interior of the Little Bennett Regional Park as recommended in the <u>2007 Little Bennett Regional Park Master Plan</u> and <u>1994 Clarksburg</u> <u>Master Plan & Hyattstown Special Study Area</u> can remain natural surface to protect the environmentally sensitive area along Little Bennett Creek.

Trail Concept is summarized in the following diagram:

Park Multi-purpose Trail natural surface trail Park House (M-NCPPC CIP project) Picnic & Parking County Sidewalk 5' min. width, hard-surface sidewalk North-Infrance on east side of MD Route 355 in ROW and Park property, connect to Hyattstown (DOT future project) Park Recreation Trail/Sidewalk 8' wide hard-surface path on east side of the Meadow Drive, begin from the South Entrance to the Park House Picnic and Parking area, connect to park Little Bennett Regional Park multi-purpose trail (M-NCPPC CIP project) Day Use Area Meadow Drive SP-72 Countywide Shared Use Path B-3 Clarksburg Master Plan Bikeway Class I, hard-surface, off-road bikeway on west side of MD Route 355 in ROW. connect from Clarksburg Town Center to Hyattstown (DOT future project) outh Entrance Park Connector Trail/Sidewalk 8' wide hard-surface sidewalk on east side of MD Route 355 in ROW and Park property, connect to M-NCPPC Maintenance Yard (M-NCPPC future project)

This report was prepared by:

Ching-Fang Chen Landscape Architect/Project Manager

Copy: Attendees

Distributed for Review: August 22, 2011



July 29, 2011



Attn: Richard Brush c/o Tom Weadon Montgomery County Department of Permitting Services 255 Rockville Pike, 2nd Floor Rockville, MD 20850

> Re: Little Bennett Regional Park Day Use Area - Facility Plan

> > North Frederick Road Clarksburg, MD 20871

Stormwater Concept #: 239813

AMT #: 108-157.008

Dear Brush,

In response to your comments received on April 18, 2011, we offer the following:

Comment #1 The maximum drainage area to a micro biofilter will be 20,000 SF. Underdrains will be required for the micro biofilter. A modified design may be utilized with meeting

the minimum criteria for micro biofilters.

Response Six (6) Sycamore Rings still remain on the plans, but only two (2) will be used for ESD credit.

The two (2) used for ESD both have drainage areas less than 20,000 SF. The others will be

used for the safe-conveyance of off-site drainage.

Comment #2 Clear maintenance access must be provided to all structures.

Clear maintenance access is now provided to all structures. The majority of the ESD credit is Response

met through road-side bioswales and Permeable Pavement, which have access by Meadow

Drive. The two (2) proposed biofilters are adjacent to Meadow Drive.

Comment #3 A Floodplain Study will be required for all areas of the site that have a drainage area

of greater than 30 acres.

Response A formal Floodplain study will be prepared at final design, if required by DPS. For now, a

Concept Level Floodplain study has been included for any area on-site draining more than 30

acres. See Appendix E.

Comment #4 A complete geotechnical report for the proposed development will be required for

concept approval.

Response A complete Geotechnical Report is included in Appendix E.

Comment #5 Explore using porous pavement for the pathways and parking spaces to obtain

additional storage volume. Explore using enhanced filters for additional storage

volume to meet the required ESDv goal.

Response Porous Pavement is now proposed for several of the parking areas. See "Impervious Area Map

Exhibit"; Appendix B. Enhanced Filters will be used for additional volume at all bio filters to

address recharge volume.

Comment #6	The protection of the Stream Valley Buffers is critical to Environmental Site Design. All proposed disturbance and development should be removed and relocated from the Stream Valley Buffers.
Response	M-NCPPC - Park Development has been coordinating with M-NCPPC - Environmental Planning, specifically Josh Penn and John Carter, regarding encroachments to the Stream Buffer. The design team was instructed that encroachments are okay as long as they fall outside of the tree line. A letter to DPS confirming this determination will be issued prior to SWM Concept Approval.
Comment #7	The existing 1.25 ac of impervious area should be included in the proposed percentage of overall site imperviousness.
Response	The existing impervious area (now 0.76 per project limit clarification) is included in the overall site impervious.
Comment #8	The SWM structures must be designed to their target ESDv and all other options for additional volume exhausted prior to maximizing volume in each structure.
Response	All SWM structures are now designed to their target ESDv. See calculations; Appendix B.

Please contact me at (301) 881-2545 or charper@amtengineering.com if you have any questions or need any additional information.

Sincerely,

A. Morton Thomas and Associates

Chuck Harper, **P.E.** Project Manager



September 6, 2011

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Civil Engineering News Magazine

Attn: Richard Brush c/o Tom Weadon Montgomery County Department of Permitting Services 255 Rockville Pike, 2nd Floor Rockville, MD 20850

Re: Little Bennett Regional Park
Day Use Area – Facility Plan

North Frederick Road Clarksburg, MD 20871

Stormwater Concept #: 239813

AMT #: 108-157.008

Dear Brush,

In response to the comments discussed on August 25, 2011, we offer the following:

in response to the	e comments discussed on August 25, 2011, we offer the following:
Comment #1	Calculate the PE value for a project using Study Points, not the Parcel area. Depending on engineering approach, there will be between two (2) and four (4) study points for the Little Bennett project. A map showing the study points and respective drainage areas shall be included with the next submission.
Response	The project has been divided into two (2) study points, POS-1 and POS-2. A map showing the study points and the respective drainage areas has been included in Appendix B1.
Comment #2	Study Points should not include any areas to the East of Soper Branch, since no work is proposed on that side of the stream. Site impervious will likely remain below 15%, keeping the PE = 1 ".
Response	Study points shown in Appendix B1 adhere to the criteria outlined above. The PE for both Study Points remained at 1".
Comment #3	The calculation of "Rv" (the dimensionless volumetric coefficient) was clarified. Rv = $0.05 + 0.009 *$ (Iproject) where (Iproject) is the percent impervious within the Project Area, or Limits of Disturbance (LOD).
Response	The Rv calculation has been revised as described above. See Appendix B2.
Comment #4	The ESD Volume required calculation was clarified. ESDv = [(PE * Rv * A) / 12], where Rv is calculated as described in Item #3, and A = Project Area (LOD). The ESDv required for Little Bennett will increase from $\pm 8,000$ CF to $\pm 30,000$ CF using this methodology.
Response	The ESD Volume required calculation has been revised as described above.
Comment #5	Permeable Pavements, Green Roof areas, and Reinforced Turf areas shall be treated as impervious for the calculation of (Iproject).
Response	The computations have been revised to include Alternative Surfaces in the calculation for (Iproject). See Appendix B2.

Comment #6	The infiltration test for the permeable pavement parking area to the North (P-1 and P-2) showed a rate of 26.01"/hr. Permeable Pavements will not be acceptable to DPS when infiltration rates exceed 6" per hour, or are less than 0.52"/hr. The Geotechnical Engineer should explain potential reasoning for the high infiltration rate in report, similar to what was done for the low rate at IT-4. (See Page 6 of Geotechnical Report).
Response	This was discussed with the Geotechnical Engineer of record. It is believed that this result of the infiltration test was anomalous. Additional Testing will be performed at the time of Final Design. For the purpose of this Concept, a second option is shown eliminating the Permeable Pavement in this area, and adding two (2) micro-bioretentions to treat the run-off from the parking area. Full ESD is met with both scenarios.
Comment #7	Current DPS policy is that Porous Concrete is the only acceptable permeable pavement for use in Parking Areas.
Response	Noted.
Comment #8	An informal Floodplain Study for Soper Branch is required with the next submission. AMT shall verify that the 100-year Floodplain is contained within the Stream Buffer. Details required include Drainage Areas, TR-55, TR-20 and Cross-Sections. (Water Surface Elevations do not need to be plotted on the plans).
Response	An informal Floodplain Study for Soper Branch has been provided with this submission. See Appendix E.
Comment #9	A Letter from M-NCPPC stating that the proposed encroachments into the Stream Buffer are acceptable is required <u>prior</u> to SWM Concept approval by DPS.
Response	As discussed via email on August 30, M-NCPPC requests that conditional approval of the Stormwater Concept be granted, pending determination by the Planning Board for the Stream Buffer encroachments.
Comment #10	Additional Geotechnical Testing is required for Permeable Pavement areas greater than 10,000 SF, in particular areas P-2 (if not removed per item #6 above) and P-3.
Response	Permeable Pavement locations have been revised so that no area is larger than 10,000 SF.
Comment #11	Permeable Pavement Area P-4 is not an acceptable application of Permeable Pavement. The impervious area draining to the permeable pavement is larger than the permeable pavement area itself. As a rule of thumb, 50% of the drainage area to Permeable Pavement shall be the permeable pavement itself.
Response	The Permeable Pavement Parking Area previously labeled "P-4" has been converted to gravel, and now drains to bio swale "S-5".
Comment #12	The Geotechnical Report shall be revised to verify that <u>ALL</u> infiltration testing was done per Montgomery County Standards. The item of confusion may be the "unofficial" Perc Test performed, and associated narratives. Language shall be added to the report to clarify infiltration-testing techniques.
Response	Language clarifying the infiltration testing techniques was added to Section 2.3 on Page 4 of the Geotechnical Report. See Appendix F.

Comment #13	Typical Cross-sections for the Permeable Pavement systems, bio filters, and bio swales shall be included with the next submission.
Response	Plan Sheet CW501 showing cross-section for the ESD measures shown has been included with this submission. See Appendix B1.
Comment #14	To increase Volume credit in the bio swale, increase the media thickness from 2' to 3', and count the 6" of sand beneath the media. DPS is still debating internally whether to count the voids in the stone beneath the sand layer. For computation purposes show the media thickness as 3.5'.
Response	The bio swale computations have been updated to show 3.5' media thickness. See Appendix B2.
Comment #15	The playscape area can be modeled as Permeable Pavement, similar to what is shown on the MNCPPC "Fibar" detail. DPS has not yet determined if spray rubber or poured-in-place rubber will be acceptable surface materials. In the meantime, AMT shall add statement to the report that "Surface Material of the Playscape will require DPS approval at the time of final design."
Response	The placyscape area is now modeled as Permeable Pavement. The surface material will be approved by DPS at final design.
Comment #16	The minimum ponding depth on a micro-bioretention shall be 6". Computations currently show 3". Please revise.
Response	The micro-bioretention computations have been revised to show a 6" ponding depth. See Appendix B2.

Please contact me at (301) 881-2545 or charper@amtengineering.com if you have any questions or need any additional information.

Sincerely,

A. Morton Thomas and Associates

Chuck Harper, P.E.Project Manager



From: Chuck Harper, P.E. April 18, 2010

Little Bennett Gateway Facility Plan

Project: Stormwater Management Concept – Permit AMT: 108-157.008

#239813

 Meeting Number:
 Meeting Date:
 Project Phase:
 Meeting Location

 DPS-1
 April 14, 2011
 Schematic Design
 DPS, 255 Rockville Pike; 2nd Floor

Meeting Purpose:

Discuss the Stormwater Management Concept plan, submitted to DPS on March 14, 2011.

Attendees: Distribution: Attachments:

Ching-Fang Chen (MNCPPC) DRAFT Comment Letter from DPS

Matthew Ernest (AMT) Chuck Harper (AMT) Tom Weadon (MC DPS) Rick Brush (MC DPS)

Item .	ction Description
1.	Ching-Fang opened the meeting by introducing the vision and intent of the Little Bennett Park design.
2.	Tom Weadon distributed a "DRAFT" comment letter for discussion (attached).
3.	Comment #1, Drainage Area limitations. DPS stated that the maximum drainage area to a micro-bioretention facilit shall be 20,000 SF.
4.	AMT modeled Sycamore Rings as three (3) separate facilities. DPS will treat Sycamore Rings as one (1) facility. The drainage area shall be limited to ±20,000 SF per Sycamore Ring. AMT requested the required distance between facilities; DPS did not provide a specific distance.
5.	AMT and MNCPPC stated that the Original Master Plan for Little Bennett Park called for roads throughout the Park. One goal of Environmental Site Design is limiting impervious area. The current design proposes significantly less impervious area than the Master Plan. Rick Brush said to document this in the SWM Concept Report.
6.	DPS stated that Sycamore Rings with large drainage areas (example Southwestern most Sycamore Ring) can be treated as "safe conveyance" when not used for Stormwater Management credit.
7.	Comment #6, Stream Valley Buffer encroachments. Rick Brush requested something in writing that Stream Buffer encroachments are acceptable to other regulatory agencies. This will be required for DPS to support plan approach
8.	Rick Brush suggested coordination with DEP, Anne English specifically.
9.	DPS stated that the Facility Plans should address stream improvements.
10.	Maintenance was discussed. DPS stated that we can work together on this once facility locations are determined.
11.	Comment #3, Floodplain Studies. DPS wants Floodplain Studies for drainage areas greater than 30 acres. For concept plan, the Floodplain Study can be conceptual.
12.	Gravel areas shall be treated as impervious, and must be treated by ESD measures. Compacted paths shall be treated as impervious for the purpose of calculating ESD volumes, but do not need to be treated.
13.	DPS informed AMT and MNCPPC of the new review procedure. Plans are distributed to DEP and DOT. Comments are returned within 21 days and included in the comment letter.
14.	Comment #8 was clarified. Since compacted paths are treated as impervious for the purpose of calculating volume facilities will be oversized to account for incremental increases.



Chuck Harper, P.E.

A Morton Thomas and Associates

Note: The above meeting minutes are the interpretation of the writer. If you feel any item requires correction or clarification, please notify the writer within 10 business days.



April 12, 2011

Mr. Chuck Harper, C. E. A. Morton Thomas and Associates, Inc. 12750 Twinbrook Parkway Rockville, MD 20852

Re:

Stormwater Management CONCEPT Request

for Little Bennett Day Use Area

Preliminary Plan #: N/A SM File #: 239813

Tract Size/Zone: 136.7/RDT Total Concept Area: 36.5ac

Lots/Block: N/A Parcel(s): -

Watershed: Little Bennett creek

Dear Mr. Harper:

Based on a review by the Department of Permitting Services Review Staff, the stormwater management concept for the above mentioned site is unacceptable. The stormwater management concept proposes to meet required stormwater management goals via biofitration and grass swales.

Please submit a revised stormwater management concept for review and approval. All submissions must be accompanied by a resubmittal application. The revised submission must incorporate the following items:

- 1. The maximum drainage area to a micro biofilter will be 20,000sf. Underdrains will be required for the micro biofilters. A modified design may be utilized with meeting the minimum criteria for micro biofilters.
- 2. Clear maintenance access must be provided to all structures.
- 3. A floodplain study will be required for all areas of the site that have a drainage area of greater than 30 acres.
- 4. A complete geotechnical report for the proposed development will be required for concept approval.
- Explore using porous pavement for the pathways and parking spaces. Use environment of the full for cudicular values.
 The protection of the Stream Valley Buffers is critical to environmental site design. All proposed disturbance and development of the stream value.
- disturbance and development should be removed and relocated from the Stream Valley Buffers.
- 7. The existing 1.25ac of impervious area should be included in the proposed percentage of overall site imperviousness
- 8. The SWM structures must be designed to their target ESDv and all other options for additional volume exhausted prior to maximizing volume in each structure.

This list may not be all-inclusive and may change based on available information at the time.

If you have any questions regarding these actions, please feel free to contact Thomas Weadon at 240-777-6309.

Sincerely,

Richard R. Brush, Manager Water Resources Section Division of Land Development Services

RRB: tla CN 239813

CC:

C. Conlon

SM File # 231893

ESD Acres:

Denled

STRUCTURAL Acres:

N/A

WAIVED Acres:

N/A





From:	Chuck Harper, P.E.	Julie 3, 2011

Project:Little Bennett Gateway Facility PlanAMT: 108-157.008

Meeting Number:Meeting Date:Project Phase:Meeting LocationDPS-2June 1, 2011Design DevelopmentMontgomery County DPS 255 Rockville Pike; 2nd Floor

Meeting Purpose:

Underlook Building Pre-Design Consultation

Attendees:	Distribution:	Attachments:
11 1 0: 1 (000)		

Melvin Stansbrough (DPS) Ching-Fang Chen (MNCPPC) Eileen Emmet (MNCPPC) Chuck Harper (AMT) Jim Cutler (Cutler Anderson) Lanshing Hwang (Symbiosis)

All Attendees

Item	Action	Description
1.		Ching-Fang opened the meeting by introducing the vision and intent of the Little Bennett Park design.
2.		Melvin outlined the applicable building codes. The plan will follow the 2009 IBC, the 2003 Life Safety Code, NEC 2008, and the 2010 ADA Code.
3.		For ADA, the County has no new adoptions to the code. At the time of final design, analysis should be done regarding any new adoptions by the State of Maryland. (The State will typically make some requirements more stringent).
4.		Jim described the two routes to the building (direct: non-ADA, meandering: ADA), and gave an overview of building materials. Half of the building is a concrete structure; the other half is wood/steel.
5.		Design team was instructed to coordinate with Marie LeBaw, the Fire Marshal contact.
6.		Melvin stated that the building's distance from the nearest public roadway (≥350') would prompt sprinkler system requirement.
7.		Melvin stated that Montgomery County requires a 20' paved vehicular access road. The current Little Bennett plan shows ±8'.
8.		Design Team was instructed that Fire Code requirements are outlined in Chapter 22 of the County Code.
9.		Jim threw out the idea of standpipes in lieu of sprinklers and larger access road. This will be discussed with Marie LeBaw at future meeting TBD.
10.		Melvin stated that if there are to be night activities, the emergency path may require lighting of 1 ft-candle.
11.		Melvin recommended that MNCPPC schedule a DRC meeting for the project.
12.		Lanshing confirmed that there are no specific EMS access requirements for the play area. Melvin mentioned that EMS can use ATV's along the accessible path.

Chuck Harper, P.E.

A Morton Thomas and Associates

Note: The above meeting minutes are the interpretation of the writer. If you feel any item requires correction or clarification, please notify the writer within 10 business days.

Little Bennett Gateway Facility Plan AMT: 108-157.008





From: Chuck Harper, P.E. June 17, 2011

Project:Little Bennett Gateway Facility PlanAMT: 108-157.008

 Meeting Number:
 Meeting Date:
 Project Phase:
 Meeting Location

 DPS-3
 June 14, 2011
 Design Development
 Montgomery County DPS 255 Rockville Pike; 2nd Floor

Meeting Purpose:

Meeting with MC DPS to discuss Well and Septic.

Attendees: Distribution: Attachments:

All Attendees

Gene Von Gunten (DPS)
Ching-Fang Chen (MNCPPC)
Eileen Emmet (MNCPPC)
Chuck Harper (AMT)
Bruce Anderson (Cutler Anderson)
Lanshing Hwang (Symbiosis)

Item	Action	Description
1.		Ching-Fang opened the meeting by introducing the vision and intent of the Little Bennett Park design.
2.		Gene stated that based on the locations of the two (2) existing wells on-site, the design should propose a new well location.
3.		The minimum size of a septic field is 10,000 SF.
4.		Composting toilets will not work for this application due to the location of the bathrooms.
5.		A sand mound could be used if the site does not Perc. A sand mound can work on up to a 12% grade.
6.		This site offers an opportunity to propose a "greener" system, which uses a sand mound in lieu of a drain field, and an aerobic unit underground in lieu of a standard septic tank.
7.		Sand mounds can have plantings, but no trees. Top soil can be added to buffer the appearance.
8.		The required distance to bedrock was discussed. A site must have 6.5'-7' of soil before encountering bedrock to use a septic tank and drain field. A site must be 2' minimum of soil to use a sand mound, and 4' minimum to the water table. If the depth is shallower than 1', there are no options.
9.		A formal Perc Test must be performed between February 1 and April 15.
10.		The formula for calculating drain field size was provided. (5 gallons per person) x (the number of users) x (a factor of 20) = Size in Square Feet. (Note: the minimum allowable size is 10,000 SF.)
11.		CFC, BA, and CH to coordinate and determine demand so plan depicts accurate drain field size.
12.		GVG suggested that M-NCPPC explore the use of a Geothermal Heating System.

Chuck Harper, P.E.

A Morton Thomas and Associates

Note: The above meeting minutes are the interpretation of the writer. If you feel any item requires correction or clarification, please notify the writer within 10 business days.



From: Chuck Harper, P.E. August 25, 2010

Little Bennett Gateway Facility Plan

Project: Stormwater Management Concept – AMT Project: 108-157.008

Permit #239813

 Meeting Number:
 Meeting Date:
 Project Phase:
 Meeting Location

 DPS-4
 August 25, 2011
 SWM Concept
 DPS, 255 Rockville Pike; 2nd Floor

Meeting Purpose:

Discuss the Stormwater Management Concept plan, submitted to DPS on August 2, 2011.

Attendees: Distribution: Attachments:

Tom Weadon (MC DPS)
Chuck Harper (AMT)
Ching-Fang Chen (MNCPPC)
Matthew Ernest (AMT)
Stephen Reid (MNCPPC)

The following is a summary of DPS comments on the Little Bennett SWM Concept:

Item	Action	Description
1.	AMT	Calculate the PE value for a project using Study Points, not the Parcel area. Depending on engineering approach, there will be between two (2) and four (4) study points for the Little Bennett project. A map showing the study points and respective drainage areas shall be included with the next submission.
2.	AMT	Study Points should ignore off-site areas, and should not include any areas to the East of Soper Branch, since no work is proposed on that side of the stream. Site impervious will likely remain below 15%, keeping the PE = 1".
3.	AMT	The calculation of "Rv" (the dimensionless volumetric coefficient) was clarified. Rv = 0.05 + 0.009 * (Iproject) where (Iproject) is the percent impervious within the Project Area, or Limits of Disturbance (LOD).
4.	AMT	The ESD Volume required calculation was clarified. ESDv = [(PE * Rv * A) / 12], where Rv is calculated as described in Item #3, and A = Project Area (LOD). The ESDv required for Little Bennett will increase from $\pm 8,000$ CF to $\pm 30,000$ CF using this methodology.
5.	AMT	Permeable Pavements, Green Roof areas, and Reinforced Turf areas shall be treated as impervious for the calculation of (Iproject).
6.	AMT/TLB	The infiltration test for the permeable pavement parking area to the North (P-1 and P-2) showed a rate of 26.01"/hr. Permeable Pavements will not be acceptable to DPS when infiltration rates exceed 6" per hour, or are less than 0.52"/hr. The Geotechnical Engineer should explain potential reasoning for the high infiltration rate in report, similar to what was done for the low rate at IT-4. (See Page 6 of Geotechnical Report).
7.		Current DPS policy is that Porous Concrete is the only acceptable permeable pavement for use in Parking Areas.
8.	AMT	An informal Floodplain Study for Soper Branch is required with the next submission. AMT shall verify that the 100-year Floodplain is contained within the Stream Buffer. Details required include Drainage Areas, TR-55, TR-20 and Cross-Sections. (Water Surface Elevations do not need to be plotted on the plans).
9.	MNCPPC	A Letter from M-NCPPC stating that the proposed encroachments into the Stream Buffer are acceptable is required <u>prior</u> to SWM Concept approval by DPS.
10.	AMT/TLB	Additional Geotechnical Testing is required for Permeable Pavement areas greater than 10,000 SF, in particular areas P-2 (if not removed per item #6 above) and P-3.
11.	AMT	Permeable Pavement Area P-4 is not an acceptable application of Permeable Pavement. The impervious area draining to the permeable pavement is larger than the permeable pavement area itself. As a rule of thumb, 50% of the drainage area to Permeable Pavement shall be the permeable pavement itself.

Little Bennett Gateway Facility Plan AMT: 108-157.008



12.	AMT/TLB	The Geotechnical Report shall be revised to verify that <u>ALL</u> infiltration testing was done per Montgomery County Standards. The item of confusion may be the "unofficial" Perc Test performed, and associated narratives. Language shall be added to the report to clarify infiltration-testing techniques.
13.	AMT	Typical Cross-sections for the Permeable Pavement systems, bio filters, and bio swales shall be included with the next submission.
14.	AMT	To increase Volume credit in the bio swale, increase the media thickness from 2' to 3', and count the 6" of sand beneath the media. DPS is still debating internally whether to count the voids in the stone beneath the sand layer. For computation purposes show the media thickness as 3.5'.
15.	AMT	The playscape area can be modeled as Permeable Pavement, similar to what is shown on the MNCPPC "Fibar" detail. DPS has not yet determined if spray rubber or poured-in-place rubber will be acceptable surface materials. In the meantime, AMT shall add statement to the report that "Surface Material of the Playscape will require DPS approval at the time of final design."
16.	AMT	The minimum ponding depth on a micro-bioretention shall be 6". Computations currently show 3". Please revise.

Chuck Harper, P.E.

A Morton Thomas and Associates

Note: The above meeting minutes are the interpretation of the writer. If you feel any item requires correction or clarification, please notify the writer within 10 business days.

Chen, Ching-Fang

From: Chen, Ching-Fang

Sent: Friday, May 27, 2011 2:21 PM

To: Carter, John

Cc: Penn, Joshua; McManus, Patricia; Frank, Andrew; Pedoeem, Mitra

Subject: Little Bennett RP-Day Use Area facility plan

Attachments: LB-DD-PreFCP.pdf

Good afternoon,

We are preparing to submit our Preliminary Forest Conservation Plan for the Little Bennett Regional Park-Day Use Area. Our proposed playground is located within stream buffer areas with impacts to forest, in order to protect a unique meadow habitat and provide linkages from the play area to environmental interpretive areas. We met with Environmental Planning staff to discuss our proposed plan, and we understand that our plan would likely be denied based on a strict interpretation of the Forest Conservation Law. The Forest Conservation Law is written to protect forest, floodplains, stream buffers, endangered species and significant trees. It offers no such protection for meadow habitats. We would like the opportunity to explain the background and intent of our project and request your guidance for how to proceed with forest conservation requirements.

The proposed site for the Day Use Area is located east of MD 355 and west of the Sopers Branch, approximately 65 acres in size. The existing site is primarily undeveloped old field with pockets of biodiversity areas. The 2007 Little Bennett Regional Park Master Plan recommends developing a Day Use Area to welcome people and provide them with an overview of the natural and cultural interpretive opportunities of the entire park. The program of requirements in the master plan includes a visitor and nature center, access road, parking, group picnic areas, a nature-based adventure playground, group fire ring, amphitheatre, trails and opportunities for nature interpretation. The Department of Parks has deferred the development of the visitor and nature center as part of this project, due to current economic conditions and the high cost of developing and operating this facility. The proposed project includes a small visitor station and outdoor classroom to provide opportunities for interpretation of nature.

When we began this project, the Department's Natural Resources Supervisor and Forest Ecologist identified the existing cultivated meadow as one of very few continuous meadow landscapes left in the County, and a high quality example with very few invasive species. The rolling topography reflects the nostalgic piedmont landscape. We observed ground nesting birds on the site, which are rarely seen anymore including wild turkey and meadowlarks. As the County continues to develop, our parkland becomes ever more important for preserving our natural heritage and biological diversity. Meadow dependent birds have suffered a precipitous population decline as greater than 90% of their habitat has disappeared due to development and natural succession. Meadow habitats are easy targets for development, lacking the legal protections of wetlands and forests.

The 2007 Little Bennett Regional Park Master Plan proposed traditional park development on this site, which would have eliminated the existing meadow landscape. The current concept in the facility plan attempts to preserve most of the meadow, while achieving the full program of requirements from the master plan. The plan recommends a loosely structured, enticing linear experience that offers physical connections to various ecosystems of the park, and also includes an active central play hub. The play elements are integrated with the natural environment and provide opportunities for exploration, education, and enjoyment for all park

users. Various themes and areas for nature interpretation can also be provided along the way, including birds, bats, butterflies and insects. The concept proposes locating the play area at the smallest finger of meadow between two stream buffers for the following reasons:

- Preservation of Meadow: The area is away from the main meadow corridor, and has the least impact
 to the continuity of meadow habitat. The disruption of the vista from all angles of the site will be also
 minimized.
- **Minimized Site Disturbance**: This area is the flattest of the entire site. Grading can be minimized to accommodate the program needs. The only alternative area available would be the site of the future visitor center.
- **Interpretive Opportunities**: This area of the site is surrounded by various types of ecosystems that offer opportunities for hands-on environmental education.
- **Connectivity:** The area is located in proximity to other program elements and provides trail connections to the sledding hill, amphitheatre, outdoor classroom, and nature interpretation areas.

We believe the impact to forest and stream buffer are offset by the following environmental benefits provided by our proposed plan:

- Preservation of Meadow Adjacent to Stream Buffers: The plan preserves large expanses of
 undeveloped meadow outside of regulatory stream buffers for Sopers Branch. This effectively expands
 the current stream buffer to protect the watershed. If necessary, we would be willing expand the
 width of the regulated buffer to offset stream buffer areas impacted by the development of our play
 area.
- **Stormwater Treatment of Offsite Runoff**: The plan proposes to voluntarily mitigate 10.3 acres of upstream off-site stormwater from other development along MD 355. This is about 70 times more than the stormwater impact of the proposed play area.
- Reforestation: Although the project has no reforestation requirements, the plan proposes to allow
 woody plants to emerge through natural succession along the main drive. The succession will be
 managed to control invasive species. The strip will become part of the hedgerow and gradually
 contribute to reforestation of the site.
- Site Sustainability: The overall sustainability of the project is significantly improved compared to the
 original master plan concept and includes less than half of the road, stormwater treatment, and site
 disturbance originally proposed. The outdoor classroom building will be LEED certified, and many
 other sustainable development practices will be employed.

We believe our overall approach to the project minimizes environmental impact and supports our mission of preservation, conservation, education and recreation. We would appreciate the opportunity of meeting with you to discuss our project.

Ching-Fang Chen, RLA, LEED-AP Landscape Architect Park Development Division

The Maryland-National Capital Park and Planning Commission 9500 Brunett Avenue Silver Spring, MD 20901 301.495.2557 Ching-Fang.Chen@montgomeryparks.org

Chen, Ching-Fang

From: Chen, Ching-Fang

Sent: Wednesday, July 27, 2011 1:12 PM

To: Penn, Joshua

Cc: Carter, John; Murray, Callum; McManus, Patricia; Pedoeem, Mitra

Subject: Little Bennett RP-Day Use Area PreFCP

Attachments: Little Bennett Day Use Area.pdf

Josh.

We really appreciate your guidance during the facility plan process and are already to submit the Forest Conservation Plan for Little Bennett Regional Park-Day Use Area. In June we discussed the design intent and implementation of the Forest Conservation regulation for the project with your team. We are able to rework the design to fulfill your recommendations.

- A large portion of the playscape program in the revised plan has been shifted to the west out of the stream buffer area.
- The revised plan keeps the limit of disturbance away from the existing tree line as delineated in the approved NRI/FSD plan and allows a minimum 5' buffer from the existing mown edge.
- Interpretation trails have been simplified to minimize stream crossing and site disturbance. One on the south and one on the north, the trails provide necessary connection to future crossings of the Sopers Branch and to the interior of the park as recommended in the 2007 Little Bennett Regional Park Master Plan.
- The interpretation boardwalk is intended to confine the visitors and provide ADA access. The 5' wide boardwalks are aligned to stay completely out of the critical root zones (CRZ) of trees larger than 30" dbh and CRZ of all 24"+ dbh trees except in two locations. The boardwalk will be constructed by hand with helical pile to minimize site disturbance. The transparent metal grating will ensure protection of existing hydrology and forest floor.
- The 5' wide mown trails in the meadow have also been aligned to keep away from the tree lines.

The facility plan is on the September 8th agenda for Planning Board approval. I understand we are tight on the PreFCP review schedule. Do you think it will feasible to proceed and receive approval in time for the Board? Your recommendation will be much appreciated.

We are ready to re-submit for stormwater management concept to DPS. The reviewer also needs your confirmation that it is acceptable for park program to be in the stream buffer area. We would like to ask for your help to provide a letter of confirmation.

Thank you very much. .

Ching-Fang Chen, RLA, LEED-AP Landscape Architect Park Development Division

The Maryland-National Capital Park and Planning Commission 9500 Brunett Avenue
Silver Spring, MD 20901
301.495.2557
Ching-Fang.Chen@montgomeryparks.org





From: Chuck Harper, P.E. June 17, 2011

Project: Little Bennett Gateway Facility Plan AMT: 108-157.008

 Meeting Number:
 Meeting Date:
 Project Phase:
 Meeting Location

 FM-1
 June 14, 2011
 Design Development
 Montgomery County DPS 255 Rockville Pike; 2nd Floor

Meeting Purpose:

Meeting with Fire Marshal to discuss access requirements

Attendees: Distribution: Attachments:

Marie LaBaw (FM)
Ching Fong Chen (MMCDDC)
All Attendees

Marie LaBaw (FM)
Ching-Fang Chen (MNCPPC)
Eileen Emmet (MNCPPC)
Chuck Harper (AMT)
Bruce Anderson (Cutler Anderson)
Lanshing Hwang (Symbiosis)

Item	Action	Description
1.		Ching-Fang opened the meeting by introducing the vision and intent of the Little Bennett Park design.
2.		Marie stated that a 12' wide access road will be required to both the Playscape area and the Underlook Building for EMS access.
3.		The 12' wide access road can be 10' of pavement with 1' of alternative surface on each side. Grasspave 2 is not an acceptable alternative surface; Grasscrete is acceptable.
4.	AMT	An ambulance turnaround is required for both the Playscape area and Underlook building. Once locations are determined, AMT is to show Auto-turn movements for turnaround. Auto-turn studies are to be based off ambulance movements.
5.		Marie stated that ATV's will not be an option.
6.		As currently designed, driving through the building, in lieu of providing turnaround, does not work due to lack of vertical clearance. 13.5' of vertical clearance is required. (Note: horizontal clearance was deemed adequate.)
7.		Using the serpentine access path that starts at the southern end of Meadow Drive as the EMS access was discussed. The path is not currently designed as a paved, and would also need to be widened. Amphitheatre could serve as a turnaround for this option.
8.	ML	Since there are no compliant water source nearby, a 30,000 Gallon storage tank will be required within a mile of the building. (Distance measured by truck travel path). ML to send CH generic spec of tank.
9.		A 25' x 80' operational area is required at the location of the 30,000 Gallon storage tank. The operational area must be paved.
10.		Sprinklers are not required for the Underlook building.
11.		In summary, there are three (3) options for EMS access to the Underlook Building:
		Increase the building height and use Amphitheatre as vehicle turnaround.
		Provide turnaround area along ADA path, before building. Widen path to 12'.
		Pave and increase width of the Maintenance Path that begins at the South end of Meadow Drive.
		rave and increase width or the Maintenance Path that begins at the South end of Meadow Drivi





12.	AMT and CA to coordinate and prepare a submission to ML showing Auto-turn movements, tank location, site signage, Typical sections of the pathways, building type, etc If acceptable, ML can issue a letter saying "the preliminary concept has been agreed upon."
13.	Lanshing was instructed to talk the inspector regarding burning the meadow. ML to email the name of the inspector.

Chuck Harper, P.E.

A Morton Thomas and Associates

Note: The above meeting minutes are the interpretation of the writer. If you feel any item requires correction or clarification, please notify the writer within 10 business days.

MEETING REPORT

MEETING DATE: December 1, 2010

STAFF ATTENDING: John Hench Parks Park Planning & Stewardship Division

Joey LamplParksCultural Resources StewardshipHeather BouslogParksCultural Resources Stewardship

Wendy Hanley Parks Northern Region Steve Radov Parks Northern Region

Patricia McManus Parks Design Ching-Fang Chen Parks Design

PROJECT: Little Bennett Regional Park, Day Use Area-Facility Planning

SUBJECT: Native American Heritage Interpretive Program

The park staff met and discussed issues related to Native American heritage in the Montgomery County Park system and clarified the program for the Little Bennett Regional Park- Day Use Area.

The archaeological data provided by the Cultural Resources Section indicated that the Native American people made less use of the piedmont region than other areas. There are no significant traces of land use by Native Americans at the Little Bennett Regional Park. The Native American camping sites found were primarily situated near the Potomac River such as rock shelters at Block House Point.

It's our objective to interpret the natural, cultural and historic resources in the park system. Native Americans are one of the important cultural and historic heritages in the County. Currently, the Montgomery County Parks are able to accommodate temporary interpretive programs at facilities that are suitable for program space and have the resources for programming, such as Black Hill Regional Park Visitor Center and Brookside Gardens Visitors Center. Contact Steve Radov for more information.

In the short term, the Cultural Resources Stewardship Section is considering an initiative to include funding in the Capital Improvements Program for Native American Heritage interpretation. The team proposed to represent Native American culture in the parks by interpretive signage at well-visited park sites, or known sites which have archaeological findings. It has been discussed that the interpretation should include what was here by the Native American as well as contemporary Native American culture in the area. Joey Lampl is the contact person for the project.

You may be aware that the Visitor Welcome and Nature Center has been put on hold in the facility plan due to current budget crisis. It's understood that a permanent interpretive program of the Native American heritage will be included when the Visitor Welcome and Nature Center is in place. We will make sure to involve your organization in the process when the project is realized in the future.

This report was prepared by:			
Ching-Fang Chen	Distributed for Review: January 14, 2011		

Copy: Attendees

Landscape Architect/Project Manager

MEETING REPORT

MEETING DATE: January 6, 2010

ATTENDING:

Ki Kim Joshua Penn Eugene Rose Rob Gibbs Bill Hamilton Norma Kawecki Rachel Newhouse Andrew Frank Stephen Reid Mike Horrigan Jim Humerick Wendy Hanley Steve Radov	Planning Planning Parks	Transportation Planning Environmental Planning Horticulture & Arboriculture Natural Resources Natural Resources Natural Resources Park & Trail Planning Environmental Engineering Environmental Engineering Northern Region Northern Region Northern Region
		•
Steve Radov Patricia McManus Heidi Sussmann Eileen Emmet Ching-Fang Chen	Parks Parks Parks Parks Parks	Northern Region Design Design Design Design

PROJECT: Little Bennett Regional Park, Day Use Area - Facility Planning

SUBJECT: Kick-off Meeting

Agenda

The kick-off meeting was intended to:

- Brief the PDCO team with background information.
- Summarize insights of the Little Bennett Regional Park Master Plan and Clarksburg Master Plan and Hyattstown Special Study Area.
- Establish the core value and vision for the team and the project.
- Review the approved concept of the Gateway Area (Day Use Area).
- Identify opportunities and differentiate the park of the Day Use Area from adjacent parks.
- Review examples of sustainable park projects.
- Review examples of recently accomplished green buildings for the visitor center.
- Discuss project approaches and program strategies.

Vision (as presented)

- Design a forward-thinking park for future generations.
- Offer unique, cohesive and memorable day use experience.
- Create experience of discovery and learning through story telling of nature.
- Promote people's understanding of nature and stewardship of land.
- Provide a platform for M-NCPPC to explore sustainable park development and to share knowledge, experiences and best practices with the visitors.
- Integrate design aspects, aesthetics, ecology into park management.

Comments and Discussion

Comments made at the meeting are summarized below.

Vision

- 1. The presented vision was well received. The team expressed strong support of the concept of sustainable park development and land stewardship.
- 2. The team discussed our current practices and what we might do differently in the future. The team examined the mission of offering recreation, education, and land management as well as contributing to the environment in terms of carbon sequestration, habitat restoration and climate control through implementation of the project.

Concept

- The team supported the Gateway Area Concept as proposed on the 2007 Master Plan focusing on nature oriented activities including picnic areas, meadow habitats, nature based adventure playground, group fire ring and amphitheatre, and interpretive landscapes. The design of the elements shall enhance the beauty of nature and the spirit of the park.
- The team expressed concerns of the focal point water feature. The feasibility of sustaining a water feature that is able to retain standing water based on the existing watershed, topography and soil will require considerable efforts of engineering and construction. It will likely be very costly. The team agreed that the element should be reexamined.

Visitor Center

- 1. The team supported realizing the concept of the Visitor Welcome and Nature Center as recommended on the 2007 Master Plan. Though a Countywide Interpretive Center Functional Plan has not been studied, the team expressed strong support of moving forward with facility planning of the visitor center. The team considered the visitor center to be an integral part of the Day Use Area and shall be developed side by side with the rest of the Day Use Area.
- 2. The location of the visitor center was discussed. The current location as shown on the Gateway Area Concept Plan was intended to keep the facility away from the interior of the environmentally sensitive area to protect the park's natural resources. For the purpose of a visitor welcome center, the concept is well-acknowledged. For the interpretive purpose of a nature center, the team expressed concerns that the location may be too far from the heart of nature to be interpreted, too close to the traffic and noise of MD-355 and I-270 and the setting to be lacking in natural character. There was

- discussion of potentially enhancing a landscape buffer along MD-355 in the near future, rather than waiting for the project to be implemented. There are upcoming plans to widen MD-355.
- 3. The team agreed that the Visitor Center shall be a green building with minimum carbon footprint. It shall be an example and inspiration to the community for sustainable park facility of the 21st century.
- 4. The team recommended follow up meetings involving a focused group of the PDCO team members to discuss, evaluate and study the requirements of the visitor and nature center

Landscape

- 1. The facility planning intends to carry out the concept proposed in the 2007 Master Plan of the Gateway Concept as nature based recreational and interpretive landscapes.
- 2. The concept of "carbon surplus landscape" was well received. The concept of innovative planting design by *Dr. Norbert Kühn* was conveyed; use plants that can clearly build stable communities under the give conditions of a site and to transform the plant communities according to a design perspective to enhance the aesthetics of the landscape.
- 3. The team supported exploring sustainable park development strategies such as: maintaining the current state through appropriate measures; allowing succession to proceed naturally; effecting changes in succession through interventions and improving the aesthetic value by changing the species composition. Possibilities of allowing plants and animals to evolve from the current state of a disturbed cropland to various states of successional landscapes and habitats were discussed. The edge communities occurring through succession will provide a higher degree of ecological service, from carbon sequestration, recycling, bio-filtration, and wildlife habitat than landscapes of conventional park land.
- 4. The interpretive program can be rooted in storytelling of the evolving ecosystems. The landscape shall be authentic, is a reminder of the site and is part of nature and the natural dynamic.
- 5. The landscape shall be designed so it can be maintained for a long time with less care and with low cost.

Access

1. The concept of assuring the park will be accessible by different modes of transportation was reinforced by the team. It is in the future Plan for the Corridor City Transitway to service the park vicinity along MD-355. The Plan does not include a transit stop near the public entrance as proposed in the Master Plan for Little Bennett Regional Park. The team suggested advocating a bus stop at the proposed entrance location to improve pedestrian connection, ease of access, and safety to the park by public transportation.

Community Input

1. It's the team's intention to involve the community. The team proposed to organize the first community meeting after the program of requirements for the Visitor Welcome and Nature Center is established.

Moving forward

- 1. The landscape architectural aspect of the project will be studied and developed in house.
- Project manager plans to send out task orders to multi-disciplinary consultants by mid February for the scope of survey, NRI/FSD, Preliminary Forest Conservation Plan, Stormwater Management, Environmental Site Design and Geotechnical Investigation. We intend to begin work on the survey and NRI/FSD right away, as recommended by Environmental Planning staff.
- 3. For visitor center, the project manager is pursuing consultants with relevant expertise to work with the PDCO team in the pre-concept phase to facilitate program development. A/E consultants and interpretive designer with expertise of visitor and nature center will be procured after the program of requirements is further defined.
- 4. For landscape, the project manager intends to pursue consultants with expertise in sustainable design, soil ecology, meadow habitat and succession management to work with the PDCO team on the facility planning.
- 5. A follow up meeting for the visitor center with the focused group has been scheduled for February 19th.

Distributed for Review: February 4, 2010

6. A site visit with the PDCO team will be scheduled for early spring.

This meeting report will be recorded as accepted unless the preparer is contacted in writing within two weeks. This report was prepared by:

Ching-Fang Chen
Landscape Architect/Project Manager

Copy: Attendees

Gerald Barrick, Central Maintenance

Harper, Chuck

From:

Ernest, Matthew

Sent:

Wednesday, November 17, 2010 11:30 AM

To:

Corren Giles (cgiles@sha.state.md.us)

Cc:

Chen, Ching-Fang; Torgerson, Steven; Harper, Chuck; Ernest, Matthew

Subject:

Little Bennet Facility Plan - Intersection Site Distance Plans

Attachments: SHA-1.pdf; SHA-2.pdf; SHA-3.pdf; SHA-4.pdf; _LB-sight distance evaluation form.pdf; CS100.pdf

Hi Corren,

I wanted to follow up from a conversation you had with Steve Torgerson from our office. Attached are revised site plans, site distance profiles and ISD worksheet for your review.

The project is only a facility plan. So construction documents are not being prepared. Whenever the project moves forward, a formal access permit submission would be made at that time. We are looking for SHA concurrence on our entrance configuration and locations.

Please review and let me know if you need any additional information or if you have any questions.

Thanks,

Matthew Ernest, P.E. Associate

A. Morton Thomas & Associates, Inc.

12750 Twinbrook Parkway Rockville, MD 20852

Phone: 301-881-2545 | Fax: 301-881-0814 | Cell: 240-994-2217

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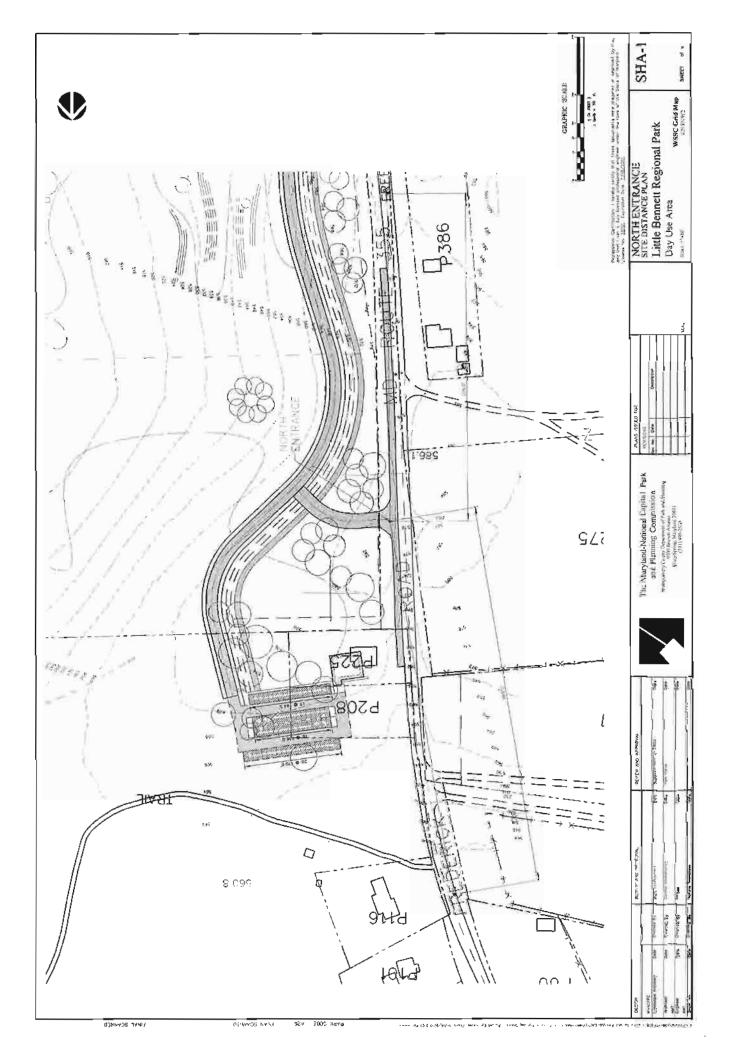
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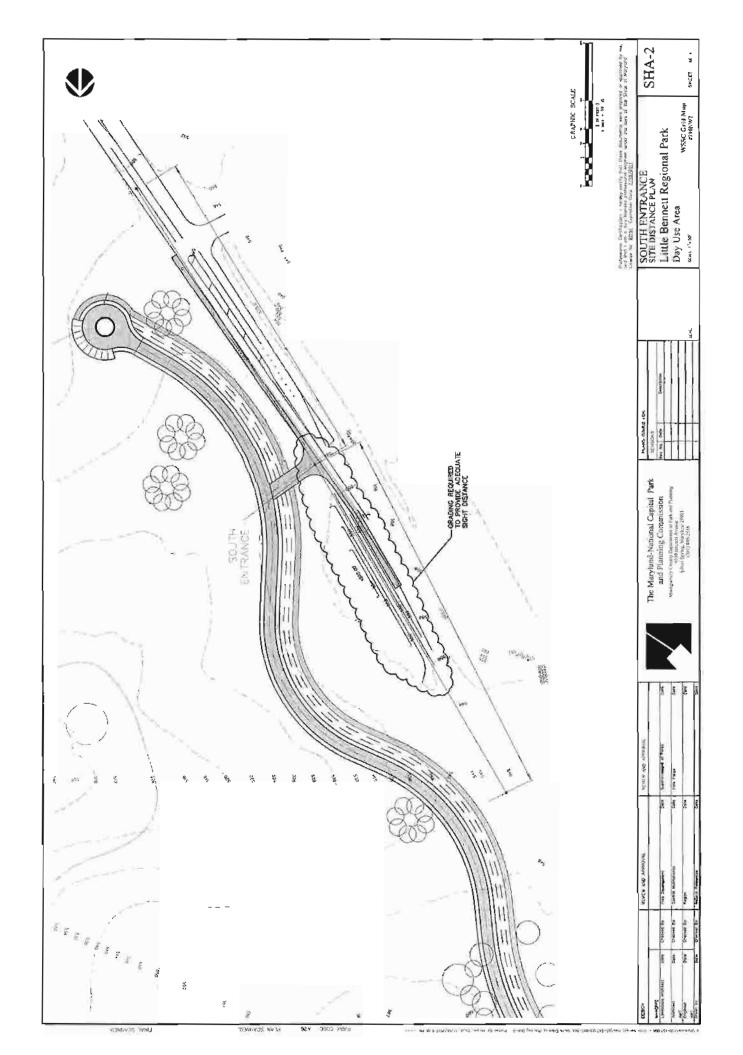
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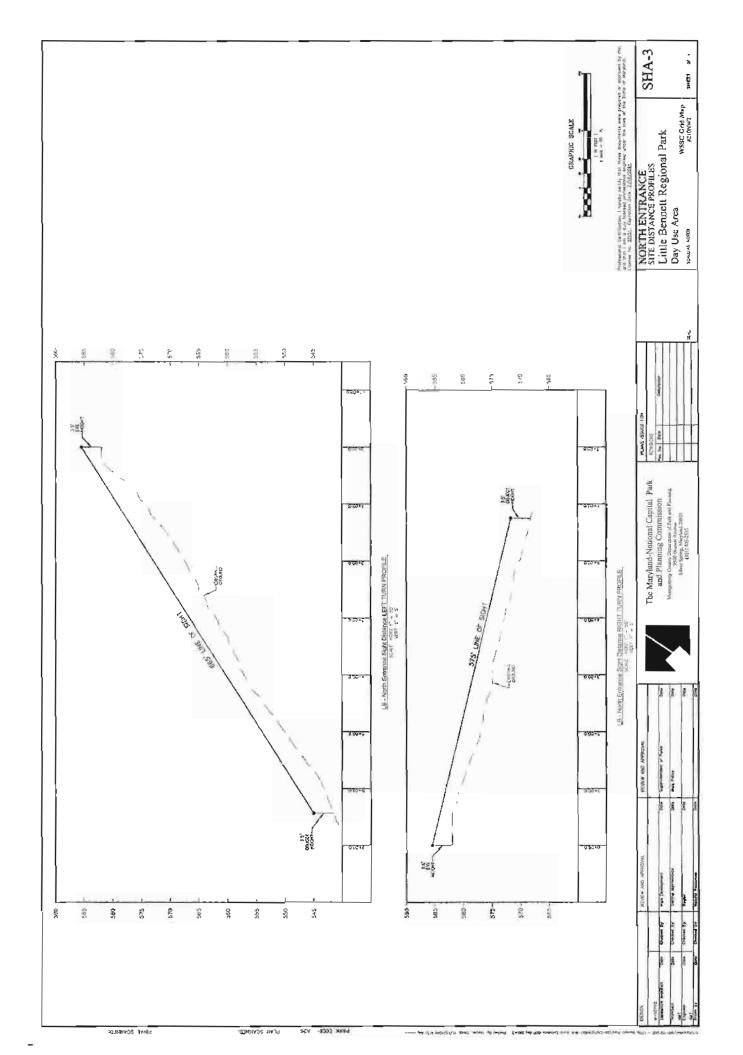
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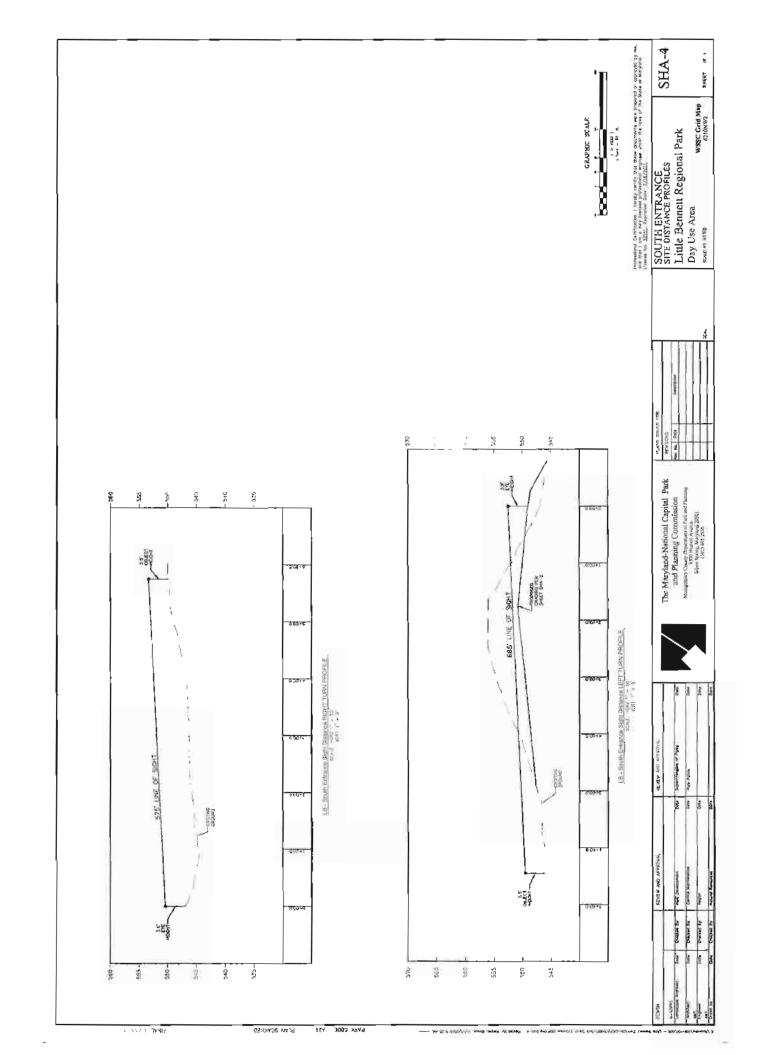
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Sight Distance Measurement and Evaluation Worksheet

LEFT (North)	RIGHT (South)
	SITE

INTERSECTION SIGHT DISTANCE	MEASURE	MENT (ft)
3.5' object placed at proposed access	LEFT	RIGHT
 3.5' driver's eye height on approaching lane 	665	575
STOPPING SIGHT DISTANCE	MEASURE	MENT (ft):
2.0' object placed at proposed access	LEFT	RIGHT
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Evaluation

Posted Speed = ____ 50 _ mph
Design Speed = Posted Speed + 10 mph = ____ 60 _ mph (EAPD Policy)

Intersection Sight Distance (ISD):

Turning Movement	State Standard ISD Requirement Based on Design Speed	Reduced ISD Requirement Based on Posted Speed *
Left Turn from Site Access	665	555
Left Turn into Site Access	570	425 -
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^{*}Substandard condition meeting this requirement may be acceptable upon consideration of site specific traffic and safety conditions, feasibility constraints, etc. Mitigation may be required for any substandard condition.

Stopping Sight Distance (SSD):		
SSD Required for the Design Speed:	570	_ f

Results

Please refer to AASHT	O's Policy on	Geometric De	sign of Highwa	ays and Streets, Tables 9-	5 and 9-58.
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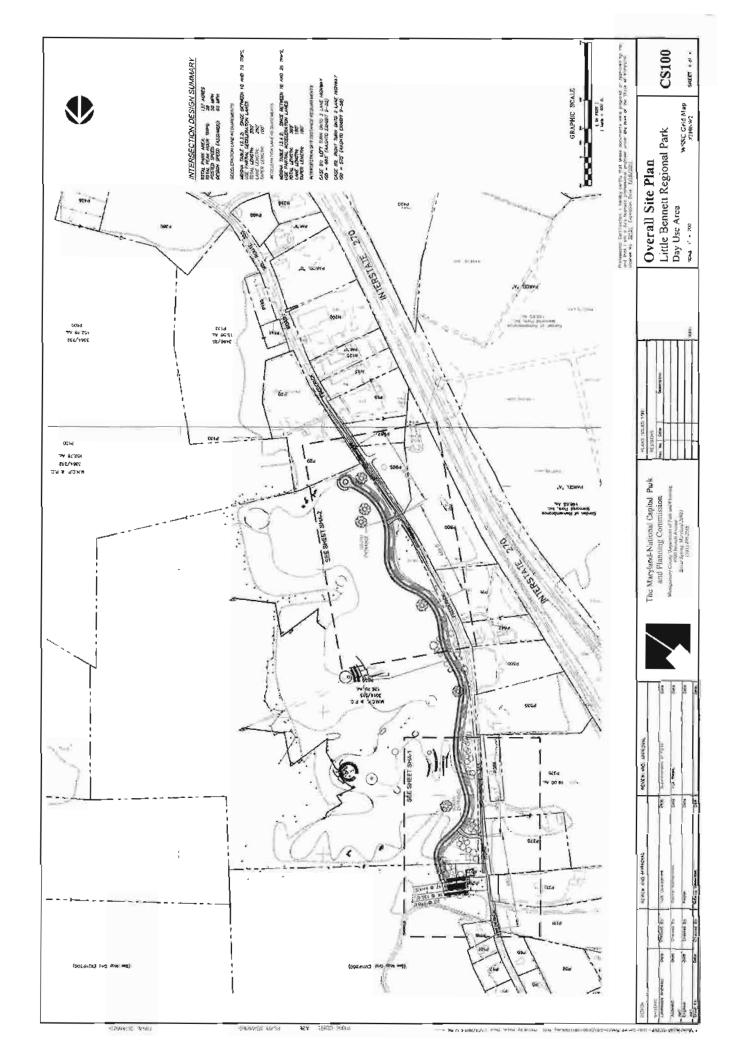
Engineering Access Permits Division SIGHT DISTANCE QUICK REFERENCE CHART

(Use for Field Review of Proposed Access Locations)

Full Movement Access on 2-Lane Undivided Highway					
Posted Speed	Assumed Design	Stopping Sight			
(mph)	Speed (mph)	Distance Required	Distance Required		
		(ft)	(ft)		
30	40	445	305		
35	45	500	360		
40	50	555	425		
45	55	610	495		
50	60	665	570		
55	65	720	645		
60	70	775	730		
65	75	830	820 .		

Notes:

- 1. Assumes passenger vehicle for design purposes and level grade ± 3% maximum. Refer to AASHTO if conditions differ.
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- 4. If alternate access locations are not available, the substandard conditions should be evaluated in detail with respect to specific movements. Mitigation and/or denial of specific movements that do not meet sight distance standards may be required. This should be discussed with the RSE or ADC prior to any conceptual approval of an access location.



Harper, Chuck

From: Corren Giles [CGiles@sha.state.md.us]

Sent: Friday, December 17, 2010 8:49 AM

To: Torgerson, Steven

Cc: Ernest, Matthew; Osband, Greg; 'Chen, Ching-Fang'; Raymond Burns (SHA)

Subject: RE: MNCPPC Little Bennett Regional Park Gateway - SHA Access Coordination Questions

Steve,

Going back through my emails, I reviewed the exhibits sent to me by Matthew. Regarding the sight distance, as long as MNCPPC is planning to modify the roadway grading as shown in the exhibits and the sight distance requirements can be met, the southern access point should be ok.

Corren V. Giles, EIT

Chief, Construction Support Section

From: Corren Giles

Sent: Tuesday, December 07, 2010 2:49 PM

To: 'Torgerson, Steven'

Cc: Ernest, Matthew; Osband, Greg; Chen, Ching-Fang

Subject: RE: MNCPPC Little Bennett Regional Park Gateway - SHA Access Coordination Questions

Hello Steve,

I have completed my review and offer the following comments. Obviously since this is not a detailed plan, I don't have many comments to offer at this point. Thank you for your patience. I have attached a checklist to be used once you are ready to submit for a formal review and the sight distance evaluation form.

General

- Each access point cannot be officially approved until sight distance evaluations (on the SHA form) have been submitted to ensure that both intersection and stopping sight distances are adequate. For the south entrance when exiting, there is a hill to the right which may impact reaching the required sight distances.
- A 5' shoulder must be provided along the entire property frontage.
- Will there be sports programs and events that will generate peak hour traffic which may necessitate a left turn lane at either or both access points?

North Entrance

- This should be relocated and placed opposite the access just south of the proposed location, on the west side of MD 355.
- Only 12' wide accel/decel lanes are required; but 13' can be provided if it is the desire of the County.
 The length will be determined once more detailed project information and data is provided.

South Entrance

- The location of this access may need to change if the sight distance is not sufficient or geometric changes will be required along MD 355 to mitigate any deficiency.
- Only 12' wide accel/decel lanes are required; but 13' can be provided if it is the desire of the County.
 The length will be determined once more detailed project information and data is provided.

Please let me know if you have any questions.

<u>PLEASE NOTE:</u> As of December 15th, I will be working in the Office of Construction. **Starting the 15th,** please contact Ray Burns for all Montgomery County Access Permits matters. He can be reached

at rburns1@sha.state.md.us or 410-545-5592. Please forward this announcement to others in your office.

Corren V. Giles

Montgomery County Area Engineer Access Management Division State Highway Administration 707 N. Calvert Street C-302 Baltimore, MD 21202 (410) 545-5595 phone (410) 209-5026 fax

From: Torgerson, Steven [mailto:storgerson@amtengineering.com]

Sent: Friday, December 03, 2010 9:44 AM

To: Corren Giles

Cc: Ernest, Matthew; Osband, Greg; Chen, Ching-Fang

Subject: MNCPPC Little Bennett Regional Park Gateway - SHA Access Coordination Questions

Good Morning Corren,

I hope that you had a wonderful holiday weekend. I am following up on the frontage access on 355 to the Little Bennett Regional Park Gateway Facility Plan. We are getting close to finalizing our stormwater concept and interior vehicular circulation for the day use area. One of the critical pieces of information that we are missing is a letter of confirmation from SHA that the access locations off 355 are acceptable. As per our earlier conversation, we are currently developing the facility plan. When completed, this document will bring the design of the park to DD documents. The plans will still have to be submitted to SHA during the preparation of the final construction documents. This will not happen until the project is funded for construction; however, we want to know that we are developing a set of plans that can easily be transferred for final construction documents. In addition, the facility plan needs to be as accurate as possible for pricing and public outreach purposes. Please see the following hyperlink for conceptual illustrative of the site.

http://friendsoflittlebennett.org/PDF/DayUseAreaPlan.pdf

What does your time frame look like to complete your review? Is there any way that you can get something to us by the end of the week? Please let me know if you need any information or if you would like to discuss the project in more detail. I can be reached at 301.881.2545.

Best regards,

Steven Torgerson, ASLA, RLA

A. Morton Thomas & Associates, Inc.

12750 Twinbrook Parkway Rockville, MD 20852

Phone: 301-881-2545 | Fax: 301-881-0814 | Cell: 240-599-6683

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Subject:

Little Bennet Facility Plan - Intersection Site Distance Plans

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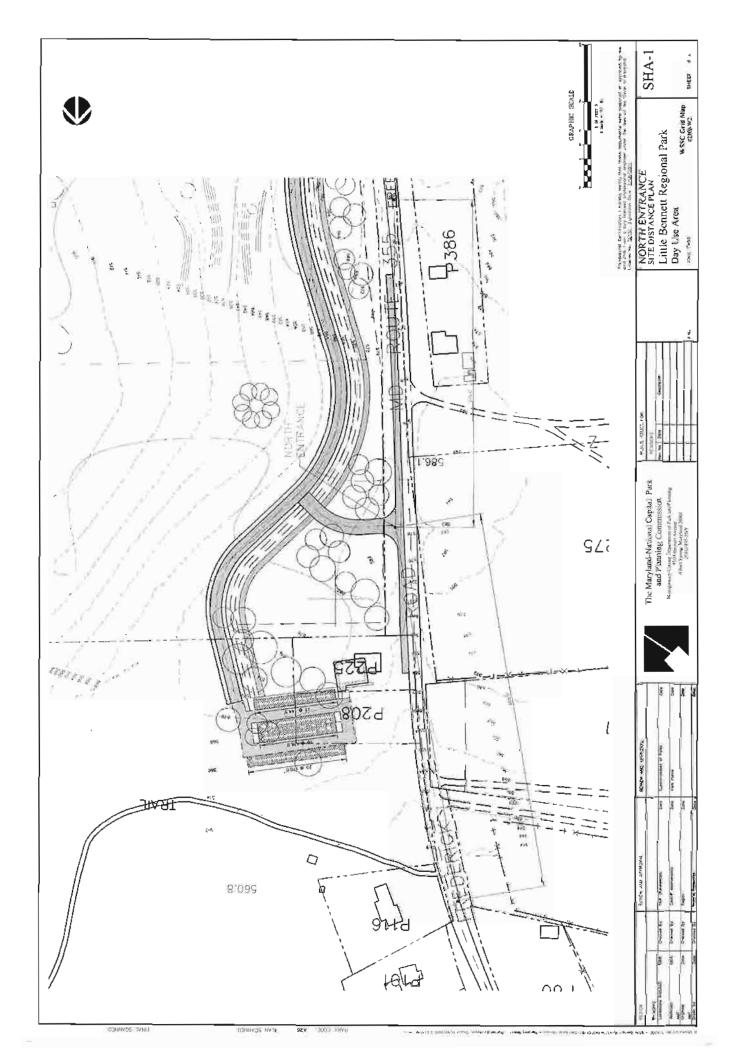
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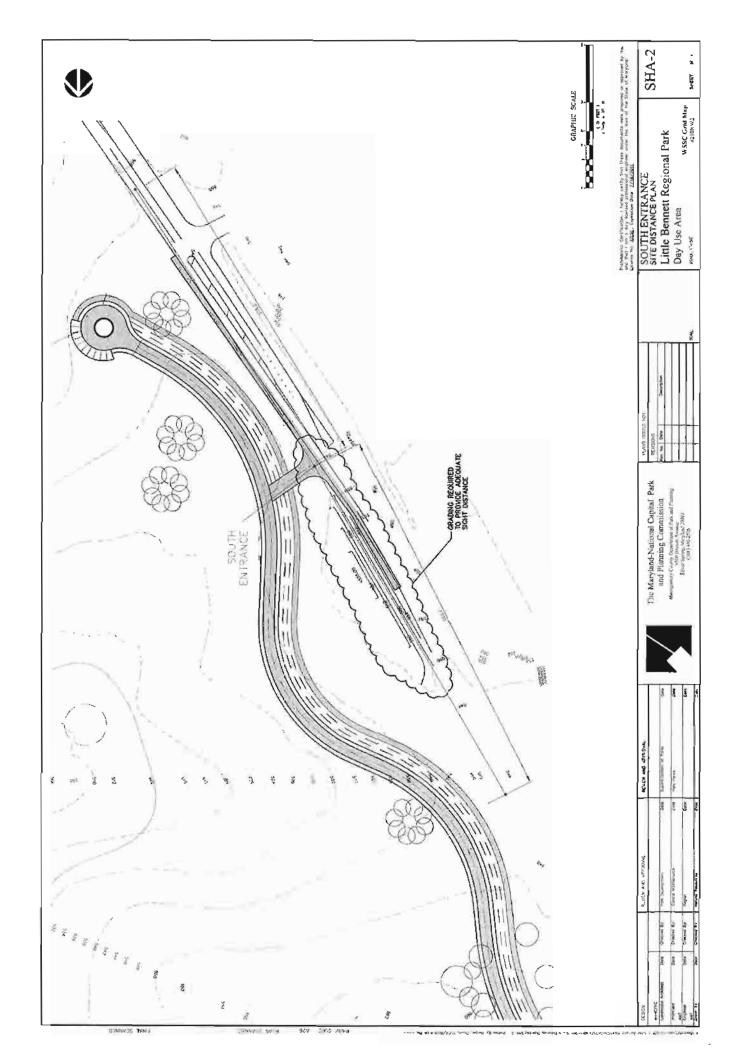
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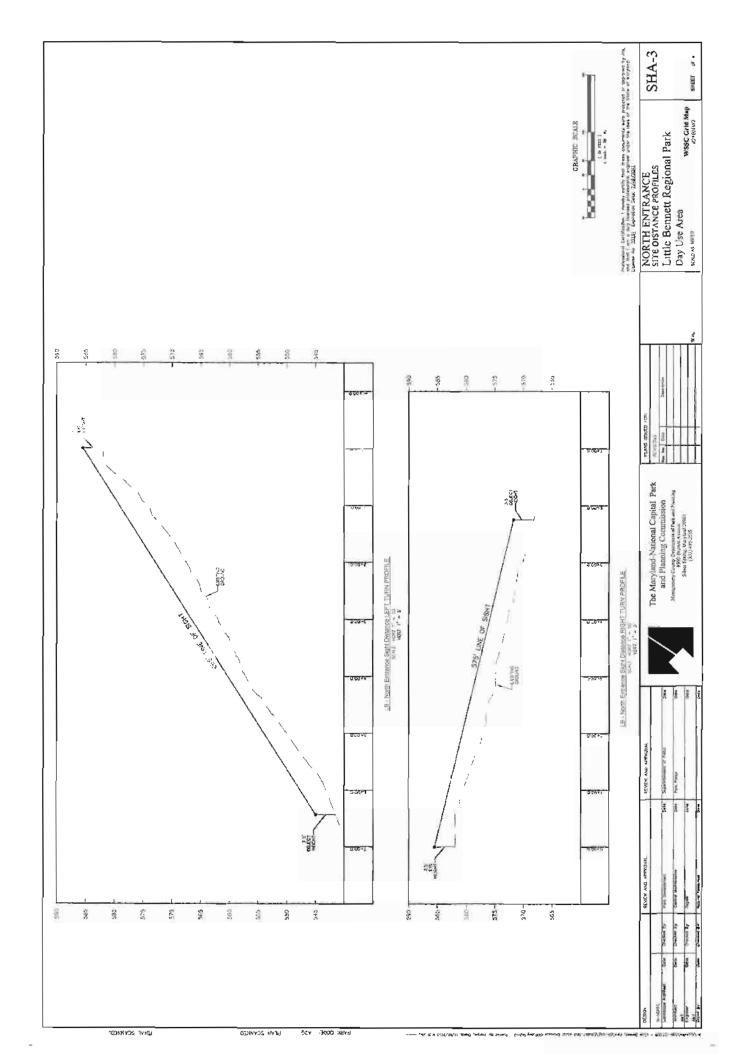
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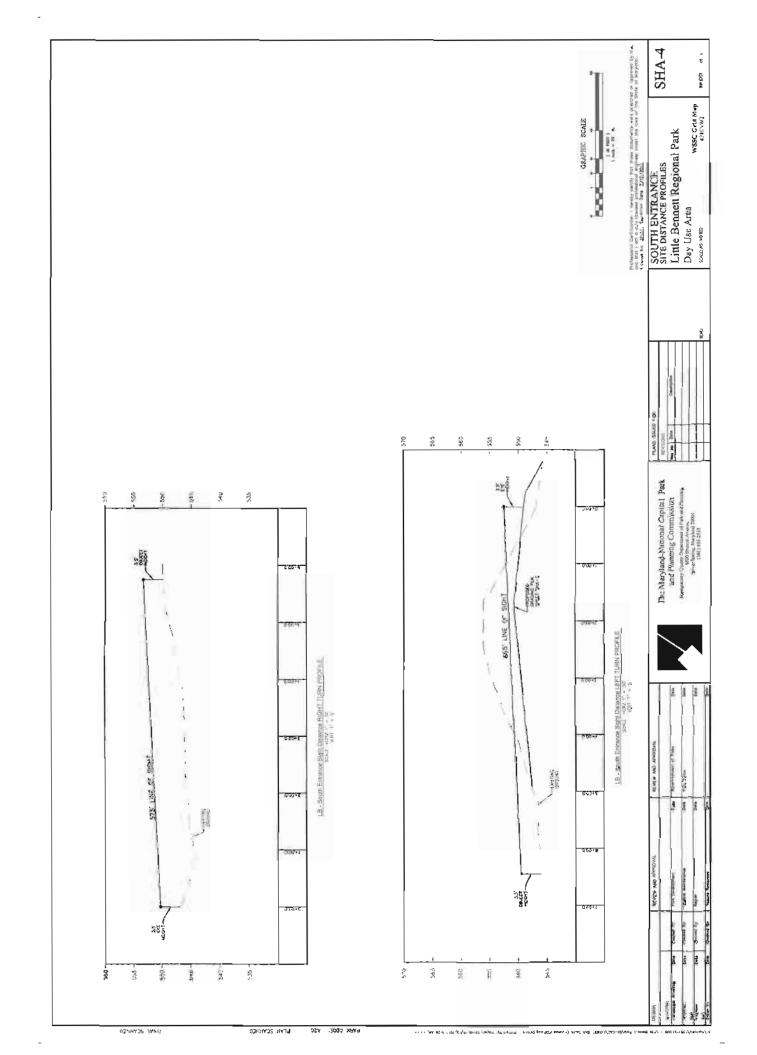
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Engineering Access Permits Division

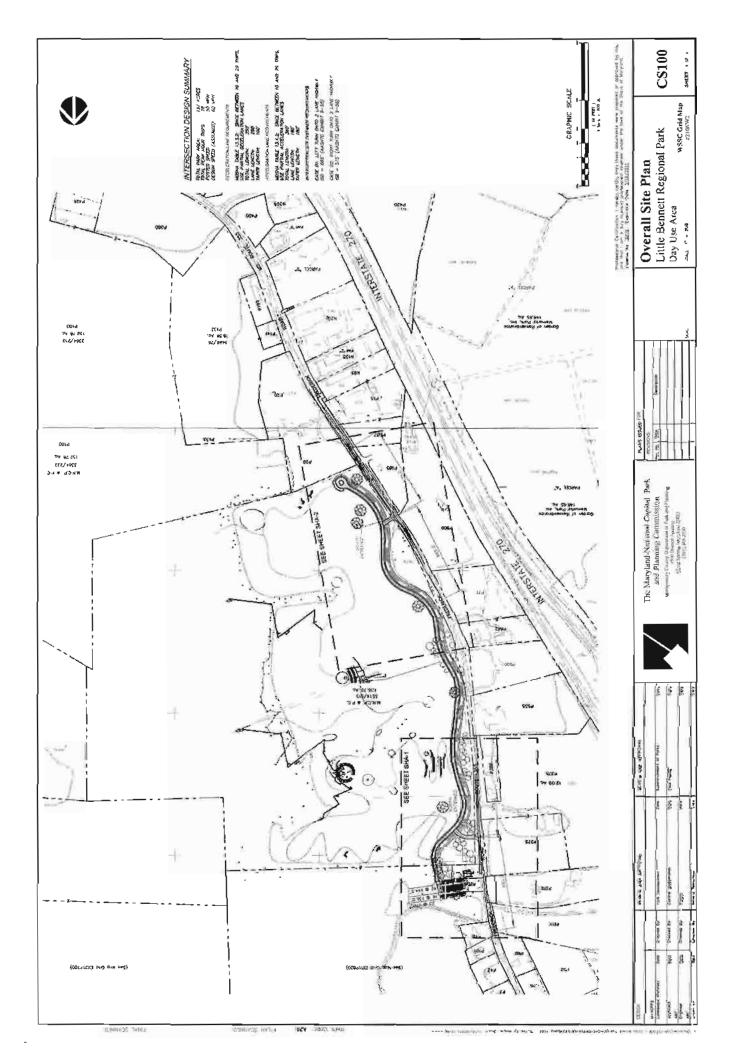
SIGHT DISTANCE QUICK REFERENCE CHART

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MEETING REPORT

MEETING DATE: June 8, 2011

STAFF ATTENDING: Ki Kim Planning Area 3

Charles Kines Parks Park Planning & Stewardship

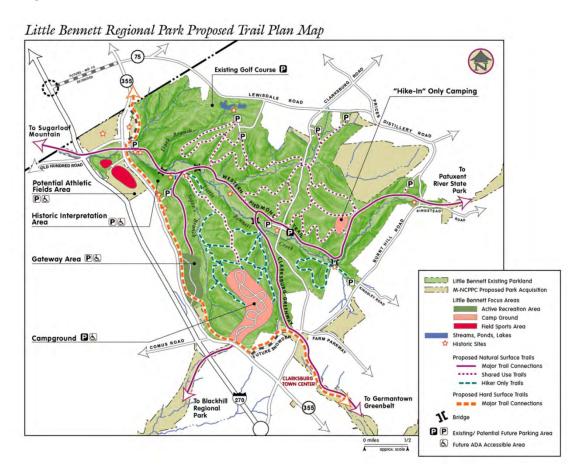
Wendy Hanley Parks Northern Region
Patricia McManus Parks Park Development
Marian Elsasser Parks Park Development
Ching-Fang Chen Parks Park Development

PROJECT: Little Bennett Regional Park, Day Use Area-Facility Planning

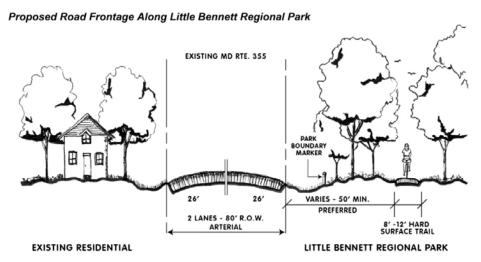
SUBJECT: Clarksburg Greenway and Countywide Bikeways

The park staff met and discussed issues related to Clarksburg Greenway and Countywide Bikeways for the facility plan of the Little Bennett Regional Park-Day Use Area.

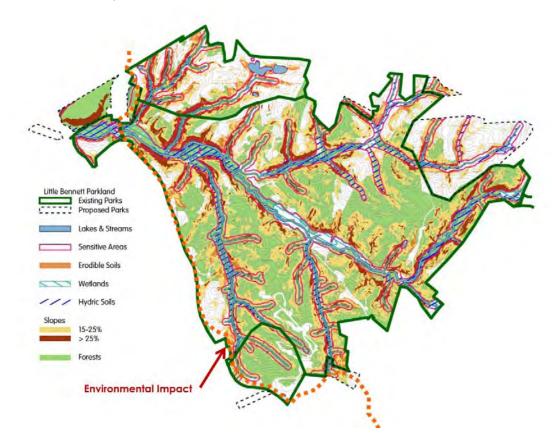
Background



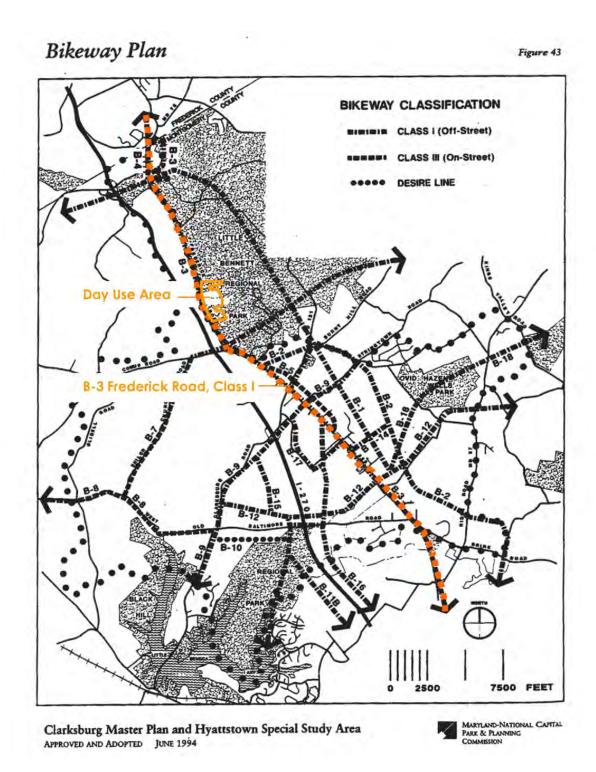
The 2007 Little Bennett Regional Park Master Plan emphasizes the concept of accessibility by different transportation modes. The Plan extends the Clarksburg greenway network to the park so people can get to the park without using their car. The Plan recommended that the trail be set back from the road right-of-way on Route 355 a minimum of 50 feet to provide a better park experience.



The section of the trail between the maintenance yard and the day use area was shown in the Little Bennett master plan to be routed through parkland near Soper's Branch, rather than along the road right-of-way to provide a better park experience. This might not be feasible for environmental reasons. The trail was shown to connect the day use area and then extend north along Route 355 in the right-of-way to Route 109 and Hyattstown.



The 1994 Clarksburg Master Plan intended the transportation bikeway on Route 355, Frederick Road, to be integrated with the Clarksburg Greenway Trail (page 131 of Clarksburg Master Plan.) The B-3, Class I (Off-Street) transportation bikeway runs along the west side of Route 355 south of Route 121, Clarksburg Road, and is intended to be located along the east side of Route 355 north of Route 121. The bikeway is intended to extend south at least to the Montgomery County Fairgrounds in Gaithersburg. The Plan recommended implementing the bikeway system as development occurs.

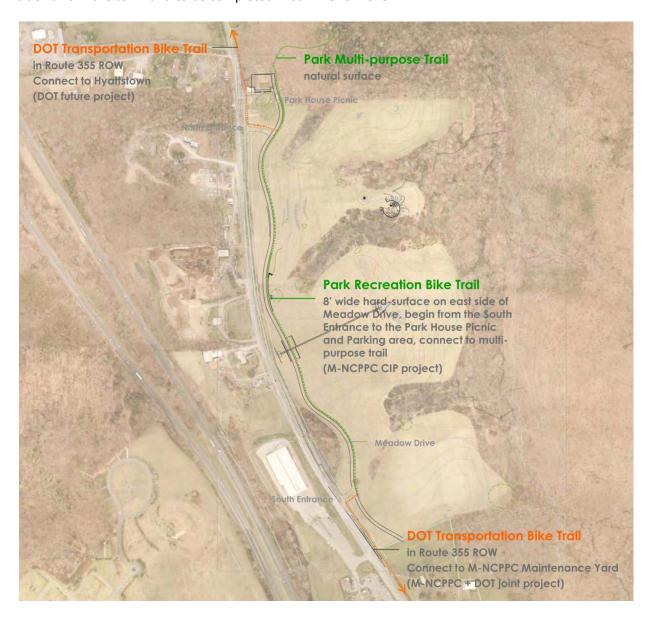


Discussion

Trail Connection

The proposed Day Use Area and Hyattstown are the major destination for the bike trail. It is important to make the connection from the terminus of the Clarksburg Greenway trail to the day use area at the time the park is developed. The bike trail between the maintenance yard and the day use area does not exist and Montgomery County DOT does not currently have this project in their CIP. The trail would be about a mile long and could be located along the road ROW, if the location in the park were unfeasible. Staff will contact Gail Tait-Nouri at DOT to discuss coordination and potential timing for this project and whether the planning for the trail could be a joint project in either the Parks CIP or the County CIP.

MCDOT has agreed to build the trail on the east side of Route 355 between Hyattstown Mill Road and Route 109 without widening the road. The trail is currently designed to connect to their planned sidewalks in the town and to be completed in summer of 2013.



Proposed Trail Location

Staff discussed whether the transportation bikeway would be routed along Route 355 right-of-way in front of the day use area or whether it could be included along the roadway within the day use area. The Route 355 ROW in this area is constrained by steep slopes and a hedgerow that is providing screening for the park. Park staff would prefer to have the trail run along the park access road, Meadow Drive, to serve both the recreation and transportation function. The alignment can be well-integrated with topography. Stormwater can be efficiently managed and the hedgerow buffer can be preserved. The recreation bike trail will connect to the transportation bike trail along the road north and south of the day use area. The trail will be set back from the busy Route 355 traffic as recommended in the Little Bennett Regional Park Master Plan. The park plan can accommodate an eight-foot wide hard-surface pedestrian and bike trail on the east side of the Meadow Drive. This area was originally intended to be used as a shoulder for casual parking. Because of the deferred development of the Visitor Welcome and Nature Center, the park plan can provide adequate parking without using this area.



Lighting

Transportation bikeways should be lighted, be in the Master Plan ROW or reasonably close to it, and be open 24 hours. The recreational trail does not have lighting because the Park facilities are close at dark. The County may not want the trail to be routed through the park but rather remain on the roadway. The park plan will propose the trail location here on an interim basis to fulfill the access needs but will not propose lighting. If DOT decides to locate the bikeway in the right-of-way in front of the park, the park trail could be converted to parking at a later date. If the trail were to remain within the park, lighting could be added in the future. The Clarksburg Greenway Trail is currently not lighted.

This report was prepared by:

Ching-Fang Chen
Landscape Architect/Project Manager

Copy: Attendees & PDCO team

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